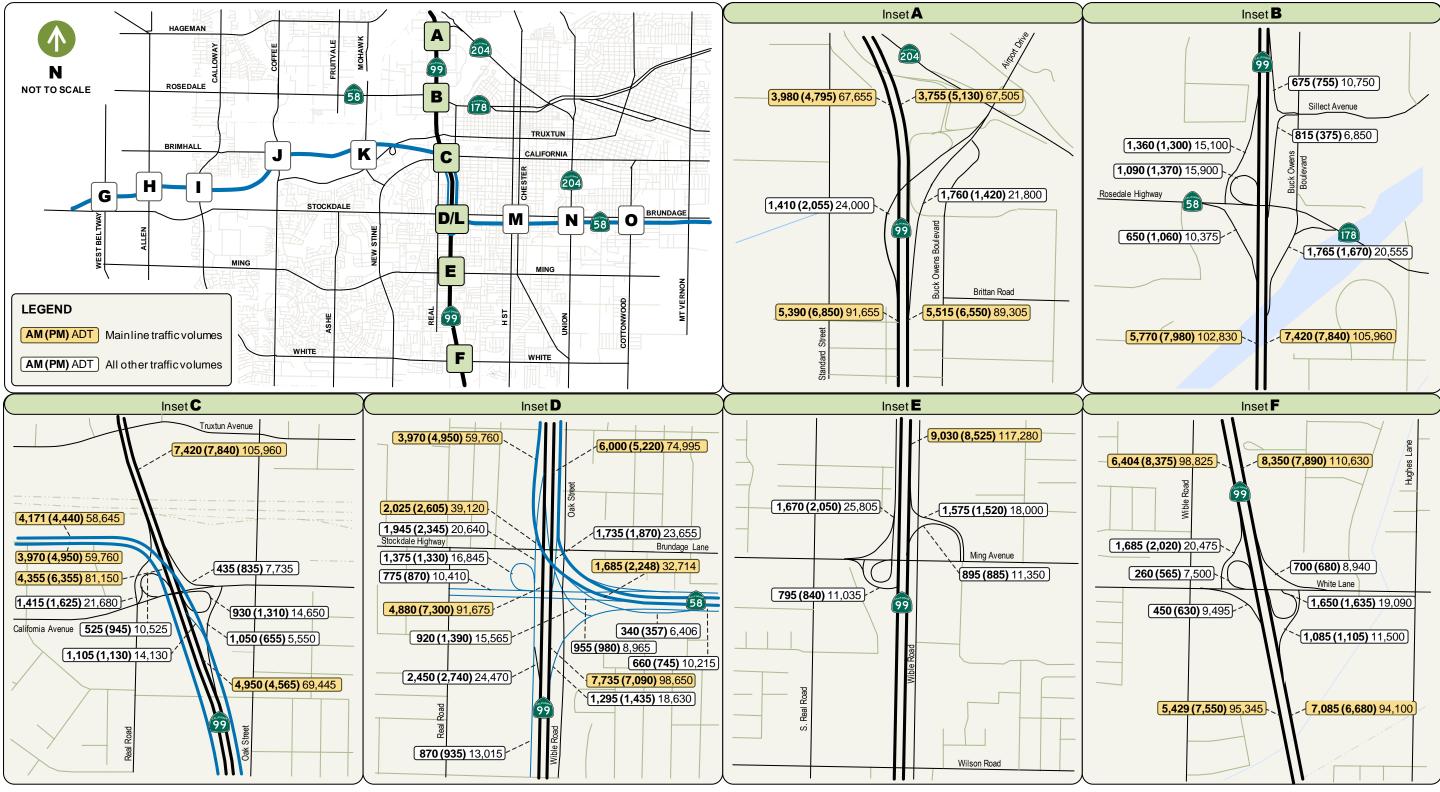
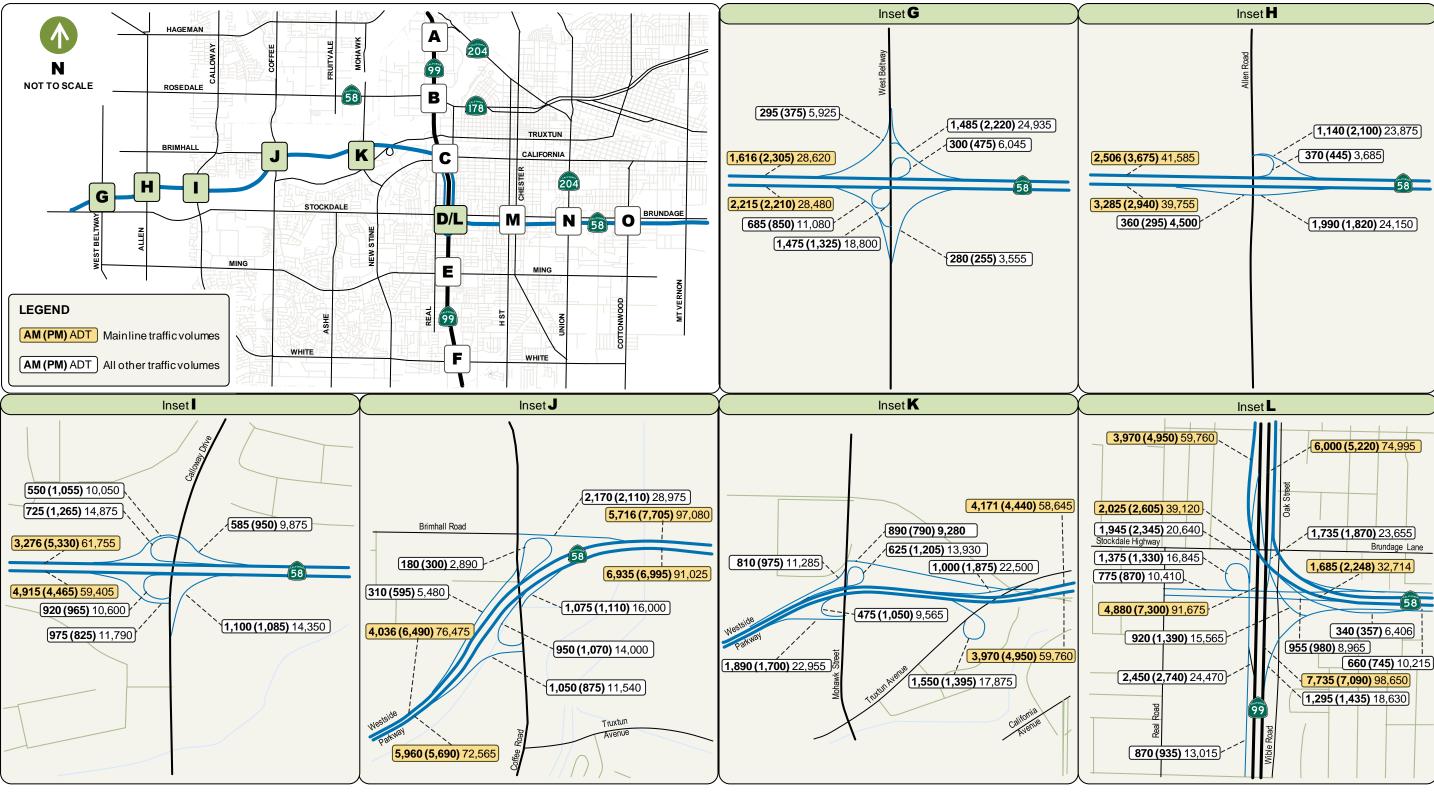


Figure 3-21: Difference between Year 2038 Alternative C and the No-Build Alternative Daily Traffic Volumes



Source: Parsons

Figure 3-22: Year 2038 Peak Hour and Daily Freeway Volumes—Alternative C



Source: Parsons

Figure 3-22: Year 2038 Peak Hour and Daily Freeway Volumes—Alternative C (2 of 3)



Inset ©

375 (585) 6,535

200 (275) 2,900

5,420 (5,455) 70,635

4,725 (6,375) 76,721

645 (1,060) 11,310

Brundage Lane

375 (585) 6,535

375 (585) 6,535

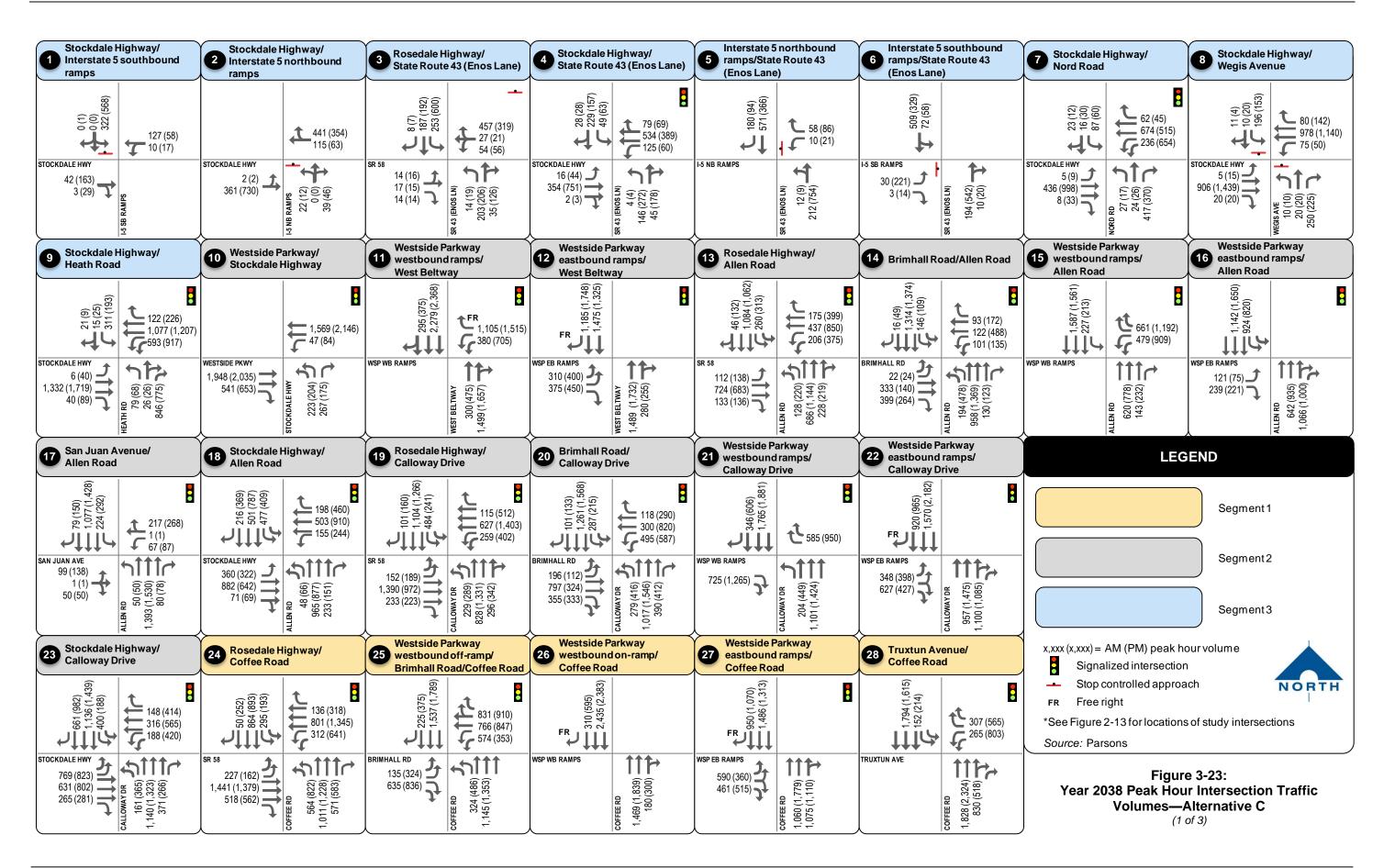
4,390 (5,590) 68,501

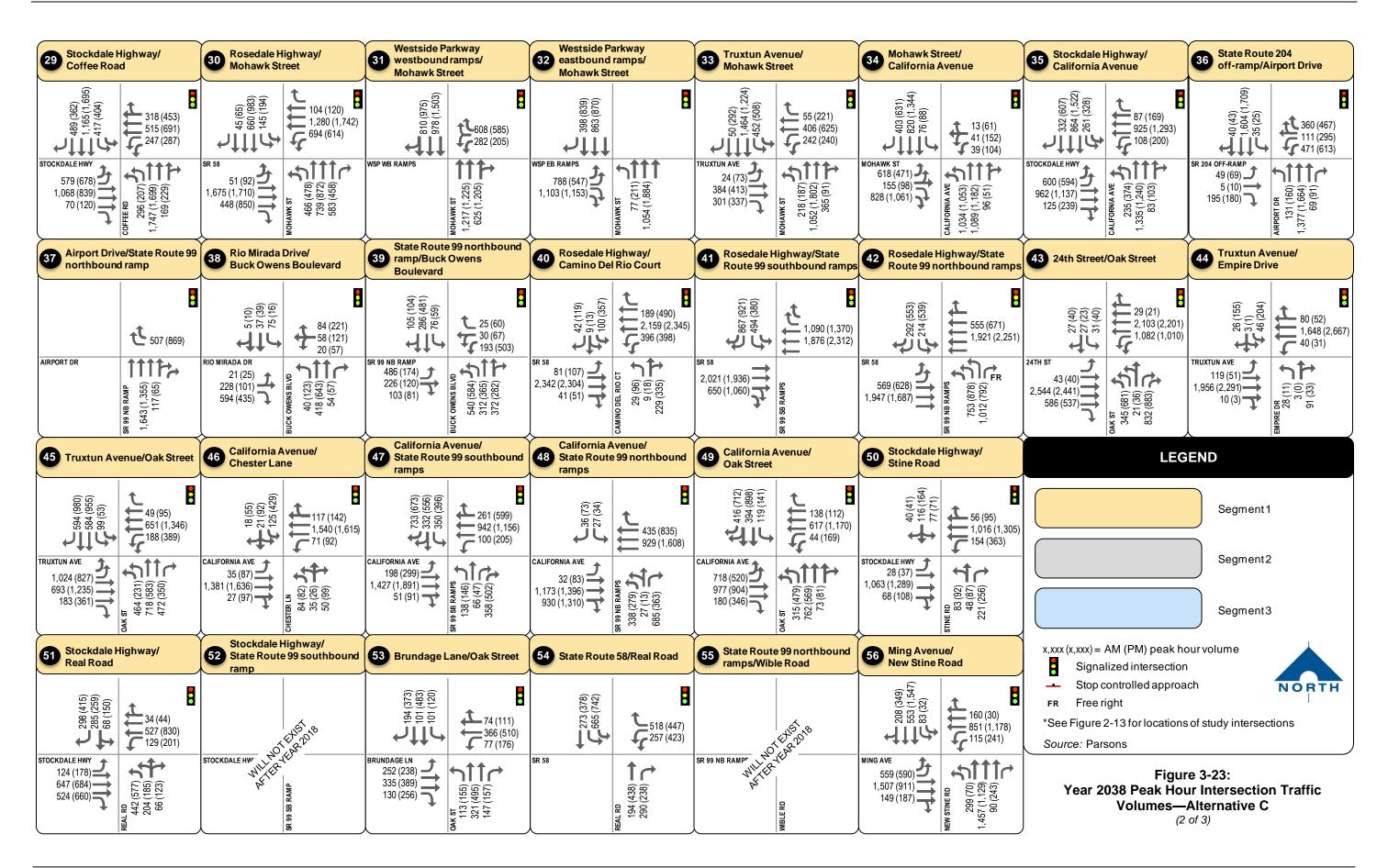
Brundage Lane

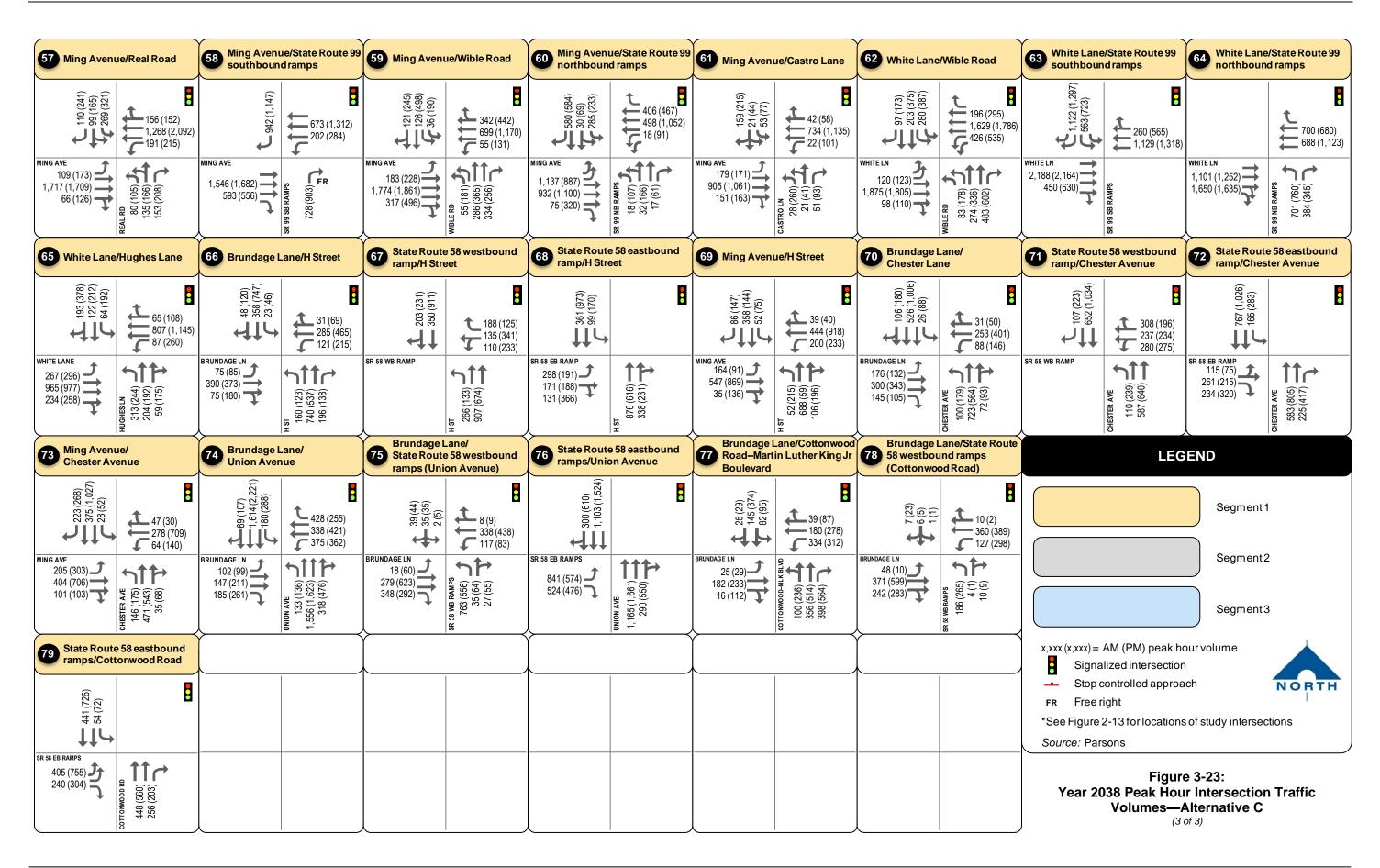
375 (585) 6,535

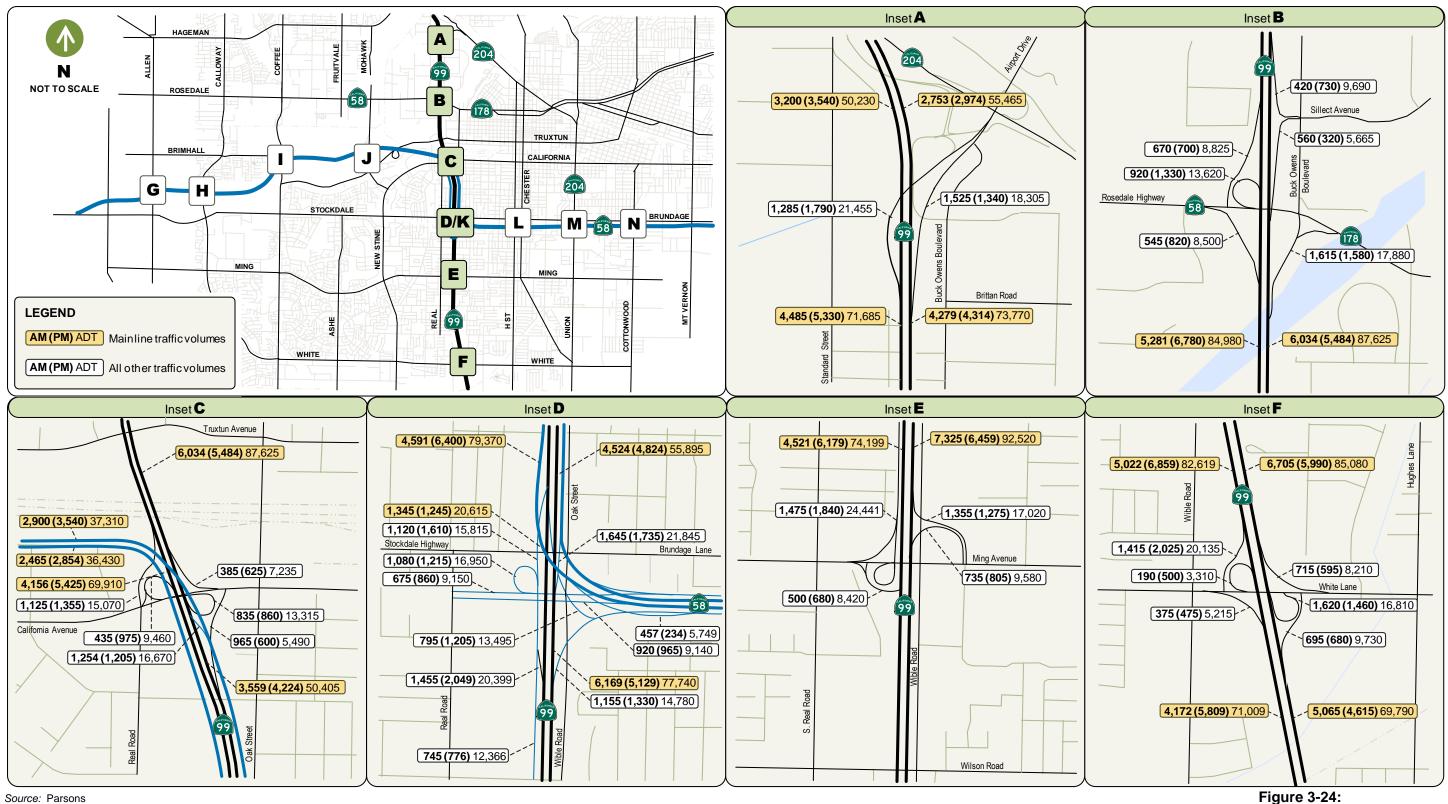
310 (275) 3,090

Figure 3-22: Year 2038 Peak Hour and Daily Freeway Volumes—Alternative C









Year 2018 Peak Hour and Daily Freeway
Volumes—Alternative C

(1 of 3)

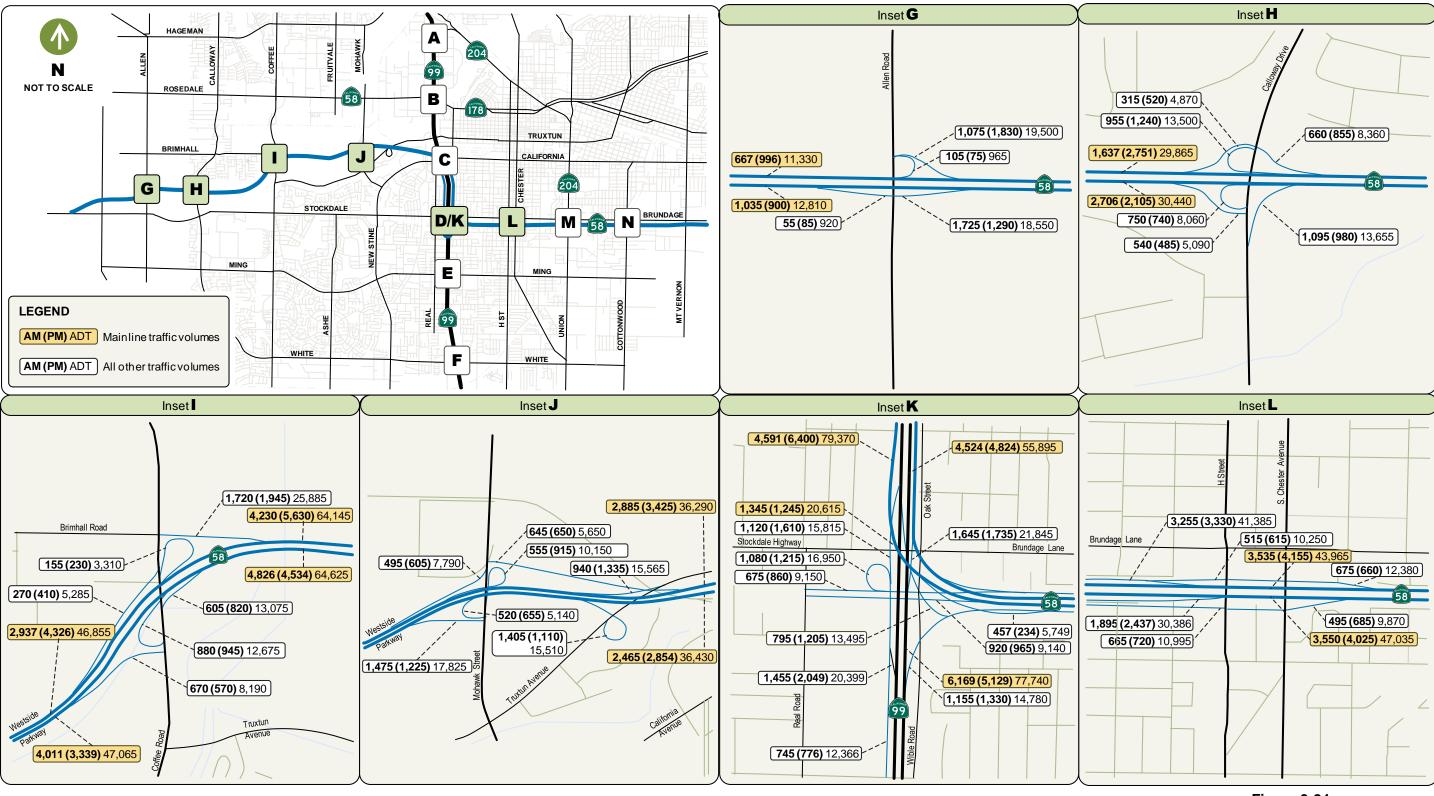


Figure 3-24: Year 2018 Peak Hour and Daily Freeway Volumes—Alternative C (2 of 3)

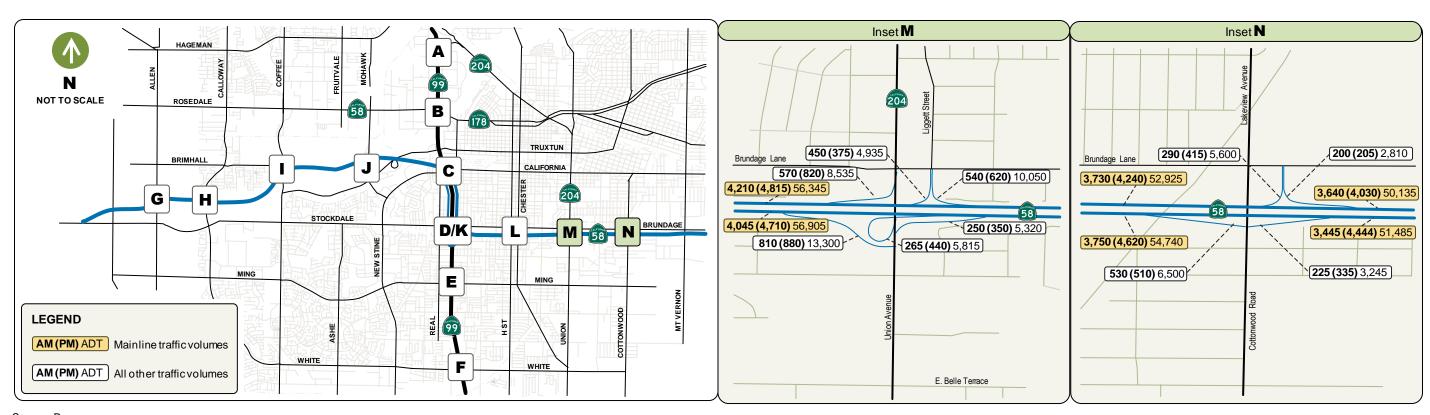
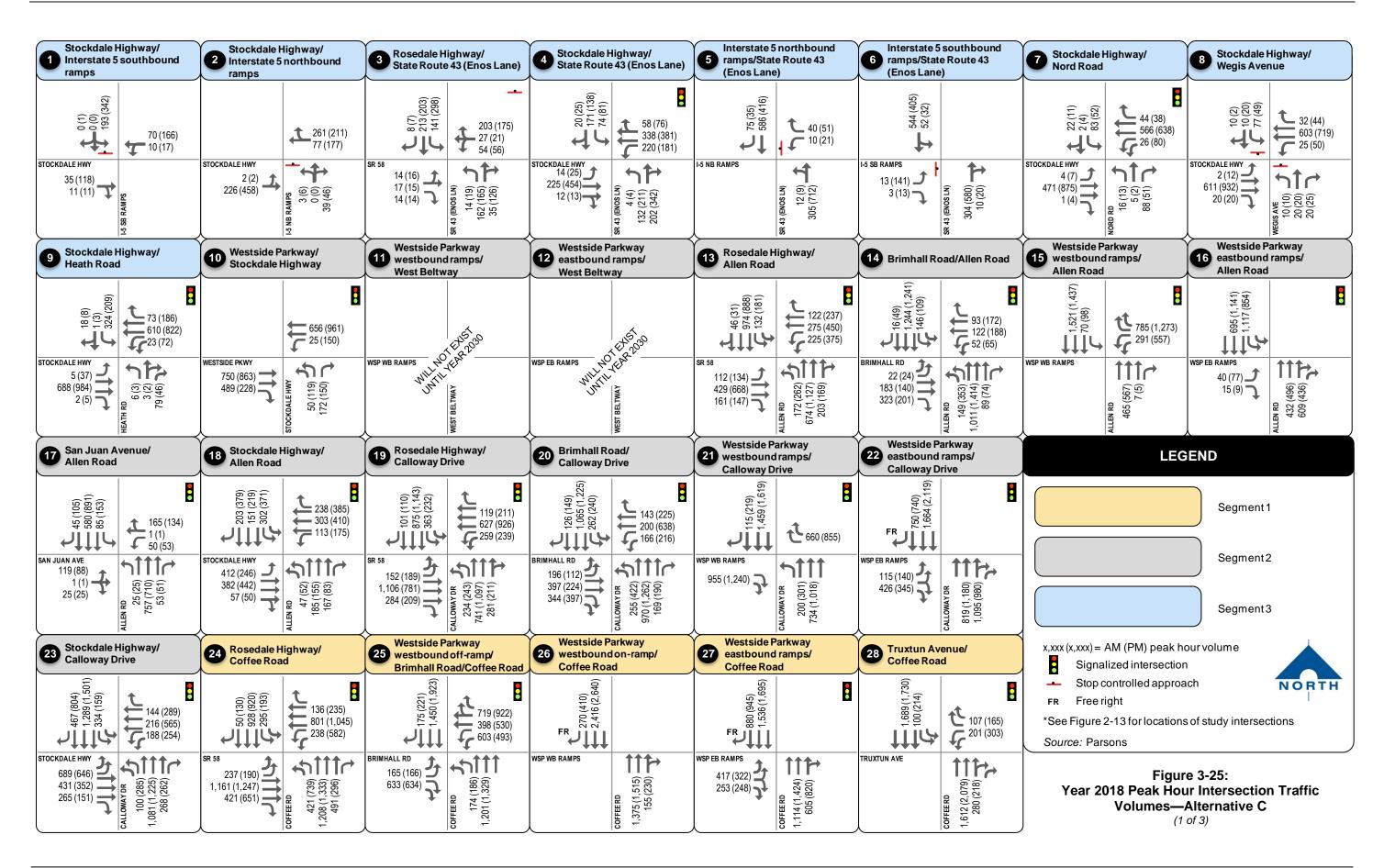
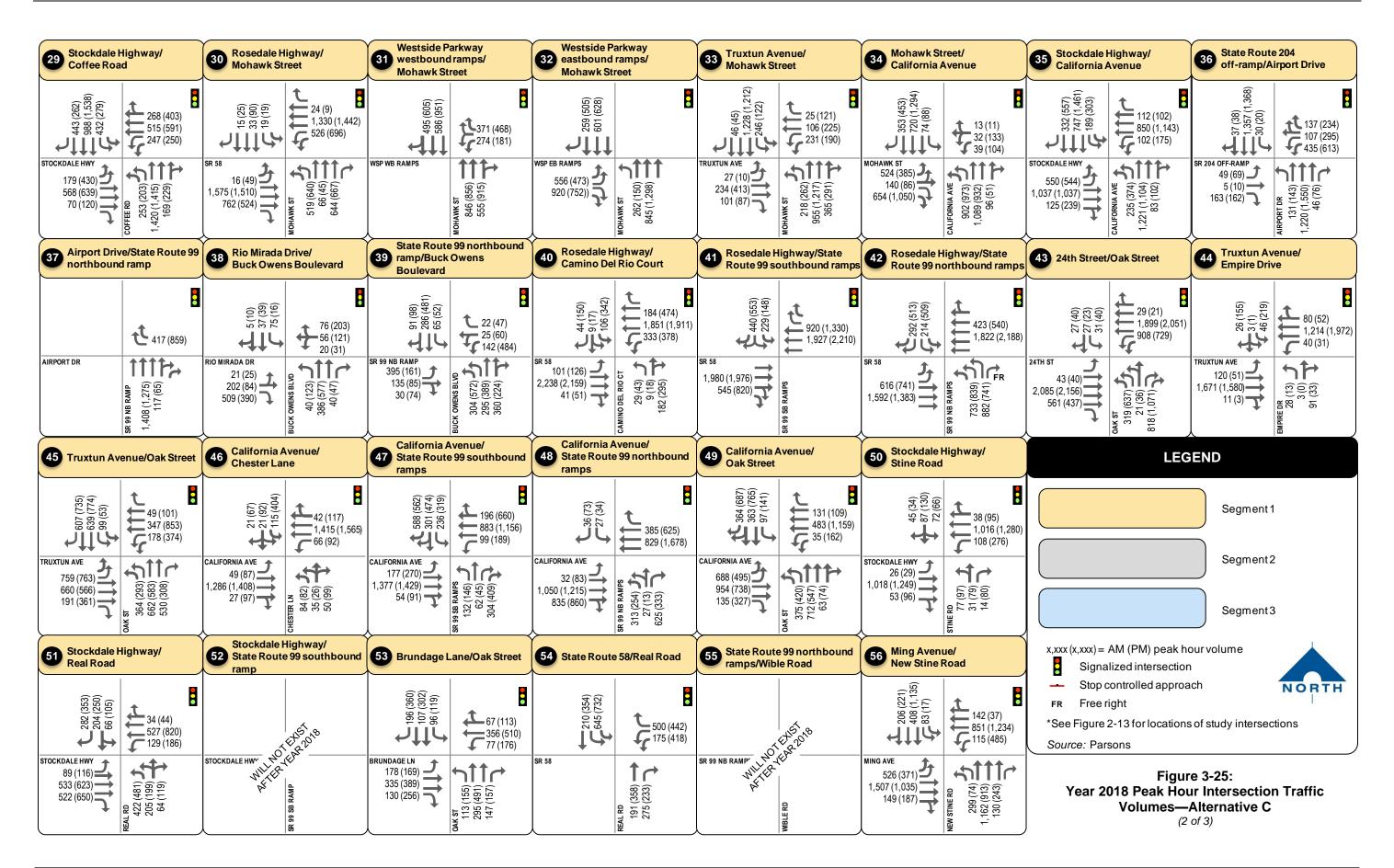


Figure 3-24: Year 2018 Peak Hour and Daily Freeway Volumes—Alternative C (3 of 3)





57 Ming Avenue/Real Road	Ming Avenue/State Route 99 southbound ramps	59 Ming Avenue/Wible Road	Ming Avenue/State Route 99 northbound ramps	61 Ming Avenue/Castro Lane	62 White Lane/Wible Road	63 White Lane/State Route 99 southbound ramps	64 White Lane/State Route 99 northbound ramps
(4/17) 66 (25) (1/17) 66 (25) (1/17) 66 (27) (1/17) 66 (27) (1/17	802 (1,417) 169 (231)	36 (408) 36 (190) 36 (130) 36 (190) 36 (131)	298 (295) 672 (1,113) 18 (96)	42 (55) 821 (1,027) 22 (101)	166 (262) 1,269 (1,501) 366 (424)	190 (500) 812 (889)	715 (595) 569 (996)
REAL RD (122) 1926 (102) 28 (105) 29 (111) 29 (105) 29 (1	MING AVE 1,546 (1,582) 381 (449) 88 88 88 88 88 88 88 88	157 (216) 1,831 (1,811) 247 (487)	1,026 (829) 1,091 (1,156) 75 (280) 1,091 (2,10) 1,091 (2,10) 1,00 (2,10) 1,00 (2,10) 1,00 (2,10) 1,00 (2,10) 1,00 (2,10) 1,00 (2,10) 1,	CASTRO LN 28 (260) 24 (46) 24 (46) 24 (46) 24 (46) 24 (46) 24 (46)	MIBLE RD 73 (168) 88 (25) 273 (326) 459 (515)	WHITE LN 2,057 (1,922) 375 (475) 88 88 88	88 3 (1,188) 1,620 (1,460) 2 562 (287)
65 White Lane/Hughes Lane	66 Brundage Lane/H Street	67 State Route 58 westbound ramp/H Street	State Route 58 eastbound ramp/H Street	69 Ming Avenue/H Street	70 Brundage Lane/ Chester Lane	State Route 58 westbound ramp/Chester Avenue	State Route 58 eastbound ramp/Chester Avenue
50 (103) 757 (998) 73 (139)	(0,1,1,1,1,1,1,1,1,1,1,1,1,1,1,1,1,1,1,1	(122 (280) 110 (221)	420 (968) 76 (152)	(SOL) 06 (F) 150 (SOL) 06 (F) 06 (F) 150 (SOL) 06 (F) 06 (F) 150 (SOL) 06 (F)	(471) 78 (471) 80 (113) (471) 80 (113) (471) 80 (113)	276 (191) 191 (213) 208 (256)	605 (974)
HUGHES LN 321 (234) 192 (183) 28 (94) 28 (94)	BRUNDAGE LN 75 (75) 389 (373) 107 (170) The state of the	\$8 WB RAMP \$701 (174) \$100 (1754) \$100 (17	SR 58 EB RAMP 305 (191) 181 (178) 179 (351) 179 (351)	MING AVE 149 (101) 536 (710) 38 (136) 7 (62) 844 (52) 8 84 (52) 84 (BRUNDAGE LN 176 (135) 310 (343) 111 (92) 111 (92) 111 (92) 111 (92) 111 (92) 111 (92) 111 (92) 111 (92)	CHESTER AVE 110 (163) 547 (529)	CHESTER AWE 104 (92) 193 (527) 196 (518) 196 (518) 174 (225) 174 (
73 Ming Avenue/ Chester Avenue	74 Brundage Lane/ Union Avenue	Brundage Lane/ State Route 58 westbound ramps (Union Avenue)	State Route 58 eastbound ramps/Union Avenue	Brundage Lane/Cottonwood Road-Martin Luther King Jr Boulevard	78 Brundage Lane/State Route 58 westbound ramps (Cottonwood Road)	LEG	END
47 (25) 216 (649) 62 (137)	(10) 09 (11) 240 (205) 273 (409) 326 (312)	(4) 306 (368) (4) 306 (368) (5) 2 (6) 306 (368) (7) 113 (79)	265 (440)	(807) 178 (268) 178 (268) 231 (231)	10 (2) 260 (373) 113 (220)		Segment 1
MING AVE 217 (187) 373 (528)	136 (109) 1	BRUNDAGE LN 18 (60)	SR 58 EB RAMPS	BRUNDAGE LN 25 (29)	BRUNDAGE LN 48 (10)		Segment 2
CHESTER AVE 38 (143) 35 (56)	11.260 (942) 273 (386) 223 (386) 242	SR 58 WB RAMPS 494 (514) 334 (60) 12 (46) 12 (46)	UNION AVE 1, 104 (941) 250 (350)	123 (502) 123 (202) 123 (202) 123 (202) 134 (202) 136 (308)	175 (190) 11 (4) 11 (4) 11 (1)		Segment3
79 State Route 58 eastbound ramps/Cottonwood Road					100	x,xxx (x,xxx) = AM (PM) peak ho Signalized intersection	
304 (475)						 Stop controlled approa FR Free right *See Figure 2-13 for locations Source: Parsons 	
327 (245) 342 (485) 203 (265) 342 (486) 642 (167) 672						Volumes—	r Intersection Traffic Alternative C of 3)

Transportation Systems Management/Transit Alternative (Alternative M)

Alternative M proposes to utilize transportation systems management strategies to improve capacity and potentially reduce demand on the east—west arterial links and selected north—south connections to facilitate regional and local east—west traffic.

Low-cost improvements include traffic signal optimization, intersection widening, and bus turnouts to reduce delay and increase the capacity along the following corridors:

- Hageman Road from Calloway Drive to State Route 99
- Rosedale Highway from Enos Lane (State Route 43) to State Route 99
- Truxtun Avenue from the Westside Parkway to Oak Street
- Stockdale Highway from Old River Road to Oak Street/Wible Road
- Ming Avenue from Old River Road to State Route 99
- Coffee Road from Stockdale Highway to Westside Parkway
- California Avenue from Stockdale Highway to Mohawk Street
- Mohawk Street from California Avenue to the Westside Parkway

These low-cost improvements are also assumed for the no-build alternative, and are therefore not specifically studied as a new component of the transportation system for alternative M.

To further increase capacity, higher cost improvements are proposed for Rosedale Highway. These include widening Rosedale Highway from Enos Lane (State Route 43) to State Route 99 and constructing grade separations at the following intersections along Rosedale Highway:

- Rosedale Highway/Allen Road
- Rosedale Highway/Coffee Road
- Rosedale Highway/Calloway Drive
- Rosedale Highway/Mohawk Street
- Rosedale Highway/San Joaquin Valley Railroad crossing

Additionally, improvements on State Route 99 to be considered include adding auxiliary lanes between State Route 58 and California Avenue or providing an additional (fifth) mainline lane to accommodate the increased north—south traffic demand.

Roadway operational improvements would include deploying intelligent transportation systems strategies to improve mobility and reduce fuel consumption and green house gas emissions.

This alternative would also include increasing transit service along Rosedale Highway and Stockdale Highway to reduce the overall vehicular demand in these east—west corridors and other potential travel demand management techniques. The transit improvements would primarily focus on an increase in frequency of service that would result in reduced auto demand. Finally, area-wide transportation demand management strategies would be considered to reduce



travel demand through establishing parking fees, encouraging carpool formation, and encouraging flextime for employees.

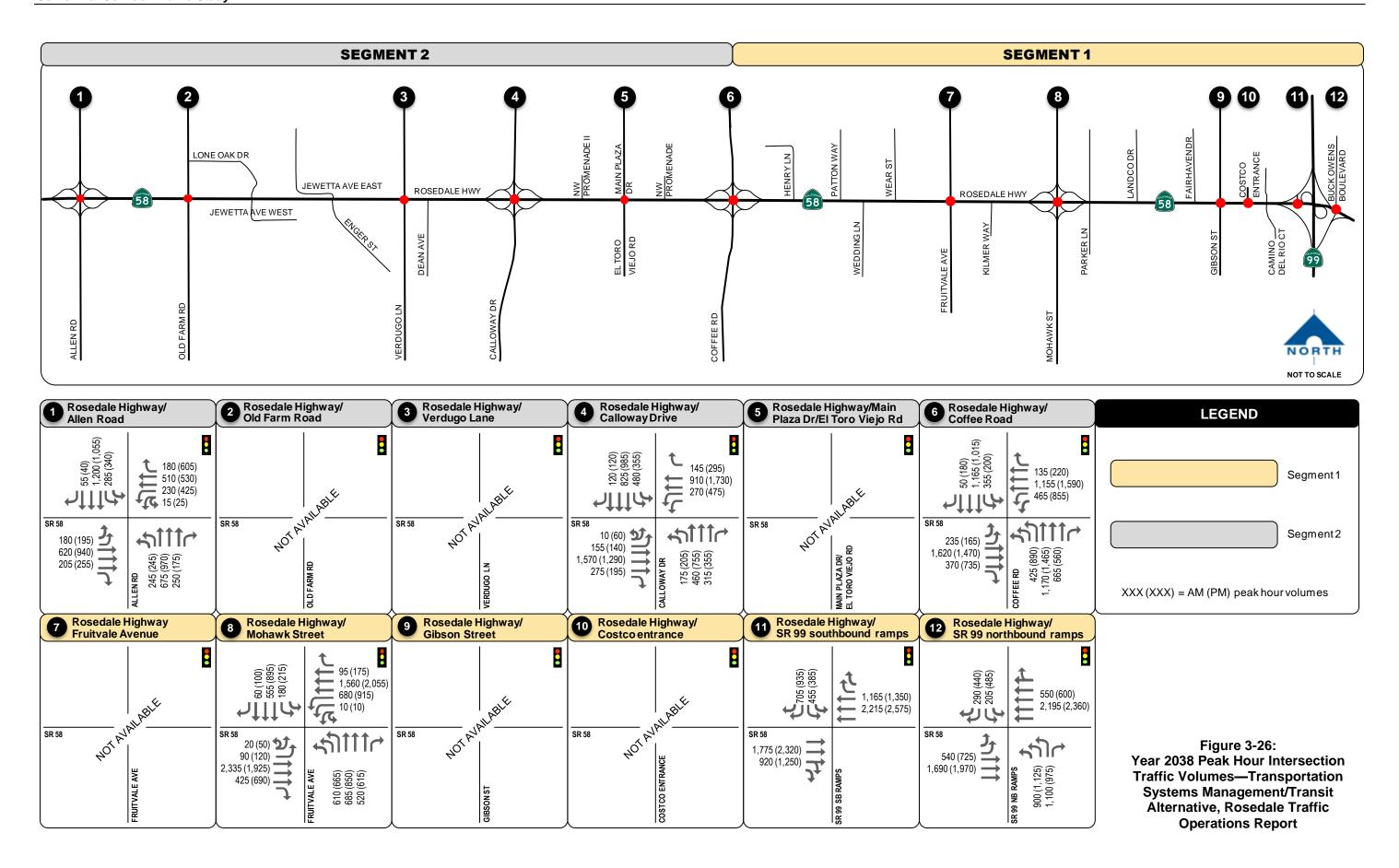
These improvements to transit service and implementation of travel demand management techniques are also assumed for the no-build alternative, and are therefore not specifically studied as a new component for alternative M.

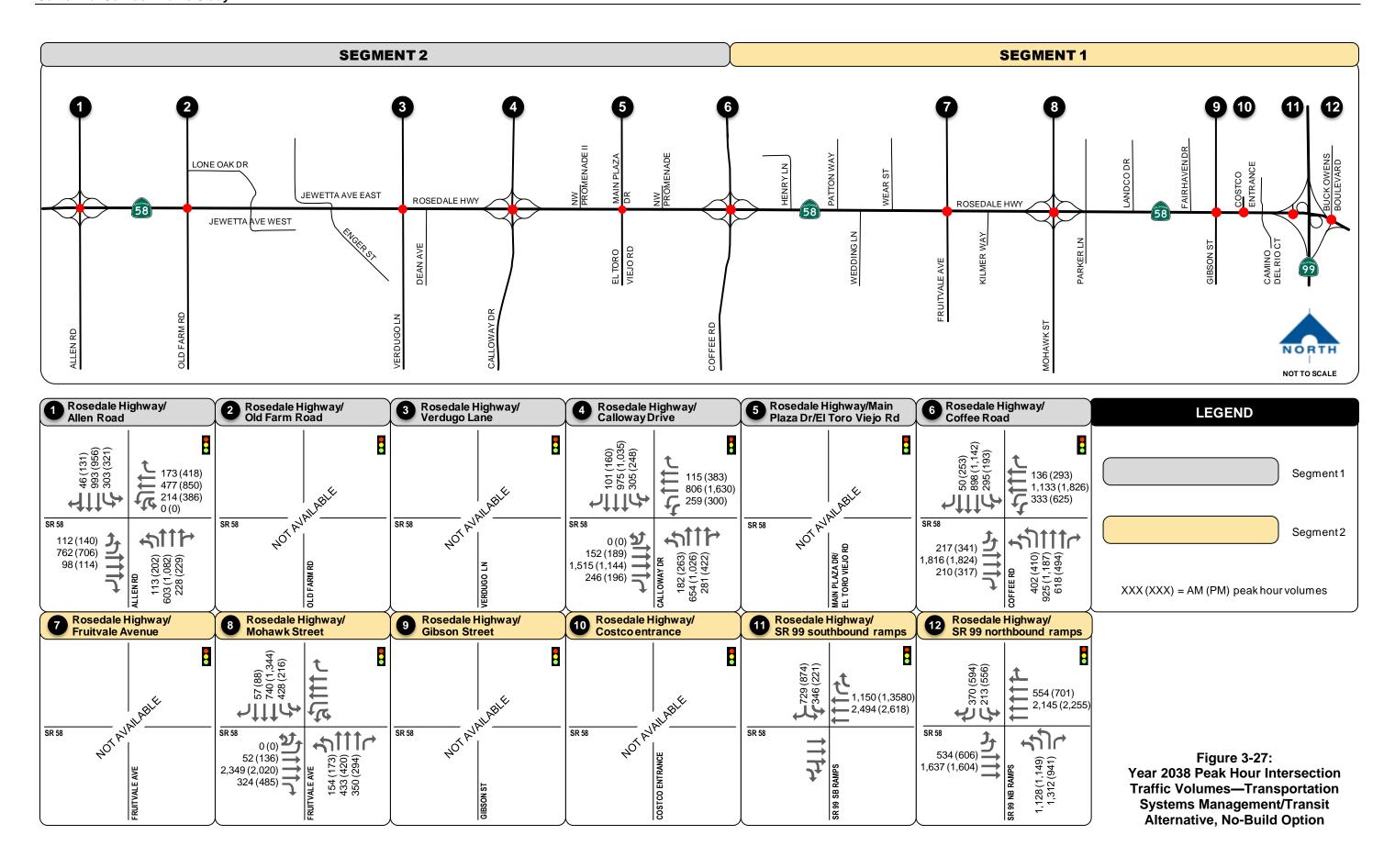
Figure 3-26 illustrates the future year forecasts of peak hour traffic volumes and lane geometry assumptions reported for the Rosedale Highway Widening Project. This project assumes that the Centennial Corridor project build alternative A, B, or C is constructed. Figure 3-27 illustrates future year forecasts of peak hour traffic volumes and lane geometry assumptions for the no-build alternative. The lane geometry is the same as defined for the Rosedale Highway Widening project; however, traffic volumes are different as motorists select different route choices to traverse the roadway network, seeking to minimize travel times by avoiding congestion. Figure 3-28 illustrates future year forecasts of peak hour traffic volumes and lane geometry assumptions for the transportation systems management/transit alternative. For reference, alternative M would increase capacity and reduce delays along Rosedale Highway by constructing grade separations over major north—south arterial streets which cross Rosedale Highway between State Route 99 and Allen Road. In addition, the number of signalized intersections along Rosedale Highway would be decreased. Figure 3-28 indicates two lane geometry options for the transportation systems management/transit alternative—a six-lane option and an eight-lane option. These options are discussed at length in Chapter 4.

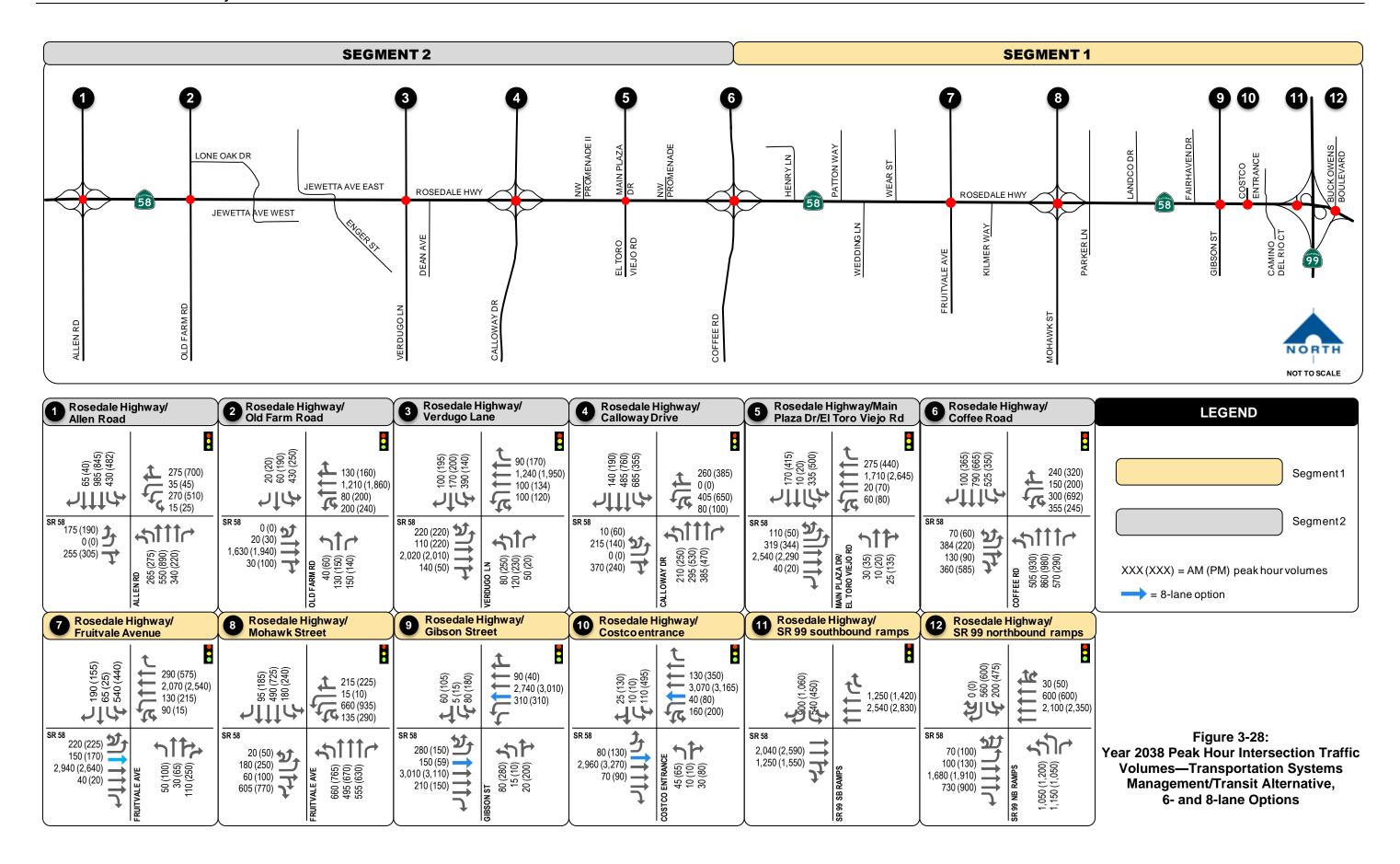
To provide an overall indication of the impact of upgrading Rosedale Highway (State Route 58 west) to a "super-arterial" between State Route 99 and Allen Road, a computer generated map is provided as Figure 3-29, which illustrates the difference between the 2038 transportation systems management/transit alternative and no-build daily traffic volumes, as assigned by the travel forecast model. The bandwidths illustrated in red depict roadways which receive additional volumes of traffic as a result of grade separating major north—south arterials crossing Rosedale Highway and reducing the number of signalized intersections along the facility. Roadways having bandwidths illustrated in blue indicate roads receiving less traffic as a result of improving State Route 58 west. The difference plot clearly indicates that the transportation systems management/transit alternative attracts more traffic volume to Rosedale Highway, while reducing traffic on parallel roadways.

The transportation systems management/transit alternative (alternative M) is not analyzed for year 2018 conditions; hence, no traffic volume forecasts are reported for that scenario.

¹ Projects 11 and 12 illustrated on Figure 3-4.







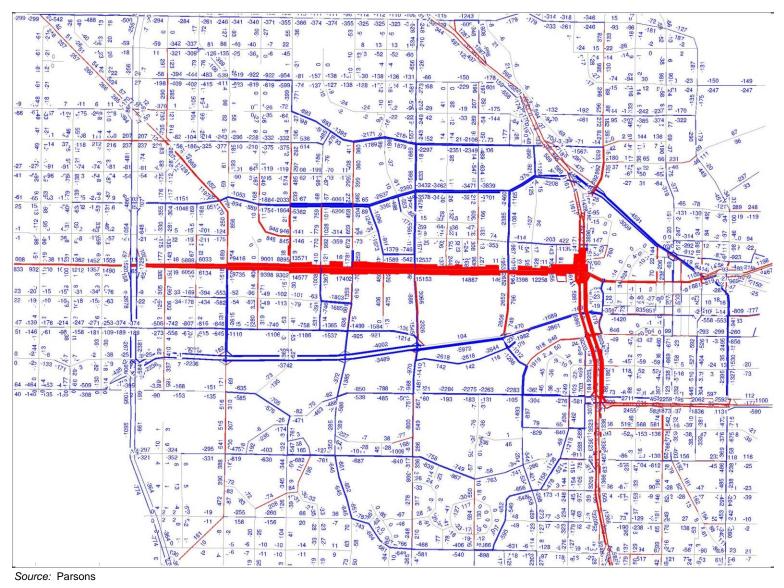


Figure 3-29: Difference between Year 2038 Transportation Systems Management/Transit Alternative and the No-Build Alternative Daily Traffic Volume Difference

3.5 Future Year Truck Volumes

The Kern Council of Governments 2006 Regional Travel Demand Model forecasts three components of trucking activity.

- Internal—internal trips, having both an origin and destination within Kern County, are estimated based on employment type and employment distribution. Trip "production" rates are listed in Table 3-4, while trip "attraction" rates are listed in Table 3-5. The estimated rates are based on the number of employees per type of employment (basic, basic warehousing, retail high, retail medium, service office, and service commercial). The truck trip rates reported in these tables are based on axle types and are estimated as vehicle trips expressed as passenger car equivalents.
- Internal—external trips are based on the 2001 Caltrans Statewide Model. A separate trip purpose is provided for trucks; however, the model does not subdivide the truck trips by type of truck. Truck trips are expressed as passenger car equivalents.
- External—external vehicle trips are forecast using the 2001 Caltrans Statewide Model. These external—external vehicle trips are origin—destination based and are added to the internal Kern county trips prior to trip assignment, after factoring for each time-of-day period. As with the internal—external trips, there is no separate accounting of truck trips as they are included within the whole.

Table 3-4. Internal Truck Trip Production Model

TYPE OF TRUCK	BASIC PRODUCTION (BASIC_EMP)	BASIC WARE- HOUSING (BWOTH_EMP)	RETAIL HIGH (RHRET_EMP)	RETAIL MEDIUM (RMRET_EMP)	SERVICE OFFICE (SOSER_EMP)	SERVICE COMMERCIAL (SCSER_EMP)
2-axle	0.0554	0.0610	0.0464	0.0464	0.0610	0.0610
3-axle	0.0059	0.0058	0.0040	0.0040	0.0058	0.0058
4- or more axle	0.0123	0.0129	0.0073	0.0073	0.0129	0.0129

Table 3-5. Internal Truck Trip Attraction Model

TYPE OF TRUCK	BASIC PRODUCTION (BASIC_EMP)	BASIC WARE- HOUSING (BWOTH_EMP)	RETAIL HIGH (RHRET_EMP)	RETAIL MEDIUM (RMRET_EMP)	SERVICE OFFICE (SOSER_EMP)	SERVICE COMMERCIAL (SCSER_EMP)
2-axle	0.0502	0.0502	0.0502	0.0502	0.0502	0.0502
3-axle	0.0051	0.0051	0.0051	0.0051	0.0051	0.0051
4- or more axle	0.0188	0.0188	0.0188	0.0188	0.0188	0.0188

Given the limitations of the Kern COG 2006 Regional Travel Demand Model insofar as forecasting truck trips, Parsons used the San Joaquin Valley Truck Model to aid in establishing future year truck volumes. Parsons used the Kern COG model to generally confirm overall traffic volumes and travel patterns for the 2006 model validation year and the 2018 and 2038 forecast years under consideration.

The San Joaquin Valley Truck Model was developed by Cambridge Systematics using a combination of commodity-based and vehicle-based modeling techniques. The study area

included eight counties in the San Joaquin region: San Joaquin, Stanislaus, Merced, Fresno, Madera, Tulare, Kings and Kern. The truck model was developed using the Caltrans Statewide Model road network and zone structure.

The Caltrans Intermodal Transportation Management System (ITMS) commodity flow data was used to obtain detailed origin-destination information about commodity flows in to, out of, and through the valley counties as the basis for estimating intercity freight movements. County-level ITMS commodity flow truck trip data were then allocated to zip codes using employment data from 2000. The truck trips were allocated to zip codes based on matching the commodity codes in the truck trip table with the employment categories in the database for each code and each zip code. For outbound flows, one-to-one correspondences were made between commodity codes in the two databases. For inbound flows, tons were allocated based on employment in the consuming industries for each commodity.

The zip code-level trips were then allocated to the traffic analysis zones in the truck model. A truck trip table for "through" trips was created through visual observation of origin—destination combinations that crossed through the San Joaquin Valley. The truck tonnage for each of the relevant origin-destination combinations was collected from ITMS data. For each pair of relevant external regions, all of the truck tons in associated counties were summed. These tonnage values were converted into truck trips using the same payload matrix used for the other truck trip tables.

Trip production rates were developed primarily from the *Quick Response Freight Manual*, which provides trip rates based on national averages for medium and heavy trucks. Truck trip consumption rates were developed to estimate the relative number of trucks that are attracted to each zone in the valley. The tables for the State of California were then disaggregated to represent truck trip rates for medium and heavy truck trips.

The truck model is designed to generate truck volumes based on average daily traffic. The truck model output reports truck volumes based on truck classes defined as medium–heavy duty, which have a gross vehicle weight rating between 14,001 and 33,000 pounds; and heavy–heavy duty, with a gross vehicle weight rating of 33,001 pounds or more.

The ITMS database provides commodity flow forecasts for 2006, 2016, and 2026, which are derived using economic activity trends and forecasts from Global Insight, Inc., and county-level forecasts from the California Transportation Economics Unit. The first step in the process was the estimation of annual growth rates from ITMS forecasts for the time periods of (1) 2000 to 2006, (2) 2006 to 2016, and (3) 2016 to 2026. Year 2030 forecasts were then extrapolated based on the 2016 to 2026 annual average growth factors. As these growth factors were based on pre-2007 economic recession trends, the year 2030 forecasts were assumed for the 2038 Centennial Corridor project planning horizon. Table 3-6 shows the growth rates by commodity type, while Tables 3-7 and 3-8 show the total productions and consumptions in the non-ITMS trip generation procedure. Growth in population and employment leads to an increased need for commodities, which results in increased productions and consumptions. There is an overall growth of more than 70 percent for both productions and consumptions. This is in line with the growth in households and employment. Table 3-9 shows the growth in ITMS truck trips. ITMS trips grow at a higher rate than the local trips, because the valley is a big generator and consumer of goods for California and the rest of the nation.

Table 3-6. Intermodal Transportation Systems Management Growth Rates by Commodity Type

		AVERAGE	ANNUAL GROW	TH FACTOR
STCC2*	COMMODITY DESCRIPTION	2000–2006	2006–2016	2016–2030
01	Farm products	1.0358	1.0320	1.0262
08	Forest products	1.0405	1.0333	1.0166
09	Fresh fish and marine products	1.0612	1.0504	1.0250
10	Metallic ores	1.0612	1.0504	1.0250
11	Coal	1.0917	1.0519	1.0133
13	Crude petroleum and natural gas	1.0456	1.0376	1.0187
14	Nonmetallic minerals	1.0354	1.0252	1.0173
20	Food products	1.0433	1.0405	1.0328
21	Tobacco products	1.0457	1.0501	1.0280
22	Textile mill products	1.0553	1.0389	1.0257
23	Apparel	1.0732	1.0411	1.0282
24	Lumber products	1.0386	1.0324	1.0246
25	Furniture	1.0527	1.0470	1.0372
26	Pulp and paper products	1.0393	1.0341	1.0250
27	Printed matter	1.0430	1.0380	1.0297
28	Chemical products	1.0436	1.0359	1.0298
29	Petroleum and coal products	1.0451	1.0343	1.0245
30	Rubber and miscellaneous plastics	1.0535	1.0426	1.0313
31	Leather products	1.0505	1.0414	1.0273
32	Clay, concrete, glass or stone products	1.0612	1.0504	1.0250
33	Primary metal products	1.0498	1.0390	1.0270
34	Fabricated metal products	1.0443	1.0371	1.0272
35	Machinery	1.0639	1.0486	1.0402
36	Electrical equipment	1.0780	1.0543	1.0430
37	Transportation equipment	1.0405	1.0340	1.0290
38	Instruments, photo and optical equipment	1.0740	1.0654	1.0525
39	Miscellaneous manufacturing products	1.0528	1.0437	1.0399
40	Waste and scrap materials	1.0557	1.0467	1.0275
50	Secondary traffic	1.0656	1.0399	1.0285

^{*}Two-digit standard transportation commodity classification

Source: Cambridge Systematics, Inc., San Joaquin Valley Goods Movement Study Truck Model Documentation

Table 3-7. Growth in Non-Intermodal Transportation Systems Management Productions

	PRODUCTIO	N TRIPS 2000	PRODUCTIO	N TRIPS 2030	GRO	WTH
EMPLOYMENT CATEGORY	MEDIUM	HEAVY	MEDIUM	HEAVY	MEDIUM	HEAVY
Agriculture	4,993	7,622	8,797	13,430	76%	76%
Mining	166	364	346	762	109%	109%
Construction	262	1,436	427	2,336	63%	63%
Manufacturing—products	28	4,304	46	7,125	66%	66%
Manufacturing—equipment	20	1,127	33	1,838	63%	63%
TCU	1,741	6,656	2,895	11,070	66%	66%
Wholesale	610	2,063	954	3,228	56%	56%
Retail trade	13,092	1,963	23,106	3,463	76%	76%
FIRES	1,557	362	2,797	650	80%	80%
Education/government	1,576	236	2,920	437	85%	85%
Households	34,572	74,420	60,721	130,712	76%	76%
Total	58,616	100,554	103,043	175,049	76%	74%

Source: Cambridge Systematics, Inc., San Joaquin Valley Goods Movement Study Truck Model Documentation



Table 3-8. Growth in Non-Intermodal Transportation Systems Management Consumptions

EMPLOYMENT	2000 CONS	UMPTIONS	2030 CONS	UMPTIONS	GRO	WTH
CATEGORY	MEDIUM	HEAVY	MEDIUM	HEAVY	MEDIUM	HEAVY
Agriculture	2,911	2,221	5,130	3,914	76%	76%
Mining	97	5,550	202	11,599	109%	109%
Construction	190	1,919	308	3,120	63%	63%
Manufacturing—products	29	3,539	48	5,859	66%	66%
Manufacturing—equipment	21	2,582	35	4,210	63%	63%
TCU	737	6,859	1,226	11,407	66%	66%
Wholesale	62	498	97	780	56%	56%
Retail trade	3,817	408	6,736	720	76%	76%
FIRES	9,045	2,195	16,248	3,943	80%	80%
Education/government	920	712	1,704	1,319	85%	85%
Households	40,354	82,633	70,878	145,135	76%	76%
Total	58,183	109,116	102,613	192,006	76%	76%

Source: Cambridge Systematics, Inc., San Joaquin Valley Goods Movement Study Truck Model Documentation

Table 3-9. Growth in Intermodal Transportation Systems Management Trips

EMPLOYMENT	2000	ITMS	2030	ITMS	GROWTH		
CATEGORY	MEDIUM	HEAVY	MEDIUM	HEAVY	MEDIUM	HEAVY	
Exports	2,153	17,742	5,708	43,484	165%	145%	
Imports	2,153	17,742	5,708	43,484	165%	145%	
Internal trips	12,600	17,235	17,153	39,403	36%	129%	
Through trips	1,326	9,516	3,226	21,815	143%	129%	
Total	18,231	62,234	31,796	148,145	74%	138%	

Source: Cambridge Systematics, Inc., San Joaquin Valley Goods Movement Study Truck Model Documentation

Table 3-10 shows the growth in truck traffic by county based on the estimated growth in employment and population. As shown in Table 3-10, the model predicts that medium-heavy truck traffic in Kern County will increase by 48 percent, while heavy—heavy truck traffic will increase by 87 percent. One notes that the San Joaquin Valley truck model indicates that the average increase in the category of medium trucks for the San Joaquin Valley is 91 percent.

Table 3-10. Growth in Truck Traffic by County

	NUMBER OF	2000 N	MODEL	2030 1	MODEL	PERCENT D	IFFERENCE
COUNTY	COUNTS	MEDIUM	HEAVY	MEDIUM	HEAVY	MEDIUM	HEAVY
Fresno	58	17,896	83,630	35,489	168,382	98%	101%
Kern	10	30,143	106,756	44,762	199,110	48%	87%
Kings	20	2,708	12,361	6,943	29,906	156%	142%
Madera	121	3,983	32,326	7,762	57,160	95%	77%
Merced	12	5,440	47,972	10,955	94,497	101%	97%
San Joaquin	18	29,039	195,691	61,927	415,107	113%	112%
Stanislaus	22	5,821	44,362	11,005	84,949	89%	91%
Tulare	220	6,157	41,676	14,073	89,682	129%	115%
Total		101,186	564,774	192,914	1,138,792	91%	102%

Source: Cambridge Systematics, Inc., San Joaquin Valley Goods Movement Study Truck Model Documentation

The medium—heavy truck traffic is assumed to correspond to the 2 to 3 axle classification of trucks used by Caltrans. For the most part, these trucks are assumed to make internal—internal trips. Heavy—heavy trucks are assumed to correspond to the 4 to 5+ axle classification of trucks used by Caltrans. A portion of these truck volumes are assumed to be external—external, while a portion are internal—external or external—internal.

Future Truck Volumes under the No-Build and Transportation Systems Management Alternatives

While the model does not identify "through-trips" as a percentage of overall vehicle trips, the model's distribution of medium—heavy and heavy—heavy trucks provides a basis for making a reasonable estimate of the amount of increase of these trips. Table 3-14 indicates the volumes of trucks using different segments of State Route 58 west (Rosedale Highway), reported from east to west. Because there is a dramatic difference in truck traffic volumes between the count location east of Enos Lane/State Route 43 (1,118 total trucks) and west of Mohawk Street (5,327 total trucks), it is assumed that the vehicle counts east of Enos Lane/State Route 43 represent primarily "through-trips." According to this method, there are a total of 1,118 through-trip trucks; 481 of which are 2 to 3 axle trucks, and 637 are 4 to 5+ axle trucks.

Using these vehicle classification counts conducted along Rosedale Highway, one knows the breakdown of vehicles according to the 13 classifications established by the Federal Highway Administration. Parsons has used the 2 to 3 axle truck volumes to correspond to the mediumheavy trucks, most of which are assumed to be primarily local in their origins and destinations. These truck volumes are assumed to increase proportionally with the overall increase in traffic volumes using Rosedale Highway. The through-trip trucks that are 4 to 5+ axles are projected to increase by 87 percent over the study period.

Given the continuation of signalized intersections along Rosedale Highway under both the no-build and the transportation systems management/transit alternatives, this facility is not expected to draw or attract a significant number of truck trips from alternate routes, such as State Route 46, Lerdo Highway, 7th Standard Road, or State Route 119. Hence, the existing truck volumes along Rosedale Highway serve as an excellent benchmark for estimating future year truck volumes under both the no-build and transportation systems management/transit conditions.

Table 3-11 presents 2008 traffic count data along with the 2038 estimated volumes of trucks utilizing Rosedale Highway on weekdays.

Insofar as other east—west roadways connecting Interstate 5 with State Route 99, Table 3-12 reports existing heavy truck volumes on roadway links immediately east of Interstate 5 and those immediately west of State Route 99. Unlike State Route 58, heavy truck volumes both east of Interstate 5 and west of State Route 99 are very similar, indicating a likelihood that these truck movements are primarily through trips (external—external) in nature.

Projected heavy truck traffic using the existing roadway network is also shown in Table 3-12. In this case, the truck volumes are increased by 87 percent across the board—a conservative assumption.

Table 3-11. Comparison of State Route 58 West (Rosedale Highway) Year 2008 Truck Traffic Counts with Year 2038 Truck Traffic Projections

			2008 TR	AFFIC CO	UNT DAT	4		2038 FORECAST VOLUMES			
LOCATION TIME	ADT TOTAL	TOTAL TRUCKS	TOTAL TRUCK PERCENT	TOTAL 2-3 AXLE VOLUME	TOTAL 2-3 AXLE PERCENT	TOTAL 4-5+ AXLE VOLUME	4-5+ AXLE PERCENT OF ADT	TOTAL 2-3 AXLE VOLUME	TOTAL 4-5+ AXLE VOLUME		
West of Mohawk Street											
Weekday average	42,511	5,327	12.5%	2,554	6.0%	2,773	6.5%	3,780	4,352		
West of Calloway D	rive										
Weekday average	30,708	3,263	10.6%	1,164	3.8%	2,099	6.8%	1,723	3,355		
West of Allen Road											
Weekday average	16,425	1,803	11.0%	765	4.7%	1,038	6.3%	1,132	1,785		
East of Enos Lane	East of Enos Lane (State Route 43)										
Weekday average	7,105	1,118	15.7%	481	6.8%	637	9.0%	712	1,191		

Notes:

Table 3-12. Vehicle and Truck Traffic on Kern County East–West Roadways (2010 and 2038)

		EAST OF IN	TERSTATE	5	WEST OF STATE ROUTE 99				
	2010			2038		2038			
ROADWAY	TOTAL VOLUME	HEAVY TRUCK VOLUME*	PERCENT HEAVY TRUCKS	HEAVY TRUCK VOLUME*	TOTAL VOLUME	HEAVY TRUCK VOLUME*	PERCENT HEAVY TRUCKS	HEAVY TRUCK VOLUME*	
State Route 46	5,830	1,820	31.20%	3,403	6,710	1,976	29.40%	3,695	
Lerdo Highway	2,103	240	11.40%	449	11,455	263	2.30%	492	
7th Standard Road	2,597	418	16.10%	782	10,667	1,048	9.80%	1,960	
Stockdale Highway	3,649	179	4.90%	335	33,527	506	1.51%	946	
State Route 119	10,000	1,182	11.80%	2,210	10,572	935	8.80%	1,748	
Total Volumes	24,179	3,839	15.88%	7,179	72,931	4,728	12.12%	8,841	

^{*}Heavy trucks are defined as 4 to 5+ axles.

Future Truck Volumes under Build Alternatives and A, B and C

Using a function provided with the Cube software (Citilabs) in the Kern COG traffic model, Parsons conducted a select link analysis of truck traffic volumes using State Route 58. This tool allows the user to pick a single point on the roadway network and illustrate the origin and destination of the traffic that passes through that point. The select link is akin to a count station which identifies the origin and destination of every vehicle that passes through that location. Figure 3-30 illustrates how the select link tool operates as a count station.

^{1.} Weekday average of Monday, February 11, 2008 through Friday, February 15, 2008.

^{2.} Through truck traffic with 4-5+ axles increases by 87 percent. Other truck traffic increases by 48 percent over this time period.

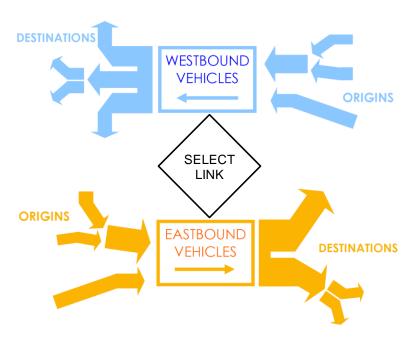


Figure 3-30: How Select Link Analysis Works

For this analysis, a point on State Route 58 west of the State Route 223 intersection and east of State Route 184 was selected. The select link tool tracked all traffic that flowed through this point as assigned by the Kern COG Regional Transportation Forecast Model. This location was chosen because it serves as a gateway to and from metropolitan Bakersfield for virtually all east—west truck traffic.

Figure 3-31 illustrates the assignment of truck traffic flowing through the "select link" on State Route 58 east of State Route 184. The Centennial Corridor freeway connector is assumed, connecting the State Route 58 freeway on the east with the Westside Parkway freeway on the west side of State Route 99. The graphic is instructive, because it illustrates that the Centennial Corridor would be the clear minimum time path for State Route 58 east truckers traveling to and from Interstate 5.

Based on the select link analysis, a diversion of trucks from their existing (no-build) routes of choice would be expected.

Table 3-13 indicates the proportion of heavy trucks and the year 2038 volume of trucks estimated to be attracted to the Centennial corridor, assuming build alternatives A, B, or C. These truck flow diversions are based on engineering judgment, based on the following rationale.



Figure 3-31: Year 2038 Daily Select Link Truck Traffic Volumes on State Route 58

Table 3-13. Projected Truck Traffic on Centennial Corridor Build Alternatives A, B, and C

	EAS	ST OF INTERST	ATE 5	WES	T OF STATE RO	UTE 99
LOCATION	ESTIMATED HEAVY TRUCK VOLUME NO-BUILD	PERCENT ATTRACTED TO CENTENNIAL CORRIDOR	ESTIMATED HEAVY TRUCKS ON CENTENNIAL CORRIDOR	ESTIMATED HEAVY TRUCK VOLUME NO-BUILD	PERCENT ATTRACTED TO CENTENNIAL CORRIDOR	ESTIMATED HEAVY TRUCKS ON CENTENNIAL CORRIDOR
State Route 46	3,403	70.0%	2,382	3,695	60.0%	2,217
Lerdo Highway	449	70.0%	314	492	60.0%	295
7th Standard Road	782	80.0%	626	1,960	60.0%	1,176
State Route 58	1,191	80.0%	953	4,352	70.0%	3,046
Stockdale Highway	335	80.0%	268	376	70.0%	263
State Route 119	2,210	40.0%	884	1,748	30.0%	524
Total Volumes	8,370		5,427	12,623		7,521

Analysis of the no-build and transportation systems management/transit alternative estimates of future year truck activity provides a baseline for estimating route choice diversions to the proposed higher speed, and shorter travel time offered by the Centennial Corridor build alternatives A, B, or C. It stands to reason that freight movement that is traveling closest to the Centennial Corridor will be more attracted to the new roadway, freight traffic that is currently traveling on a route further away, will be less attracted. This diversion will also be influenced based on whether the traffic is closest to Interstate 5 or State Route 99. Truckers bound to or from Interstate 5 are more likely to be through traffic, and will likely have a greater attraction to the new facility, which will be significantly faster and shorter than the existing routes.

The movement of light and medium duty trucks (2- and 3-axle trucks, FHWA classifications 4, 5, and 6) on the Centennial Corridor, will have a significantly different attraction pattern than the heavy duty trucks (4- and 5+-axle) projected to use this facility. The light and medium duty trucks primarily carry local freight, with less through movement than the regional and interregional travel pattern of the heavy-duty trucks. Because they serve a different purpose, it is projected that these trucks will be attracted to the Centennial Corridor facility in proportion to the overall use of the corridor by mixed traffic, excluding heavy (through) trucks.

Table 3-14 provides a comparison of the 2008 truck counts on Rosedale Highway with the year 2038 projections for Centennial build alternatives A, B, and C. Table 3-15 provides a similar set of data for year 2018 conditions.

Table 3-14. Comparison of State Route 58 West (Rosedale Highway) Year 2008 Truck
Traffic Counts with Centennial Corridor Year 2038 Forecasts

	2008 TI		JNT DATA ON JTE 58	STATE	CENTENNIAL CORRIDOR YEAR 2038 FORECASTS ALTERNATIVES A, B AND C				
LOCATION	ADT TOTAL	TOTAL 2-3 AXLE	TOTAL 4-5+ AXLE	TOTAL TRUCKS	ADT TOTAL	TOTAL 2-3 AXLE	TOTAL 4-5+ AXLE	TOTAL TRUCKS	
West of State Route 99									
Weekday average	42,511	2,554	2,773	5,327	121,375	7,275	8,250	15,525	
West of Calloway									
Weekday average	30,708	1,164	2,099	3,263	121,785	4,675	6,900	11,575	
West of Allen Road	i								
Weekday average	16,425	765	1,038	1,083	81,033	3,725	6,250	9,975	
East of Enos Lane	(State Rou	ıte 43)							
Weekday average	7,105	481	637	1,118	57,083	3,800	5,950	9,750	
East of Interstate 5									
Weekday average	7,043	453	819	1,272	14,580	825	3,325	4,150	

Notes:

Table 3-15. Comparison of State Route 58 West (Rosedale Highway) Year 2008 Truck Traffic Counts with Centennial Corridor Year 2018 Forecasts

	2008 TI		JNT DATA ON JTE 58	STATE	CENTENNIAL CORRIDOR YEAR 2018 FORECASTS ALTERNATIVES A, B AND C					
LOCATION	ADT TOTAL	TOTAL 2-3 AXLE	TOTAL 4-5+ AXLE	TOTAL TRUCKS	ADT TOTAL	TOTAL 2-3 AXLE	TOTAL 4-5+ AXLE	TOTAL TRUCKS		
West of State Rout	e 99									
Weekday average	42,511	2,554	2,773	5,327	74,310	4,454	5,051	9,505		
West of Calloway										
Weekday average	30,708	1,164	2,099	3,263	61,310	2,354	3,474	5,827		
West of Allen Road	West of Allen Road									
Weekday average	16,425	765	1,038	1,083	25,225	1,160	1,946	3,105		
East of Enos Lane (State Route 43)										
Weekday average	7,105	481	637	1,118	14,210	946	1,481	2,427		
East of Interstate 5										
Weekday average	7,043	453	819	1,272	9,555	541	2,179	2,720		

Source: Parsons

Notes:

^{1.} Trucks with 2-3 axles are primarily local traffic. Trucks with 4-5+ axles are primarily interregional traffic.

^{2.} Rosedale Highway in 2008 does not include Westside Parkway or Centennial Corridor facilities.

^{1.} Trucks with 2-3 axles are primarily local traffic. Trucks with 4-5+ axles are primarily interregional traffic.

^{2.} Rosedale Highway in 2008 does not include Westside Parkway or Centennial Corridor facilities.

3.6 Comparison of Build Alternative versus No-build Traffic Performance

The build alternatives allow more vehicles to utilize State Route 58 compared with the no-build alternative; meaning that vehicles traverse fewer miles on parallel arterial streets and more miles on the State Route 58/Westside Parkway freeway under alternatives A, B, or C, or on Rosedale Highway, which is reclassified as an expressway between State Route 99 and Allen Road under the transportation systems management/transit alternative. Under the no-build alternative, traffic cascades across the highway network seeking available capacity; hence, traffic volume impacts are regional in addition to the study area.

To provide a regional comparison of the transportation systems management/transit build versus no-build traffic-related impacts, Parsons calculated regional benefits using the Kern COG 2006 Regional Travel Demand Model (Update 1) and STEAM 2.0, a system-wide analysis tool.

Table 3-16 reports daily travel statistics produced by the Kern COG 2006 Regional Travel Demand Model (Update 1) for year 2038 no-build and build alternative conditions. The table indicates that overall daily vehicle miles traveled increase for the four build alternatives compared to the no-build condition. The table indicates that the build alternatives have less overall vehicle hours traveled (VHT) than the no-build alternative. Average speeds increase from the no-build condition to the build alternatives.

In addition to the regional travel model, Parsons used STEAM 2.0 to calculate annual performance statistics. The Federal Highway Administration (FHWA) introduced the first version of the Surface Transportation Efficiency Analysis Model (STEAM) in 1997. STEAM was the first FHWA impact analysis product to use input directly from the four-step travel demand modeling process for detailed, system wide analysis of alternative transportation investments. STEAM 2.0 was released in 2000 to expand the scope of the program to address environmental justice measures.

"Like STEAM, STEAM 2.0 is based on the principles of economic analysis, and allows development of monetized impact estimates for a wide range of transportation and investments and policies, including major capital projects, pricing and travel demand management. Impact measures are monetized to the extent feasible, but quantitative estimates of natural resources usage (i.e., energy consumption) and environmental impacts (i.e., emissions) are also provided. Net monetary benefits (or costs) of alternatives can then be used to evaluate trade-offs against non-monetizable benefits, including sustainability and community livability."²

An important feature of STEAM 2.0 is that the software program post-processes the traffic assignment volumes generated from conventional four-step travel forecasting models. The purpose of this feature is to compute benefits based on more accurate highway travel speeds, and to provide FHWA with a more consistent basis of comparison between urban areas.

_

²Surface Transportation Efficiency Analysis Model (STEAM 2.0): User Manual, Cambridge Systematics, Inc., December 2000.

Table 3-16. Regional Travel Model Measures of Effectiveness

	2038 NO-BUILD ALTERNATIVE		TRANSPORTATION SYSTEMS MANAGEMENT/ TRANSIT ALTERNATIVE		BUILD ALTERNATIVE A		BUILD ALTERNATIVE B			BUILD ALTERNATIVE C					
FACILITY TYPE	VMT	VHT	AVG SPEED	VMT	VHT	AVG SPEED	VMT	VHT	AVG SPEED	VMT	VHT	AVG SPEED	VMT	VHT	AVG SPEED
Freeways	19,424,678	301,424	64.44	19,381,198	298,924	64.84	19,902,411	308,027	64.61	19,970,560	309,097	64.61	20,004,215	310,239	64.48
Expressways	939,844	20,462	45.93	1,538,010	36,488	42.15	951,632	20,734	45.90	953,046	20,784	45.85	951,366	20,718	45.92
Major arterials	15,746,341	452,289	34.81	15,201,145	431,083	35.26	15,288,008	436,522	35.02	15,278,278	437,271	34.94	15,244,957	436,299	34.94
Minor arterials	1,825,513	67,724	26.96	1,815,597	67,360	26.95	1,766,960	65,039	27.17	1,760,345	64,789	27.17	1,766,001	65,370	27.02
Collectors	762,494	38,448	19.83	755,071	38,046	19.85	725,959	35,759	20.30	724,242	35,701	20.29	733,412	36,330	20.19
Centroids	2,429,247	110,402	22.00	2,429,655	110,416	22.00	2,429,851	110,429	22.00	2,430,041	110,434	22.00	2,429,589	110,404	22.01
Diamond ramps	417,504	26,220	15.92	425,257	25,313	16.8	513,792	22,985	22.35	482,343	22,948	21.02	455,281	22,368	20.35
Loop ramps	37,300	2,103	17.74	34,709	2,096	16.56	35,507	1,755	20.23	36,565	1,890	19.35	35,106	1,765	19.89
Cordon	655,308	32,767	20.00	655,308	32,767	20.00	655,308	32,767	20.00	655,308	32,767	20.00	655,308	32,767	20.00
All Facilities	42,238,229	1,051,840	40.16	42,235,949	1,042,494	40.51	42,269,429	1,034,017	40.88	42,290,728	1,035,681	40.83	42,275,236	1,036,259	40.80

The STEAM 2.0 speed models account for delays due to incidents, using data on the frequency, severity, and duration of incidents. Incidents account for a large share of total travel delays due to congestion, especially on freeways. The models also account for peak spreading that occurs when facilities become more congested. The traffic temporal distributions used in developing the models were based on data collected from 579 urban automatic traffic recorders across the nation. Separate temporal distributions were developed for freeways and arterials with low, moderate, and high ratios of average daily traffic to capacity. The models further account for day-to-day variations in traffic. The relationship between delays due to congestion and traffic volumes are highly non-linear in nature, especially when the ratio of demand volume to capacity is close to 1.0. Lastly, the STEAM 2.0 speed models account for the decrease in highway capacity that occurs after demand volumes exceed capacity. The 1994 Highway Capacity Manual notes that observations of freeway queue departure rates range from 1,500 to 2,000 passenger cars per hour per lane. In contrast, freeway capacities for 12-foot lanes with no lateral obstructions are 2,200 to 2,300 passenger cars per hour per lane. Not accounting for the fact that queue departure rates are generally lower than freeway capacities can result in a large understatement of the delays due to queuing.³

Table 3-17 reports measures of effectiveness computed with STEAM 2.0, based on travel demand forecast input from the Kern COG regional travel model. Monetized benefits and costs are reported in Section 5 of this document along with assumptions used in the STEAM 2.0 computations.

The data reported in Table 3-17 indicates that like the regional travel model performance statistics, annual vehicle miles traveled (VMT) increase with the build alternatives as motorists take advantage of higher speed routes to travel east-west across town. Travel time decreases, however, due to more efficient route choices and less congestion.

Crashes are forecast to decrease by implementing the build alternative improvements, as the accident rates for urban principal arterials, such as the no-build version of Rosedale Highway, are higher than expressways and freeways. The build alternatives also result in reduced fuel consumption.



Table 3-17. STEAM 2.0 Measures of Effectiveness (Year 2038)

	NO DUIL D	TRANSPORTATION	BUILD ALTERNATIVE					
	NO-BUILD ALTERNATIVE	SYSTEMS MANAGEMENT/ TRANSIT ALTERNATIVE	A	В	С			
1. Travel Demand								
VMT (million VMT/year)	9,951.4	9,955.1	9,994.3	10,004.0	10,006.8			
Travel time (million person hours/year)	397.43	395.95	391.57	391.93	391.53			
2. Tons of Emissions (tons/year)								
VMT Related Emissions								
HC	1							
СО								
NO _X	Please see detailed technical studies							
PM ₁₀								
Cold start emissions								
3. Greenhouse Gas Emissions								
Btu energy consumption (100 billion Btu/year)		Diagram and data:		_				
CO ₂ emissions (1,000 tons/year)	Please see detailed technical studies							
4. Accidents								
Fatalities	68.3	68.2	67.8	67.9	67.9			
Injuries	7,409.1	7,373.1	7,351.6	7,367.8	7,367.7			
Property damage only	12,336.7	12,292.5	12,300.2	12,322.9	12,327.4			
5. Fuel Consumption								
Gallons (1,000 gallons/year)	490,911.3	490,030.0	488,715.5	489,141.6	488,885.5			

Source: Parsons (based on STEAM 2.0)

CHAPTER 4. TRAFFIC OPERATIONS ANALYSIS

4.1 Introduction

As indicated in Section 1.1, Caltrans, in cooperation with the City of Bakersfield, proposes to provide a continuous route along State Route 58 from Interstate 5 to existing State Route 58 east of State Route 99. The proposed continuous route has been divided into three distinct segments. The combination of these three segments comprises the Centennial Corridor project. Segment 1 is the eastern segment that would connect the Westside Parkway (Segment 2) to the existing State Route 58 east freeway. Segment 2 is comprised of what is locally known as the Westside Parkway and extends from Heath Road to Truxtun Avenue. Segment 3 is the western segment that extends from Interstate 5 to Heath Road.

The project's segments are numbered from east to west as shown on Figure 4-1.

Segment 1 extends from the easterly terminus of Segment 2 near Truxtun Avenue to the existing State Route 58 east freeway in the vicinity of Cottonwood Road. It is the only segment where construction alternatives A, B and C are being considered. These three build alternatives plus a transportation systems management/transit alternative (alternative M) and a no-build alternative are being considered.

Segment 2 encompasses a local freeway facility known as the Westside Parkway. As part of the Centennial Corridor project, it is anticipated that the Westside Parkway would be successfully adopted as State Route 58 (under build alternatives A, B, or C).

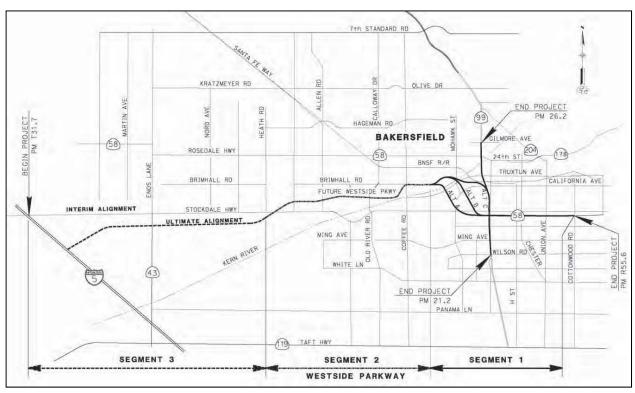


Figure 4-1: Segments for the Centennial Corridor

Westside Parkway is a local, access controlled, multilane freeway that extends from its western terminus near the intersection of Stockdale Highway near Heath Road to its eastern terminus near Mohawk Street, Truxtun Avenue and the Kern River.

Segment 3 extends from Interstate 5, approximately two miles south of the Stockdale Highway/ Interstate 5 interchange, to the intersection of Heath Road and Stockdale Highway. The Centennial Corridor project proposes an interim connection to Interstate 5 via Stockdale Highway from the end of Segment 2 near Heath Road under build alternatives A, B, or C. Under alternative M, the connection to Interstate 5 would remain on existing State Route 58, known locally as Rosedale Highway.

This traffic operations analysis is focused on Segment 1, as the limits of construction primarily occur within this segment along with the majority of potential traffic related impacts. The traffic operations analysis addresses Segment 2 to determine if the freeway connector project (Segment 1) will impact the design requirements of the Westside Parkway. The analysis of Segment 3 traffic operations is limited to the interim connection to Interstate 5 via Stockdale Highway under build alternatives A, B, or C.

The no-build alternative, build alternatives A, B, and C, and the transportation systems management/transit alternative (alternative M) are addressed in this chapter. A summary comparison of the alternatives is provided at the end of the chapter.

4.2 Methodology

The traffic operations analysis considers the overall performance of the highway network, the performance of the freeways affected by the project, specifically State Route 99 and State Route 58, local street intersections impacted by the project, and local traffic circulation impacts. These areas of interest are addressed for the opening year of the project, 2018, and a horizon year of 2038, which constitutes the design year for new construction related to the Centennial Corridor project.

The overall performance of the highway network was gauged using the Kern Council of Governments Regional Transportation Travel Forecasting Model, as previously discussed and reported in Chapter 3.

Freeway operational performance was measured using the CORSIM traffic simulation model. CORSIM (corridor simulation) is the traffic modeling and simulation software package funded by the Federal Highway Administration (FHWA) and is widely used for analyzing existing conditions and evaluating plans for new construction. To apply the simulation model, the physical features of the highway network must first be coded. This as-built condition was ascertained using an aerial photography base map and ground level reconnaissance. Traffic volumes and volume-weighted origin-destination matrices were then entered and the simulation model was calibrated to replicate existing conditions as measured by observed versus modeled traffic volumes, speeds, and traffic queues. Volume II (part 1 of 2) of this report documents the calibration of the CORSIM model for local conditions in metropolitan Bakersfield.

The calibration goal was to replicate local drivers' behavior and traffic characteristics to within ± 10 percent of the existing count and travel time data collected. As CORSIM is a stochastic model, traffic response is simulated on random seeds. Thus, each scenario of network conditions

(AM and PM) is simulated 10 times with different random seeds. The final result is based on the average of the 10 runs. Once the model is calibrated, it can be used to analyze both existing conditions and future scenarios.

To gauge traffic operational performance, level of service (LOS) is a qualitative measure used to describe the driver's experience within a traffic stream, generally in terms of service measures such as speed and travel time, freedom to maneuver, traffic interruptions and delay, and comfort and convenience. Six levels of service are defined by the Highway Capacity Manual (HCM) 2000. Letters designate each level—from LOS A (indicating traffic flows with little or no delay) to LOS F (indicating over-saturated conditions where traffic flow exceeds freeway capacity, generally resulting in long queues and delays). The level of service criteria for density on freeways are presented in Table 4-1.

Table 4-1. Mainline and Ramp Level of Service Definitions

		DENSITY	(pc/mi/ln)
LOS	DESCRIPTION	MAINLINE	RAMPS
Α	Describes free-flow operations. Free-flow speeds prevail.	<11	<10
В	Represents reasonably free-flow operations and free-flow speeds are maintained.	>11 and <18	>10 and <20
С	Provides for flow with speeds at or near the free-flow speed of the freeway.	>18 and <26	>20 and <28
D	Describes the level at which speeds begin to decline slightly with increasing flows and density begins to increase somewhat more quickly.	>26 and <35	>28 and <35
E	At this level's highest density value, it describes operation that is at capacity of the freeway.	>35 and <45	>35
F	Describes breakdown in vehicular flow and queues forming behind the breakdown points.	>45	Demand exceeds capacity

Source: Highway Capacity Manual 2000

To determine freeway level of service, the density of each mainline segment was extracted from the CORSIM output and compared to the criteria listed on Table 4-1.

It must be noted that CORSIM does not output HCM level of service. Simulation model measures of effectiveness are usually not directly translatable into HCM LOS measures, because the HCM bases hourly level of service on the performance of the facility during the peak 15-consecutive-minute period within the analysis hour. Also, most all simulation tools such as CORSIM report the density of vehicles; while the density used in HCM LOS for uninterrupted flow facilities (no signals or stop signs) is the passenger car equivalent in passenger car units of the actual density of vehicles on the facility.

Since each microscopic and macroscopic analysis tool (including the HCM method) has notably different definitions of what constitutes stopped and queued vehicles, and because the tools also vary significantly in the determination of which vehicles to include in the computations (vehicles that both entered and exited during the analysis period, vehicles that did neither, or vehicles that either entered or exited, but not both, during the analysis period), it is not feasible for an analyst to take the macroscopic output from one tool, apply a conversion factor (or procedure) and compare the results to that of another tool.

This conclusion means that looking up HCM levels of service using measures of effectiveness (MOE) produced by a different analytical method is not completely accurate.

The CORSIM derived LOS determinations are therefore considered approximations, to be viewed along with density and speed MOEs.

Insofar as existing conditions, Tables 4-2 through 4-5 list the freeway segment LOS computed via the HCM Highway Capacity Software, versus that estimated based on CORSIM microsimulation.

A comparison of the HCM versus CORSIM results indicates the following: For northbound State Route 99, the basic freeway level of service results are similar or better when the freeway is uncongested, operating at LOS D or better. For merge/diverge sections, CORSIM densities (and therefore LOS) are similar or higher than HCM calculations. Similar findings were also noticed for the southbound direction of State Route 99.

Examining the density parameter, upon which the level of service is graded, indicates the following:

- On State Route 58, which is less congested than State Route 99, the HCM High Capacity Software provides indications of higher densities, and thus more congestion, compared to the more extensive microsimulation of existing traffic operations using the CORSIM model.
- On State Route 99, CORSIM yields higher densities than HCS in segments which experience LOS D or worse traffic operations.

The HCM High Capacity Software software analyzes traffic operations based on isolated link conditions, whereas CORSIM analyzes freeway conditions taking downstream congestion and spillback into account.

For future year conditions, freeway traffic performance will be measured using a traffic simulation program, such as CORSIM, as recommended by FHWA for saturated traffic conditions in an urban setting.¹

In addition to level of service, additional measures of effectiveness can be extracted from the CORSIM traffic simulation model output. These include link volumes served by the facility (throughput), speed, delay time, vehicle miles traveled, vehicle hours traveled, and overall vehicle delay. These are defined on Figure 4-2, which presents an example table of CORSIM analysis results.

In addition to evaluating the performance of the State Route 99 and State Route 58/Westside Parkway freeway segments in the study corridor, interchange ramp termini intersections were evaluated as part of this study. The CORSIM software was again used for the intersection analysis, as both freeway and the ramp intersections were modeled as an integrated system. CORSIM is capable of analyzing both unsignalized and signalized intersections and was used to compute control delay per vehicle. HCM 2000-defined levels of service criteria for intersections are presented in Table 4-6.

Table 4-7 lists the 79 intersections selected for study and Figure 4-3 illustrates their locations. As noted above, ramp terminal and adjacent intersections were analyzed using the CORSIM microsimulation software. The remaining isolated intersections were analyzed using SYNCHRO, version 6.0 software.

_

¹ Interstate System Access Informational Guide, August 2010.

Table 4-2. Freeway Mainline and Ramp Junction Level of Service—Existing Conditions—State Route 58 Eastbound

,														
					HCN	/ 1					COF	RSIM ²		
				AM PEAK HOUR			PM PEAK HO	OUR		AM PEAK H	OUR		PM PEAK H	OUR
LOCATION	LANES	TYPE	LOS	DENSITY	SPEED	LOS	DENSITY	SPEED	LOS	DENSITY	SPEED	LOS	DENSITY	SPEED
SR 99 to H Street	3	Weave	D	_	_	D	_	_	D	31.9	57	D	29.9	57
H Street off-ramp to Chester Ave on-ramp	2	Basic	D	29.0	63.5	D	27.0	64.3	С	20.7	59	С	20.8	61
Chester Avenue on-ramp	2	Merge	D	34.9	52.5	D	33.8	53.3	D	32.7	57	D	32.5	55
Chester Avenue to Union Avenue	2	Basic	D	34.3	60.3	D	33.7	60.7	D	33.3	53	D	33.6	52
Union Avenue off-ramp	2	Diverge	Е	38.5	56.2	Е	38.1	56.8	Е	35.5	53	Е	36.1	51
Union Avenue off-ramp to on-ramp	2	Basic	С	24.7	64.9	D	27.4	64.2	С	24.1	60	С	25.8	59
Union Avenue SB on-ramp	2	Merge	D	29.1	55.4	D	31.8	54.3	С	25.4	60	С	27.6	59
Union Avenue NB on-ramp	2	Merge	D	30.5	55.3	D	33.5	53.6	С	26.9	60	D	29.4	60
Union Avenue to Cottonwood Road	2	Basic	D	28.1	63.9	D	33.2	61.0	С	25.9	61	D	28.2	61

¹Source: Fehr & Peers, 2010. See Table 2-6 for notes.

²Source: Parsons, 2011 Level of service E or F

Table 4-3. Freeway Mainline and Ramp Junction Level of Service—Existing Conditions—State Route 58 Westbound

					HCN	/ 11					COR	SIM ²		
				AM PEAK HO	UR	Р	M PEAK HO	UR		AM PEAK H	OUR		PM PEAK H	OUR
LOCATION	LANES	TYPE	LOS	DENSITY	SPEED	LOS	DENSITY	SPEED	LOS	DENSITY	SPEED	LOS	DENSITY	SPEED
Cottonwood Road to Union Avenue	2	Basic	D	30.4	62.8	D	26.9	64.4	D	34.7	50	С	24.5	62
Brundage Lane off-ramp	2	Diverge	Е	35.7	50.9	D	32.7	51.3	Е	48.2	32	С	24.9	61
Brundage Lane off-ramp to on-ramp	2	Basic	С	24.8	64.9	С	23.6	65.0	С	22.9	57	С	22.1	62
Brundage Lane on-ramp	2	Merge	D	29.4	55.3	D	28.5	55.6	С	23.6	59	С	24.9	60
Union Avenue SB on-ramp	2	Merge	D	31.3	54.4	D	31.0	54.5	D	30.3	54	D	30.7	54
Chester Avenue off-ramp	2	Diverge	Е	35.2	57.0	Е	35.0	57.1	D	34.5	48	D	34.7	48
Chester Ave off-ramp to H Street on-ramp	2	Basic	С	24.7	64.9	С	24.8	64.9	С	21.4	60	С	23.8	60
H Street on-ramp	2	Merge	D	29.8	55.5	D	31.0	55.0	С	24.8	58	D	28.9	57
H Street to SR 99	2	Basic	D	27.9	64.0	D	29.4	63.3	С	23.4	59	D	27.2	58
SR 99 NB off-ramp	2	Diverge	D	33.6	58.2	D	34.8	58.4	С	24.3	60	D	28.1	59
SR 99 NB off-ramp to SB off-ramp	2	Basic	В	17.0	65.0	С	18.6	65.0	В	15.9	58	С	19.5	57
SR 99 SB off-ramp	2	Diverge	С	22.2	49.9	С	24.1	49.5	В	17.1	53	С	21.0	53

¹Source: Fehr & Peers, 2010. See Table 2-7 for notes.

²Source: Parsons, 2011

Level of service E or F



Table 4-4. Freeway Mainline and Ramp Junction Level of Service—Existing Conditions—State Route 99 Northbound

					НС	M1					COR	SIM ²		
				AM PEAK H	OUR		PM PEAK HO	OUR		AM PEAK HO	DUR		PM PEAK H	OUR
LOCATION	LANES	TYPE	LOS	DENSITY	SPEED	LOS	DENSITY	SPEED	LOS	DENSITY	SPEED	LOS	DENSITY	SPEED
Panama Lane to White Lane	3	Basic	С	21.9	65.0	В	15.1	65.0	С	18.8	63	В	12.8	64
White Lane off-ramp	3	Diverge	С	27.3	61.2	С	21.0	60.8	В	18.6	63	В	13.0	64
White Lane off-ramp to loop on-ramp	3	Basic	С	20.6	65.0	В	13.1	65.0	В	17.9	62	В	11.1	63
White Lane EB loop on-ramp	3	Merge	Е	35.6	53.9	С	25.0	58.1	Е	47.9	38	С	24.4	50
White Lane WB on-ramp	3	Merge	Е	35.3	54.0	С	23.9	58.6	Е	38.3	53	С	23.7	59
White Lane to Ming Avenue	3	Basic	D	34.9	59.8	С	21.5	65.0	D	29.3	59	С	18.3	61
Ming Avenue off-ramp	4	Diverge	С	27.6	59.4	С	20.4	59.6	D	32.6	59	С	22.2	61
Ming Avenue off-ramp to on-ramp	4	Basic	C	22.8	65.0	В	14.6	65.0	D	29.7	45	В	14.4	56
Ming Avenue on-ramp	4	Merge	D	31.2	57.3	С	24.0	59.5	Е	48.9	43	D	32.4	52
SR 58 off-ramp	4	Diverge	Е	40.6	61.4	D	30.0	62.4	Е	48.9	43	D	32.5	52
SR 58 off-ramp to Wible Road on-ramp	4	Basic	С	21.0	65.0	В	13.6	65.0	С	19.7	60	В	12.1	63
Wible Road on-ramp	4	Merge	С	23.3	59.0	В	17.7	60.2	С	26.9	49	С	20.5	57
SR 58 on-ramp	4	Merge	D	31.1	57.4	С	24.8	59.4	D	34.0	56	D	32.0	53
SR 58 to California Avenue	4	Basic	D	28.8	63.6	С	20.9	65.0	D	26.3	58	С	19.6	59
California Avenue off-ramp	4	Diverge	Е	36.7	60.9	С	27.0	62.4	Е	35.9	55	С	27.8	59
California Avenue off-ramp to on-ramp	4	Basic	С	23.4	65.0	В	18.0	65.0	С	24.1	54	В	17.7	57
California Avenue EB on-ramp	4	Merge	С	25.3	58.5	С	22.2	59.4	D	31.7	51	D	28.9	53
California Avenue WB on-ramp	4	Merge	С	24.8	58.5	С	21.4	59.5	Е	35.9	54	D	34.9	52
California Avenue to Rosedale Highway	4	Basic	D	26.5	64.5	С	22.1	65.0	D	26.0	55	С	20.9	56
Rosedale Highway off-ramp	4	Diverge	Е	38.2	59.5	D	34.3	59.2	Е	40.1	51	D	32.8	55
Buck Owens Boulevard off-ramp	4	Diverge	O	26.3	57.7	В	18.9	59.9	С	21.0	58	В	15.6	61
Buck Owens Boulevard off-ramp to on-ramp	4	Basic	В	16.5	65.0	В	13.6	65.0	В	13.2	63	В	11.2	63
Buck Owens Boulevard on-ramp	4	Merge	В	18.6	60.0	В	17.8	60.2	С	20.6	60	В	19.9	59
Airport Drive off-ramp	4	Diverge	С	27.9	59.2	С	23.8	60.3	С	22.8	60	С	20.9	60
Airport Drive to Golden State Avenue	3	Basic	В	14.4	65.0	В	14.2	65.0	Α	9.7	64	Α	9.6	63

¹Source: Fehr & Peers, 2010. See Table 2-8 for notes. ²Source: Parsons, 2011



Level of service E or F

Table 4-5. Freeway Mainline and Ramp Junction Level of Service—Existing Conditions—State Route 99 Southbound

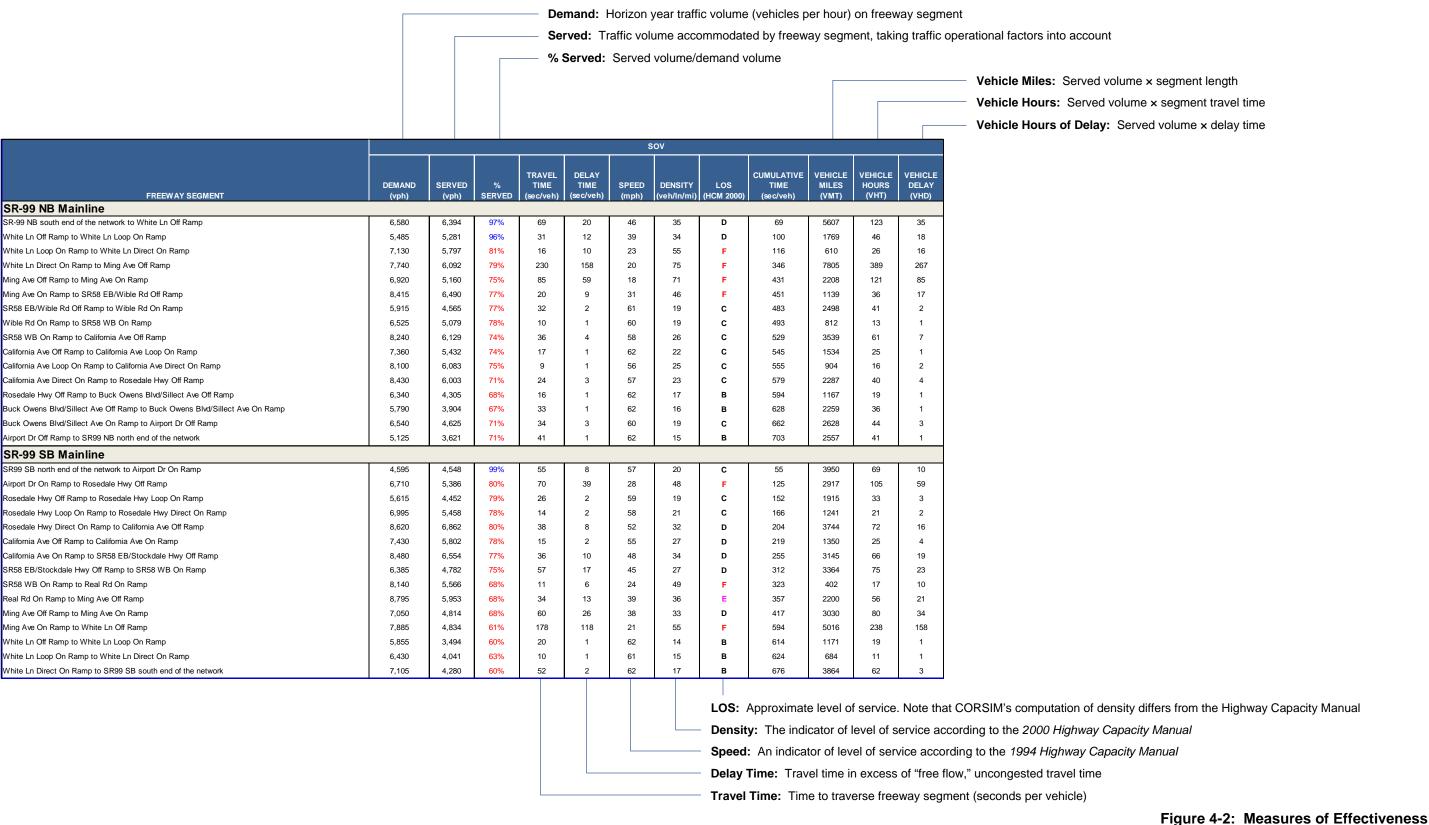
					Н	CM ¹		CORSIM ²							
			Д	M PEAK H	OUR		PM PEAK H	IOUR	A	M PEAK H	OUR	F	M PEAK H	OUR	
LOCATION	LANES	TYPE	Los	DENSITY	SPEED	Los	DENSITY	SPEED	LOS	DENSITY	SPEED	Los	DENSITY	SPEED	
Golden State Avenue to Airport Drive	3	Basic	С	19.8	65.0	С	19.7	65.0	В	15.5	64	В	16.6	64	
Airport Drive on-ramp	4	Merge	С	23.9	59.5	D	29.2	58.1	С	27.2	57	D	33.9	55	
Airport Drive to Rosedale Highway	4	Basic	С	19.4	65.0	O	22.6	65.0	В	16.9	59	C	21.0	57	
Rosedale Highway off-ramp	4	Diverge	С	25.4	62.6	D	28.5	62.4	С	25.5	60	D	32.0	57	
Rosedale Highway off-ramp to on-ramp	4	Basic	В	16.7	65.0	O	19.7	65.0	В	14.9	58	C	25.4	42	
Rosedale Highway WB on-ramp	4	Merge	С	21.9	59.5	D	28.9	57.9	С	25.7	55	D	33.7	49	
Rosedale Highway EB on-ramp	4	Merge	С	25.9	58.9	D	32.4	56.6	D	33.0	59	Е	37.2	58	
Rosedale Highway to California Avenue	4	Basic	С	23.4	65.0	D	30.5	62.7	С	20.8	59	D	29.9	54	
California Avenue off-ramp	4	Diverge	D	31.8	61.2	Е	37.1	61.1	D	33.6	58	Е	42.7	52	
California Avenue off-ramp to on-ramp	4	Basic	С	18.7	65.0	С	25.2	64.8	В	16.1	62	D	31.2	45	
California Avenue on-ramp	4	Merge	С	20.4	59.6	D	29.9	57.4	D	29.2	57	D	34.8	56	
California Avenue to SR 58	4	Basic	С	20.0	65.0	D	29.4	63.3	В	17.9	59	D	27.4	57	
SR 58 off-ramp	4	Diverge	D	31.0	61.9	F	_	_	D	28.1	59	Е	41.1	53	
SR 58 off-ramp to on-ramp	4	Basic	В	13.2	65.0	С	20.1	65.0	В	11.5	63	С	20.6	56	
SR 58 on-ramp	4	Merge	С	20.5	59.8	С	27.3	58.2	D	28.9	49	D	31.4	51	
Real Road on-ramp	4	Merge	В	18.3	60.1	С	24.6	58.6	С	26.6	56	D	34.2	55	
Ming Avenue off-ramp	4	Diverge	С	24.7	62.1	Е	36.2	60.0	С	24.7	59	Е	39.0	52	
Ming Avenue off-ramp to on-ramp	4	Basic	В	14.8	65.0	С	19.5	65.0	В	12.7	63	В	17.5	63	
Ming Avenue on-ramp	3	Merge	С	22.0	58.9	D	29.3	57.0	В	18.2	60	С	25.2	59	
Ming Avenue to White Lane	3	Basic	С	21.4	65.0	D	30.6	62.6	В	16.3	62	С	24.0	59	
White Lane off-ramp	3	Diverge	В	15.1	59.1	С	24.7	57.3	С	22.3	61	D	34.6	56	
White Lane off-ramp to on-ramp	3	Basic	В	13.5	65.0	В	17.7	65.0	В	11.2	63	В	15.3	63	
White Lane WB on-ramp	3	Merge	В	16.1	59.6	С	20.1	58.9	В	12.0	62	В	15.5	61	
White Lane EB on-ramp	3	Merge	В	16.2	59.8	С	20.0	59.2	В	12.3	62	В	16.1	62	
White Lane to Panama Lane	3	Basic	В	14.7	65.0	С	19.1	65.0	В	11.8	63	В	16.0	62	

¹Source: Fehr & Peers, 2010. See Table 2-9 for notes.

Level of service E or F



²Source: Parsons, 2011



Derived from CORSIM

Traffic Simulation Model

Table 4-6. Level of Service Definitions for Intersections

		CONTROL DELA (sec/	
LOS	DESCRIPTION	UNSIGNALIZED	SIGNALIZED
Α	Traffic flows with very little delay and optimal speeds. Most vehicles do not stop at all.	0–10	<10
В	Traffic flows with very little delay and speeds may be slightly reduced. Very infrequent and short waits at traffic signals. More vehicles stop at intersections than for LOS A.	>10–15	>10-20
С	Traffic speeds continue to slow. Some vehicles may stop at this level, although many vehicles still pass through the intersection without stopping.	>15–25	>20-35
D	Congestion becomes more noticeable. Many vehicles stop and the proportion of vehicles not stopping declines.	>25–35	>35–55
E	Low speeds and traffic backups at intersections. Often considered to be the limit of acceptable delay.	>35–50	>55-80
F	Very slow speeds and congestion. Long traffic backups. Very likely to wait for multiple greens to get through an intersection. This is considered to be unacceptable to most drivers.	>50	>80

Source: Highway Capacity Manual 2000

Table 4-7. List of Study Intersections

Segment 3	N
1 I-5 southbound ramps/Stockda	ale Highway
2 I-5 northbound ramps/Stockda	le Highway
3 SR 43 (Enos Lane)/Rosedale I	Highway
4 SR 43 (Enos Lane)/Stockdale	Highway
5 SR 43 (Enos Lane)/I-5 northbo	ound ramps
6 SR 43 (Enos Lane)/I-5 southbo	ound ramps
7 Stockdale Highway/Nord Road	j
8 Stockdale Highway/Wegis Ave	enue
Segment 2	
9 Stockdale Highway/Heath Roa	ıd
10 Stockdale Highway/Westside F	Parkway
11 West Beltway/Westside Parkw ramp	ay westbound
12 West Beltway/Westside Parkw ramp	ay eastbound
13 Allen Road/Rosedale Highway	,
14 Allen Road/Brimhall Road	
15 Allen Road/Westside Parkway	westbound
ramps	
16 Allen Road/Westside Parkway ramps	eastbound
17 Allen Road/San Juan Avenue	
18 Allen Road/Stockdale Highway	/
19 Calloway Drive/Rosedale High	way
20 Calloway Drive/Brimhall Road	
21 Calloway Drive/Westside Park ramps	way westbound
22 Calloway Drive/Westside Park ramps	way eastbound
23 Calloway Drive/Stockdale High	nway
Segment 1	
24 Coffee Road/Rosedale Highwa	ay
25 Coffee Road/Brimhall Road	
26 Coffee Road/Westside Parkwa ramps	ay westbound
27 Coffee Road/Westside Parkwa ramps	y eastbound
28 Coffee Road/Truxtun Avenue	
00 0 " D 1/0: 1 1 1 ::: :	ay
29 Coffee Road/Stockdale Highwa	
29 Coffee Road/Stockdale Highwa 30 Mohawk Street/Rosedale High	way
-	•
30 Mohawk Street/Rosedale High 31 Mohawk Street/Westside Park	way westbound
30 Mohawk Street/Rosedale High 31 Mohawk Street/Westside Park ramps 32 Mohawk Street/Westside Park	way westbound
30 Mohawk Street/Rosedale High 31 Mohawk Street/Westside Park ramps 32 Mohawk Street/Westside Park ramps	way westbound way eastbound e

NO.	INTERSECTION
36	Airport Drive/State Road-SR 204 off-ramp
37	Airport Drive/SR 99 northbound ramp
38	Buck Owens Boulevard/Rio Mirada Drive
	SR 99 northbound ramps/Buck Owens
39	Boulevard
40	Rosedale Highway/Camino Del Rio Court
41	Rosedale Highway/SR 99 southbound ramps
42	Rosedale Highway/SR 99 northbound ramps
43	24th Street/Oak Street
44	Truxtun Avenue/Empire Drive
45	Truxtun Avenue/Oak Street
46	California Avenue/Chester Lane
47	California Avenue/SR 99 southbound ramps
48	California Avenue/SR 99 northbound ramps
49	California Avenue/Oak Street
50	Stockdale Highway/Stine Road
51	Stockdale Highway/Real Road
52	Stockdale Highway/SR 99 southbound ramp
53	Brundage Lane/Oak Street
54	Real Road/SR 58
55	Wible Road/SR 99 northbound ramps
56	Ming Avenue/New Stine
57	Ming Avenue/Real Road
58	Ming Avenue/SR 99 southbound ramps
59	Ming Avenue/Wible Road
60	Ming Avenue/SR 99 northbound ramps
61	Ming Avenue/Castro Lane
62	White Lane/Wible Road
63	Ming Avenue/SR 99 southbound ramps
64	Ming Avenue/SR 99 northbound ramps
65	Ming Avenue/Hughes Lane
66	H Street/Brundage Lane
67	H Street/SR 58 westbound ramp
68	H Street/SR 58 eastbound ramp
69	H Street/Ming Avenue
70	Chester Avenue/Brundage Lane
71	Chester Avenue/SR 58 westbound ramp
72	Chester Avenue/SR 58 eastbound ramp
73	Chester Avenue/Ming Avenue
74	Union Avenue/Brundage Lane
75	Brundage Lane/SR 58 westbound ramps
76	Union Avenue/SR 58 eastbound ramps
77	Cottonwood Road-MLK/Brundage Lane
78	Cottonwood Road-Brundage Lane/SR 58 westbound ramps
79	Cottonwood Road/SR 58 eastbound ramps



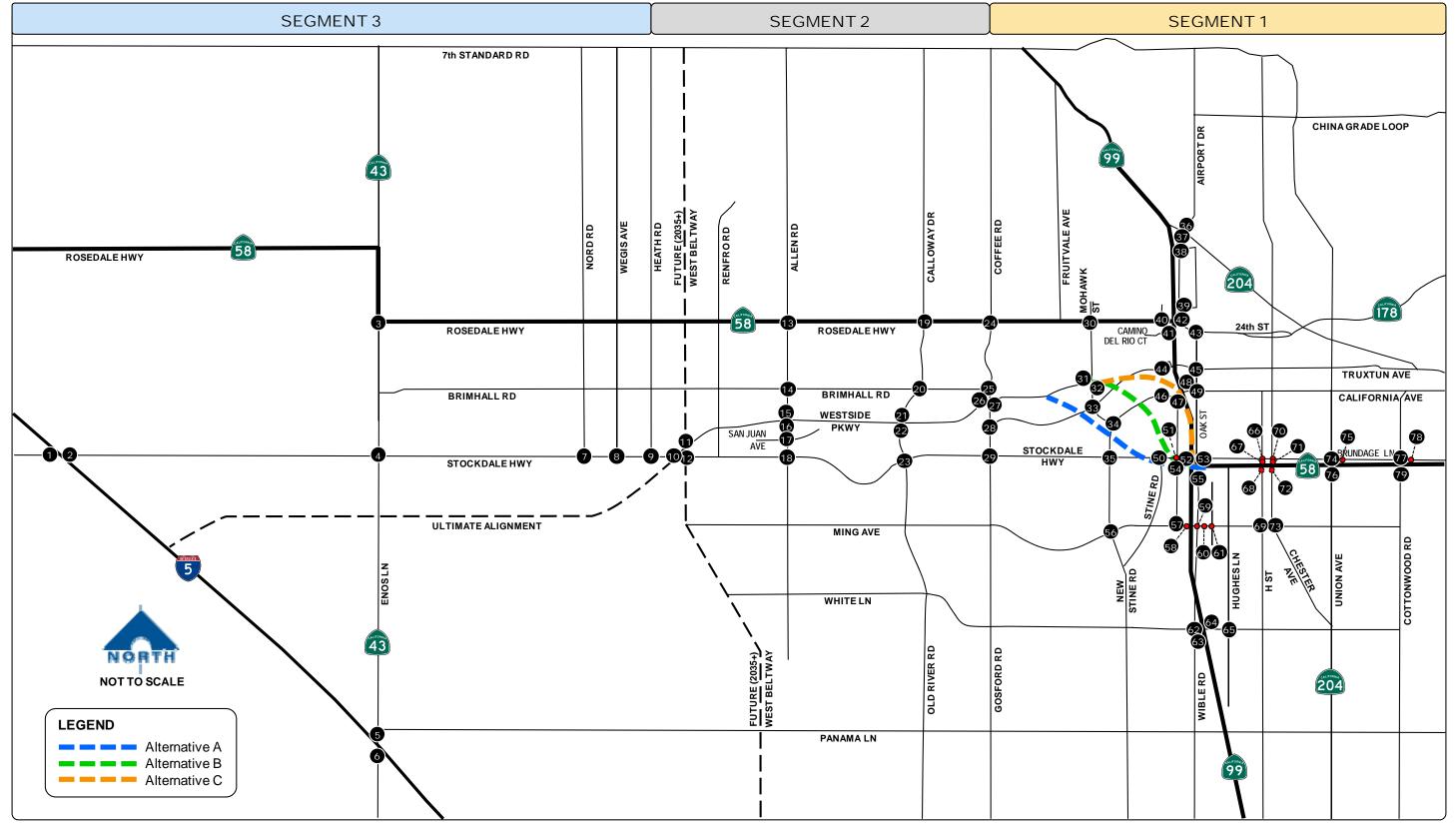


Figure 4-3: Location Map of Study Intersections

4.3 Traffic Performance of the No-build Alternative

The no-build alternative was described in Section 3.4, but is repeated here for reference.

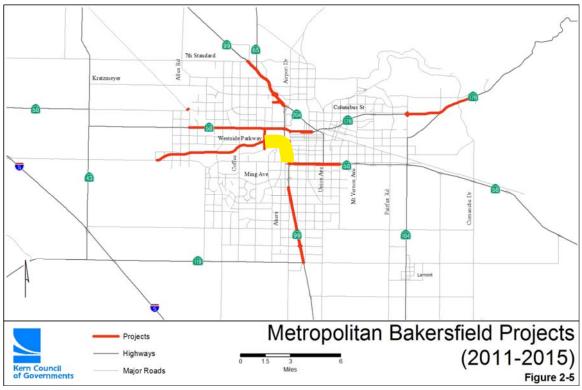
The no-build alternative includes all projects identified in Table 3-3 and Figures 3-4 and 3-5 except for the State Route 58 Connector/Centennial Corridor (State Route 58 to Westside Parkway) as identified on Figure 3-4 as project number 22.

Key projects included with the no-build alternative, all of which are identified in Amendment 1 to the 2011 Regional Transportation Plan and the associated Air Quality Conformity Determination, are listed below with the locations of the projects illustrated on Figure 4-4.

Project	Location	Start
Route 58	Rosedale Highway—Calloway Drive to Route 99—Widen existing highway	2013
Route 58	Rosedale Highway—Allen Road to Calloway Drive—Widen existing highway	2013
Route 58	Route 99 to Cottonwood Road—Widen to six lanes	2015
Route 99	Wilson Road to Route 119—Widen to eight lanes	2012
Route 99	Route 204 to 7th Standard Road—Widen to eight lanes (Phase 1)	2012
Westside Parkway	Route 99/Oak Street to Heath Road—Construct local freeway	2009
Hageman Flyover	Knudsen Drive to Route 204—Construct extension	2013
Oak St/24th St	Route 178 (24th Street) and Oak Street—Construct improvements	2012
24th Street	Route 178 (24th and 23rd St) Oak Street to M Street—Widen existing highway	2013
Route 58	Rosedale Highway—Route 43 to Allen Road—Widen existing highway	2025
West Beltway	Rosedale Highway to Westside Parkway—Construct new facility	2025
Route 204	Airport Drive to Route 178—Widen existing highway	2030
Route 204	F Street—Construct interchange	2030
Route 58	At various locations—ramp improvements	2033
Route 99	Route 204 to 7th Standard Road—Widen to eight lanes (Phase 2)	2033
Route 99	At various locations—ramp improvements	2033
West Beltway	Pacheco Road to Westside Parkway—Construct new facility	2033
West Beltway	Rosedale Highway to 7th Standard Road—Construct new facility	2033
West Beltway	Taft Highway to Pacheco Road—Construct new facility	2033

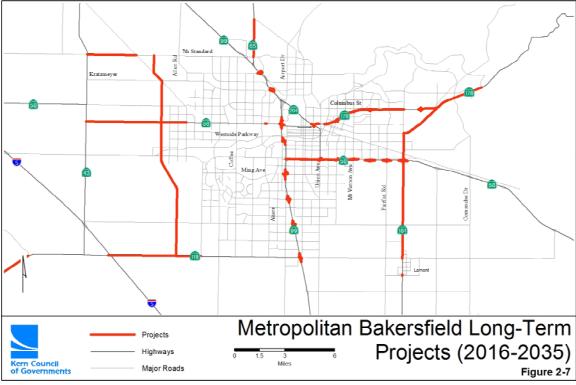
Given these investments, the roadway network will experience congestion along Rosedale Highway, 24th Street, the Truxtun Avenue extension west of Oak Street, portions of Stockdale Highway, State Route 99, portions of State Route 178, Coffee Road, Mohawk Street, and Union Avenue. The severity of congestion, as measured by level of service grade (D, E, or F), and location of congestion are depicted on Figure 4-5.

As indicated above, the CORSIM traffic simulation model was used to quantify the performance of the State Route 58 and State Route 99 freeway segments and ramps located within the study area. The configuration of these freeway segments, insofar as the number of lanes and interchange on- and off-ramps, and traffic demand volumes for 2038 are illustrated on Figure 4-6.



Source: Final Regional Transportation Plan Amendment No. 1, Kern Council of Governments, May 2011

Centennial Corridor Project



Source: Final Regional Transportation Plan Amendment No. 1, Kern Council of Governments, May 2011

Figure 4-4: Locations of Key No-Build Alternative Projects

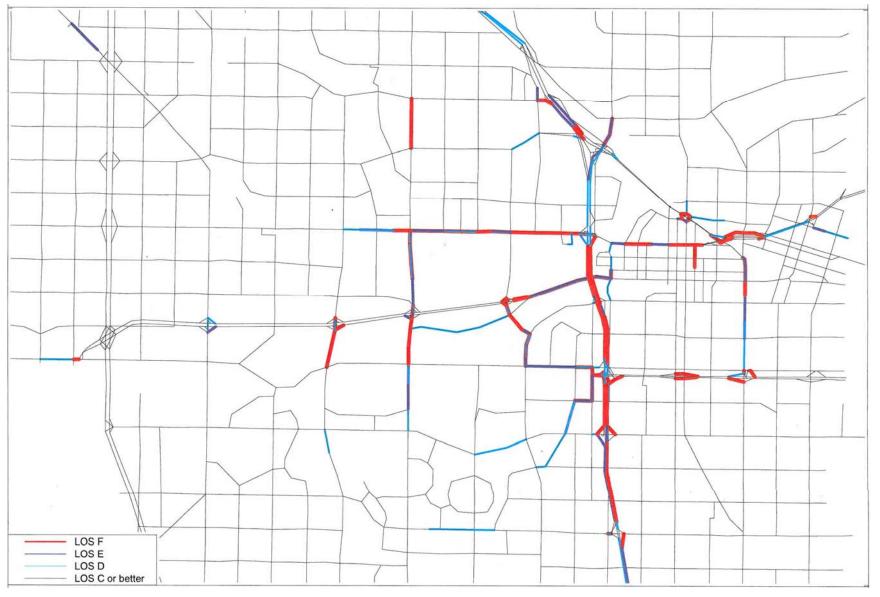


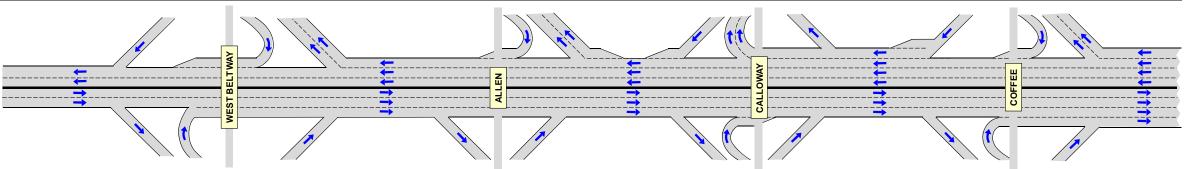
Figure 4-5: Locations of Congestion for the No-Build Alternative

A. State Route 58 and Westside Parkway

State Route 58 Wes	oth ound	MAINLINE			WEST	BELTWAY		MAINLINE		ALLEN ROA	D	MAINI INE		CALL	I YAWO.	DRIVE		MAINLINE		CC	OFFEE R	ROAD		MAINLINE
State Route 58 Wes	Stbound	WAINLINE	ON	MAINLINE	ON	MAINLINE	OFF	IVIAIIVLIIVE	ON	MAINLINE	OFF	WAINLINE	ON	MAINLINE	OFF	MAINLINE	OFF	WAINLINE	ON	MAINLINE	ON	MAINLINE	OFF	WAINLINE
NO BLILLD	AM Peak	1,550	290	1,260	395	1,575	1,060	1,925	225	1,700	850	2,550	465	2,085	580	2,665	312	2,977	645	2,332	555	1,777	1,185	2,962
NO-BUILD ALTERNATIVE	PM Peak	2,305	350	1,955	545	2,545	1,875	3,285	330	2,955	1,880	4,835	985	3,850	1,015	4,865	510	5,375	1,025	4,350	980	3,370	1,215	4,585
ALIERNATIVE	ADT	27,240	5,955	21,285	6,185	25,810	16,690	31,790	2,615	29,175	20,705	49,880	12,330	37,550	10,545	48,095	6,486	54,580	10,565	44,015	7,860	36,155	16,215	52,370

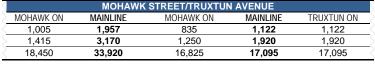


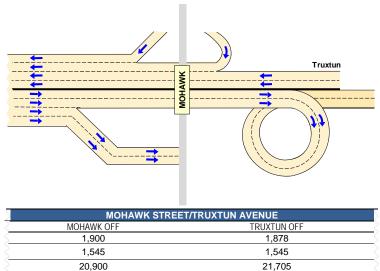
MAINLINE | State Route 58 WB



State Route 58 Eas	oth ound	MAINLINE		WE	ST BELTW.	AY		MAINLINE	A	LLEN ROA		MAINLINE		CALL	OWAY I	DRIVE		MAINI INF		COI	FEE RO	DAD		MAINLINE
State Route 36 Eas	Stbound	WAINLINE	OFF	MAINLINE	ON	MAINLINE	ON	WAINLINE	OFF	MAINLINE	ON	MAINLINE	OFF	MAINLINE	ON	MAINLINE	ON	WAINLINE	OFF	MAINLINE	ON	MAINLINE	ON	WAINLINE
NO BLIII D	AM Peak	2,190	800	1,725	1,185	2,910	550	3,124	195	2,929	1,810	4,739	1,108	3,631	484	4,115	914	5,029	2,185	2,844	385	3,229	550	3,778
NO-BUILD ALTERNATIVE	PM Peak	2,180	955	1,615	875	2,490	145	2,245	180	2,065	1,610	3,675	929	2,745	381	3,127	762	3,889	1,705	2,184	430	2,614	475	3,090
	ADT	28,010	10,835	21,675	12,190	33,865	1,960	31,325	2,575	28,750	21,980	50,730	13,170	37,560	5,115	42,675	11,125	53,800	22,365	31,435	6,555	37,990	4,615	42,605

STATE ROUTE 99 SYSTEM INTERCHANGE





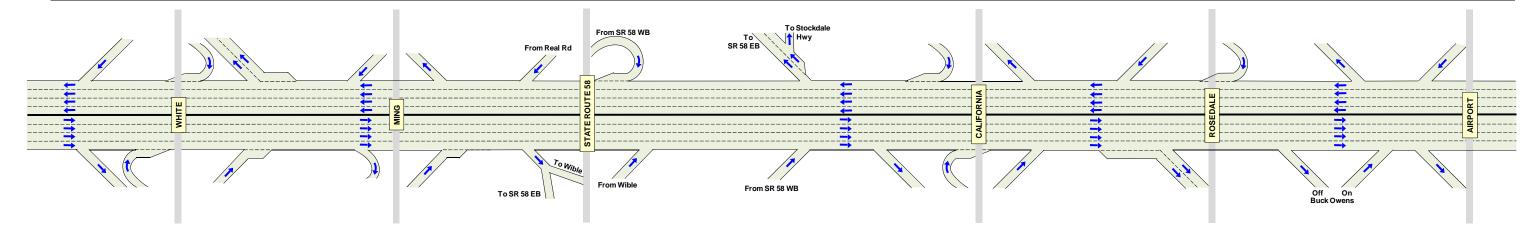
210 REAL RD OFF	F OFF	MAINLINE	OFF		ON	MAINLINE	OFF		ON I	NAINLINE	ON	MAINLINE	OFF		ON	MAINLINE	. OFF			
930	1,550	2,480	1,785	4,265	405	3,860	735	4,595	400	4,195	220	3,975	880	4,855	240	4,615	255	4,870	AM Peak	NO-BUILD
1,085	1,755	2,840	1,715	4,555	595	3,960	645	4,605	555	4,050	395	3,655	595	4,250	400	3,850	240	4,090	PM Peak	ALTERNATIVE
17,870	19,960	37,830	22,555	60,385	9,880	50,505	14,760	65,265	9,165	56,100	5,310	50,790	12,650	63,440	5,570	57,870	3,600	61,470	ADT	ALIERNATIVE
	To SR 99 SB		From is 99 NB			H/CHESTE	/*/			\(\frac{t}{1}\)	NON			-	1	COTTONWC		*		
FROM REAL RD		ON	ANGE	MAINLINE	OFF	T/CHEST MAINLIN		MAINLINE	OFF	MAINLIN	ION AVE	MAINLINE	ON	MAINLINE		NWOOD RO	ON	MAINLINE	State Ro	oute 58 WB
1,345	-	3,505		4,850	545	4,305		4,900	915	3,985	_	4,285	275	4,560	550	4,010	305	4,315	AM Peak	
1,195		3,390		4,585	475	4,110		4,950	645	4,305	770	5,075	635	5,710	600	5,110	485	5,595	PM Peak	NO-BUILD ALTERNATIVE
10,947		51,251		62,198	11,67	5 50,52	13,735	64,258	13,115	51,143	9,085	60,228	6,025	66,253	6,615	59,638	5,085	64,723	ADT	/\L!\L!\\\/\!!VL

Figure 4-6: Year 2038 No-Build Alternative Freeway Lane Configuration and Forecast Traffic Volumes (1 of 2)

B. State Route 99



State Rou	te 99	MAINLINE		V	/HITE L	ANE		MAINLINE			MIN	G AVENUE/R	REAL ROA	D/STATE ROI	JTE 58			MAINLINE	_	FORNIA AV	ENUE	MAINLINE		ROSE	DALE HI	GHWAY		MAINLINE	AIRPORT DRIVE	MAINLINE
Southbou	nd	WAINLINE	ON	MAINLINE	E ON	MAINLINE			MING ON	MAINLINE	MING OFF	MAINLINE	REAL ON	MAINLINE	SR 58 WB ON	MAINLINE	SR 58 EB	WAINLINE	ON	MAINLINE	OFF	WAINLINE	ON	MAINLINE	ON	MAINLINE	OFF	MAINLINE	ON	WAINLINE
2038	AM Peak	5,410	460	4,950	210	4,740	1,495	6,235	690	5,545	1,230	6,775	510	6,265	1,550	4,715	1,890	6,605	720	5,885	1,200	7,085	1,690	5,395	1,150	4,245	1,075	5,320	1,425	3,895
NO-BUILD	PM Peak	7,105	675	6,430	575	5,855	2,030	7,885	835	7,050	1,745	8,795	655	8,140	1,755	6,385	2,095	8,480	1,050	7,430	1,190	8,620	1,625	6,995	1,380	5,615	1,095	6,710	2,115	4,595
ALTERNATIVE	ADT	92,700	9,595	83,105	7,560	75,545	20,500	96,045	12,125	83,920	24,570	108,490	3,150	105,340	19,960	85,380	26,465	111,845	15,475	96,370	16,875	113,245	20,210	93,035	16,400	76,635	13,105	89,740	24,200	65,540



State Rout	te 99			WI	HITE LAN	ΙE					MIN	3 AVENUE	/WIBLE RO	DAD/STATE	ROUTE 5	58				CALIFO	RNIA A	/ENUE				ROSEDA	ALE HIG	HWAY			AIRPORT DRIVE	
Northboun		MAINLINE		MAINLINE	ON	MAINLINE		MAINLINE		MAINLINE	MING ON	MAINLINE	WIBLE/ SR 58 EB OFF	MAINLINE	WIBLE ON	MAINLINE	SR 58 WB ON	MAINLINE	OFF	MAINLINE	ON	MAINLINE	E ON	MAINLINE	OFF	MAINLINE	OFF	MAINLINE	ON	MAINLINE	OFF	MAINLINE
2038	AM Peak	7,025	1,065	5,960	1,605	7,565	695	8,260	790	7,470	1,585	9,055	2,825	6,230	640	6,870	1,785	8,655	1,180	7,475	565	8,040	240	8,280	2,440	5,840	825	5,015	600	5,615	1,750	3,865
NO-BUILD	PM Peak	6,580	1,095	5,485	1,645	7,130	610	7,740	820	6,920	1,495	8,415	2,500	5,915	610	6,525	1,715	8,240	880	7,360	740	8,100	330	8,430	2,090	6,340	550	5,790	750	6,540	1,415	5,125
ALTERNATIVE	ADT	92,950	11,875	81,075	18,790	99,865	8,500	108,365	10,580	97,785	17,975	115,760	36,680	79,080	6,995	86,075	22,555	108,630	10,595	98,035	10,030	108,065	4,910	112,975	26,655	86,320	9,615	76,705	10,590	87,295	21,795	65,500

Figure 4-6: 2038 No-Build Alternative Freeway Lane Configuration and Forecast Traffic Volumes (2 of 2)

Table 4-8 presents AM peak hour measures of effectiveness performance data for the freeway mainline segments while Table 4-9 presents PM peak hour measures of effectiveness. Segments with level of service E or F performance in the design year of 2038 are highlighted.

Insofar as year 2038 AM peak hour traffic operations, Table 4-8 indicates that all segments along the Westside Parkway operate at LOS D or better, with served volumes nearly equaling the demand volumes (95 percent or higher). Regarding State Route 58 east of State Route 99, all segments are expected to operate at LOS D or better. Along State Route 99, year 2038 traffic congestion prevails in the northbound direction from south of White Lane to State Route 58. This congestion creates a bottleneck condition whereby the traffic volumes served are significantly lower than demand volumes (70 to 82 percent). Southbound, all segments operate at LOS D or better.

During the PM peak hour, Table 4-9 indicates that all segments along the Westside Parkway are expected to operate at LOS C or better in year 2038, with served volumes nearly equaling demand volumes (91 percent or higher). Along State Route 58 east of State Route 99, all but two freeway segments will operate at LOS D or better. Bottleneck conditions along State Route 99 will affect traffic flows to and from State Route 58 east. As with the AM conditions, PM LOS F conditions are expected along State Route 99 in the northbound direction from White Lane to State Route 58. Southbound, four segments are expected to operate at LOS E and two segments will operate at LOS F. The volume of traffic served will be significantly lower than the demand volume in the northbound direction as of year 2038 (below 90 percent).

In addition to analyzing freeway mainline segments, freeway merging/diverging conditions and on-ramp and off-ramp performance of the freeways were analyzed using the same CORSIM traffic simulation model. These results are reported in Tables 4-10 through 4-13. In cases where a low percentage of on-ramp traffic is served, such as the White Lane loop on-ramp to northbound State Route 99, traffic would be expected to spill back onto local streets until such time that motorists divert to alternate routes.

Traffic level of service at ramp termini and study area intersections are reported in Table 4-14 for no-build conditions. The locations of the study intersections are illustrated on Figure 4-3 and the proposed traffic lane geometric configurations and demand traffic volumes are illustrated on Figure 3-8. For the ramp terminal and other study intersections reported in Table 4-14, under year 2038 conditions, all intersections perform at LOS D or better except for 11 of 79 study intersections during the AM peak hour and 22 locations during the PM peak hour (9 with LOS E and 13 with LOS F).

These conditions are expected, due in part to freeway mainline constraints spilling back onto the arterial street system, as well as the overall intensity of land development located at the nodes of high volume freeway interchanges with arterial streets.

Detailed level of service input parameters and output results are provided in Volume II (part 1 of 2) of this report.

Table 4-8. AM Peak Hour Measures of Effectiveness for the No-Build Alternative—Freeway Mainline Segments

							YEA	R 2018												YEAR :	2038					
	DEMAND	SERVED	%	TRAVEL TIME	DELAY TIME	SPEED	DENSITY	LOS (HCM	CUMULATIVE TIME	VEHICLE MILES	VEHICLE HOURS	VEHICLE DELAY	CORSIM SERVED VOLUMES/ LANES	DEMAND	SERVED	%	TRAVEL TIME	DELAY TIME	SPEED	DENSITY	LOS (HCM	CUMULATIVE TIME	VEHICLE MILES	VEHICLE HOURS	VEHICLE DELAY	CORSIM SERV VOLUMES/
FREEWAY SEGMENT	(vph)	(vph)	SERVED	(sec/veh)	(sec/veh)	(mph)	(veh/ln/mi)	2000)	(sec/veh)	(VMT)	(VHT)	(VHD)	(vphl)	(vph)	(vph)	SERVED	(sec/veh)	(sec/veh)	(mph)	(veh/ln/mi)	2000)	(sec/veh)	(VMT)	(VHT)	(VHD)	LANES (vp
tate Route 99 NB Mainline																										
R 99 NB south end of the network to White Lane off-ramp	4,940 4,260	4,940	100% 100%	50 19	1	63	20	С	50 60	4346 1420	69	2	1237 1061	7,025 5,960	6,570	94%	86 30	36	38 41	44 34	E	86	5886	156 46	66 17	1643 1382
Vhite Lane off-ramp to White Lane loop on-ramp Vhite Lane loop on-ramp to White Lane direct on-ramp	4,260 5,850	4,242 5,156	100% 88%	19 7	1	63 56	17 23	B C	69 76	1420 542	23 10	1	1061 1289	5,960 7,565	5,527 5,850	93% 77%	30 14	11 8	41 28	34 53	D F	115 129	1850 615	46 22	17 13	1382 1463
White Lane diagonal on-ramp to Ming Avenue off-ramp	6,540	5,977	91%	76	5	60	25	С	152	7566	126	9	1494	8,260	6,159	75%	197	125	23	66	F	326	7838	337	214	1540
Ming Avenue off-ramp to Ming Avenue on-ramp	6,025	5,519	92%	34	10	51	28	D	185	2658	52	15	1380	7,470	5,204	70%	85	59	18	72	F	411	2232	123	85	1301
Ming Avenue on-ramp to SR 58 EBWible Road off-ramp SR 58 EBWible Road off-ramp to Wible Road on-ramp	7,345 5,240	6,867 4,775	93% 91%	19 32	8 2	33 62	45 19	F C	204 236	1205 2614	36 42	16 2	1717 1194	9,055 6,230	6,807 4,624	75% 74%	20 32	2	32 61	47 19	F C	431 463	1194 2531	37 41	17 2	1702 1156
Wible Road on-ramp to SR 58 WB on-ramp	5,830	5,272	90%	9	0	61	21	č	246	843	14	1	1318	6,870	5,135	75%	9	1	61	21	č	472	820	13	1	1284
SR 58 WB on-ramp to California Avenue off-ramp	7,115	6,551	92%	35	4	59	27	D	281	3753	64	6	1638	8,655	6,950	80%	36	4	58	29	D	508	4020	69	8	1738
California Avenue off-ramp to California Avenue loop on-ramp California Avenue loop on-ramp to California Avenue diagonal on-ramp	5,970 6,505	5,524 6,007	93% 92%	16 9	1	62 61	22 25	C	297 306	1560 893	25 15	1	1381 1502	7,475 8,040	5,982 6,396	80% 80%	16 9	1	62 60	24 26	C	525 534	1689 951	27 16	1	1496 1599
California Avenue diagonal on-ramp to Rosedale Highway off-ramp	6,740	5,959	88%	32	2	60	23	Č	338	3128	52	3	1324	8,280	6,349	77%	32	2	60	25	C	565	3333	56	3	1411
Rosedale Highway off-ramp to Buck Owens Boulevard/Sillect Avenue off-ramp	4,590	4,217	92%	17	1	62	17	В	354	1198	19	1	1054	5,840	4,599	79%	17	1	62	19	С	582	1307	21	1	1150
Buck Owens Boulevard/Sillect Avenue off-ramp to Buck Owens Boulevard/Sillect Avenue on-ramp Buck Owens Boulevard/Sillect Avenue on-ramp to Airport Drive off-ramp	3,765 4,150	3,458 3,824	92% 92%	24 33	1	63 61	14 16	B B	378 411	1437 2173	23 35	1	865 956	5,015 5,615	3,917 4,451	78% 79%	24 34	1	63 61	16 18	B C	606 639	1628 2531	26 42	1	979 1113
Airport Drive off-ramp to SR 99 NB north end of the network	2,615	2,411	92%	40	1	63	9	A	451	1702	27	1	603	3,865	3,168	82%	40	1	63	13	В	680	2237	35	1	792
State Route 99 SB Mainline						•					•													•		
CR 99 SB north end of the network to Airport Drive on-ramp	3,195	3,195	100%	48	1	64	13	В	48	2741	43	1	804	3,895	3,895	100%	48	1	64	15	В	48	3342	52	1	980
Airport Drive on-ramp to Rosedale Highway off-ramp	4,460	4,455	100%	31	2	61	18	С	79 104	2379	39	2	1114	5,320	5,320	100%	32	3	60	22	С	80	2859	48	4	1332
Rosedale Highway off-ramp to Rosedale Highway loop on-ramp Rosedale Highway loop on-ramp to Rosedale Highway diagonal on-ramp	3,800 4,700	3,800 4,700	100% 100%	25 9	1 1	63 58	15 18	B B	104 113	1637 719	26 12	1 1	952 1187	4,245 5,395	4,245 5,395	100% 100%	25 9	1	63 59	17 19	B C	105 114	1848 833	30 14	1	1074 1375
Rosedale Highway diagonal on-ramp to California Avenue off-ramp	5,880	5,880	100%	38	4	58	25	С	152	3608	63	7	1476	7,085	7,085	100%	40	6	55	30	D	155	4460	81	13	1798
California Avenue off-ramp to California Avenue on-ramp	4,775	4,770	100%	13	1	62	19	С	165	1111	18	1	1193	5,885	5,885	100%	14	1 5	62	23	С	168	1379	22	1	1482
California Avenue on-ramp to SR 58 EB/Stockdale Highway off-ramp SR 58 EB/Stockdale Highway off-ramp to SR 58 WB on-ramp	5,200 3,555	5,179 3,555	100% 100%	28 40	2	59 63	21 14	C B	193 233	2388 2504	40 40	3	1295 890	6,605 4,715	6,605 4,715	100% 100%	30 41	5 2	55 62	27 18	D B	199 239	3076 3326	56 53	8 2	1657 1182
SR 58 WB on-ramp to Real Road on-ramp	4,840	4,484	93%	5	1	51	22	C	238	323	6	1 1	1121	6,265	5,829	93%	6	2	52	34	D	245	420	10	4	1457
Real Road on-ramp to Ming Avenue off-ramp	5,230	5,110	98%	23	2	58	20	С	261	1877	32	3	1277	6,775	6,721	99%	24	4	54	28	D	270	2469	45	7	1680
Ming Avenue off-ramp to Ming Avenue on-ramp Ming Avenue on-ramp to White Lane off-ramp	4,230 4,725	4,106 4,598	97% 97%	35 60	1 3	63 62	16 18	B B	296 356	2525 4714	40 76	1 3	1026 1150	5,545 6,235	5,410 6,043	98% 97%	36 60	1 3	62 61	21 24	C	305 366	3327 6198	53 101	2 6	1353 1511
White Lane off-ramp to White Lane loop on-ramp	3,360	3,248	97%	19	0	63	13	В	375	1089	17	0	812	4,740	4,628	98%	19	1	63	18	Č	385	1552	25	1	1157
White Lane loop on-ramp to White Lane diagonal on-ramp	3,540	3,411	96%	10	0	63	14	В	385	577	9	0	853	4,950	4,828	98%	10	0	62	19	C	395	818	13	1	1207
White Lane diagonal on-ramp to SR 99 SB south end of the network	3,890	3,581	92%	52	1	63	14	В	436	3239	51	1	895	5,410	4,952	92%	52	2	62	20	С	447	4478	72	3	1238
Nestside Parkway EB Mainline																							1			
Westside Parkway west end of the network to West Beltway off-ramp West Beltway off-ramp to West Beltway loop on-ramp							— NOT AP	DI ICABI I	-					2,190 1,390	2,190 1,341	100% 96%	6 14	2	47 61	24 11	C B	6 20	183 315	4	1 0	1117 671
West Beltway loop on-ramp to West Beltway diagonal on-ramp							-NOT AF	FLICABLE	-					2,574	2,518	98%	9	1	61	14	В	29	393	6	0	839
West Beltway diagonal on-ramp to Allen Road off-ramp	835	779	93%	55	1	64	4	Α	55	766	12	0	260	3,124	3,031	97%	56	2	63	16	В	86	2981	48	1	1010
Allen Road off-ramp to Allen Road on-ramp	770	740	96%	115	3	62	5	A	170	1461	24	1	247	2,929	2,842	97%	33	1	63	15	В	118	1618	26	1	947
Allen Road on-ramp to Calloway Drive off-ramp Calloway Drive off-ramp to Calloway Drive loop on-ramp	2,195 1,690	2,133 1,681	97% 99%	58 24	2	63 64	11 9	B A	228 252	2160 728	34 11	0	711 560	4,739 3,631	4,642 3,583	98% 99%	61 25	5 1	60 63	26 19	C	179 204	4720 1553	78 25	6 1	1547 1194
Calloway Drive loop on-ramp to Calloway Drive diagonal on-ramp	2,155	2,080	97%	11	Ö	63	11	В	263	394	6	Ö	693	4,115	4,034	98%	11	1	62	22	Č	215	764	12	1	1345
Calloway Drive diagonal on-ramp to Coffee Road off-ramp	3,210	3,148	98%	49	3	61	17	В	312	2616	43	3	1049	5,029	4,989	99%	51	5	59	28	D	266	4154	71	7	1663
Coffee Road off-ramp to Coffee Road loop on-ramp Coffee Road loop on-ramp to Coffee Road diagonal on-ramp	1,985 2,300	1,921 2,212	97% 96%	23 11	0	64 64	10 9	A A	335 345	768 419	12 7	0	640 553	2,844 3,229	2,844 3,229	100% 100%	23 11	1	64 63	15 13	B B	289 299	1154 620	18 10	0 0	962 819
Coffee Road diagonal on-ramp to Mohawk Street off-ramp	2,655	2,544	96%	53	1	63	10	Ä	399	2392	38	1	636	3,779	3,779	100%	54	2	63	15	В	354	3578	57	2	951
Westside Parkway WB Mainline																										
Mohawk Street loop on-ramp to Mohawk Street diagonal ramp	1,730	1,730	100%	14	1	63	9	Α	40	431	7	0	581	1,957	1,957	100%	14	1	63	11	Α	40	487	8	0	657
Mohawk Street diagonal ramp to Coffee Road off-ramp	2,450	2,370	97%	42	1	62	9	A	82	1728	28	1	527	2,962	2,919	99%	42	1	62	11	В	82	2131	34	1	834
Coffee Road off-ramp to Coffee Road loop on-ramp Coffee Road loop on-ramp to Coffee Road diagonal on-ramp	1,325 1,510	1,264 1,426	95% 94%	35 12	0	64 64	6	A A	117 130	791 311	12 5	0	316 356	1,777 2,332	1,757 2,225	99% 95%	35 12	0	64 63	/ q	A	117 130	1100 485	17 8	0	586 742
Coffee Road diagonal on-ramp to Calloway Drive diagonal off-ramp	1,740	1,658	95%	47	1	64	7	A	177	1392	22	ő	415	2,977	2,840	95%	48	2	63	12	В	178	2386	38	1	710
Calloway Drive diagonal off-ramp to Calloway Drive loop off-ramp	1,545	1,450	94%	11	0	63	8	A	188	274	4	0	363	2,665	2,550	96%	11	0	62	14	В	189	483	8	0	638
Calloway Drive loop off-ramp to Calloway Drive on-ramp Calloway Drive on-ramp to Allen Road off-ramp	1,115 1,355	1,066 1,322	96% 98%	24 56	0	64 64	б 7	A	212 268	456 1315	7 21	0	355 441	2,085 2,550	1,996 2.446	96% 96%	24 57	0	64 63	10 13	A B	213 270	853 2432	13 39	0	665 815
Allen Road off-ramp to Allen Road on-ramp	605	605	100%	26	Ö	64	3	A	295	292	5	0	208	1,700	1,643	97%	26	ō	64	9	Ā	296	770	12	Ö	548
Allen Road on-ramp to West Beltway off-ramp	705	705	100%	89	1	64	5	Α	383	1168	18	0	246	1,925	1,864	97%	56	1	63	10	A	353	1840	29	1	621
West Beltway off-ramp to West Beltway loop on-ramp West Beltway loop on-ramp to West Beltway diagonal on-ramp							— NOT AP	PLICARLI	=_					865 1,260	825 1,158	95% 92%	22 6	0	64 62	6 a	A A	375 381	328 115	5 2	0 0	413 579
West Beltway diagonal on-ramp to West Beltway diagonal on-ramp West Beltway diagonal on-ramp to Westside Parkway west end of the network							NOTAP	LICADLI						1,550	1,156	94%	6	0	60	9 12	В	387	140	2	0	726
State Route 58 EB Mainline																										
SR 58 west end of the network to SR 99 on-ramp	1,195	1,195	100%	42	3	56	11	В	42	823	15	1	625	1,345	1,345	100%	42	3	56	12	В	42	890	16	1	1352
SR 99 on-ramp to H Street off-ramp	3,960	3,960	100%	10	1	59	23	С	52	655	11	1	1321	4,850	4,300	89%	10	1	58	25	С	52	710	12	1	1433
H Street off-ramp to Chester Avenue on-ramp	3,475 4,030	3,475 3,993	100% 99%	36 27	1 2	62 60	19 22	C	88 115	2163 1833	35 30	1 2	1166 1331	4,305 4,900	3,815 4,494	89% 92%	36 28	2 2	62 59	21 25	C	88 116	2359 2065	38 35	2	1272 1498
Chester Avenue on-ramp to Union Avenue off-ramp Jnion Avenue off-ramp to Union Avenue loop on-ramp	4,030 3,175	3.168	100%	27 17	1	63	17	В	115 132	1833 920	30 15	0	1331 1056	4,900 3,985	4,494 3,718	92%	28 17	1	59 62	25 20	C	116 133	1080	35 17	ა 1	1498 1239
Jnion Avenue loop on-ramp to Union Avenue diagonal on-ramp	3,420	3,388	99%	8	Ö	62	18	С	140	474	8	0	1129	4,285	3,969	93%	8	Ö	62	22	Č	141	556	9	ó	1323
Jnion Avenue diagonal on-ramp to Cottonwood Road off-ramp	3,650	3,641	100%	31	1	62	20	С	171	1934	31	1	1214	4,560	4,249	93%	31	2	61	23	С	172	2257	37	2	1416
Cottonwood Road off-ramp to Cottonwood Road on-ramp Cottonwood Road on-ramp to SR 58 east end of the network	3,190 3,420	3,190 3,420	100% 100%	28 25	1 1	63 62	17 18	B C	199 224	1546 1496	25 24	1 1	1066 1143	4,010 4.315	3,751 4,052	94% 94%	28 25	1 1	62 62	20 22	C	200 226	1814 1768	29 29	1	1250 1351
State Route 58 WB Mainline	0,720	0,720	10070	20	<u>'</u>	, J2			<u> </u>	1-1-00			1170	7,010	7,002	J-70			<u> </u>				1700			1001
SR 58 east end of the network to Cottonwood Road off-ramp	3,600	3,600	100%	26	1	64	19	С	26	1646	26	1	1206	4,870	4,870	100%	26	1	63	26	D	26	2227	35	1 1	1633
Cottonwood Road off-ramp to Cottonwood Road on-ramp	3,355	3,336	99%	18	1	63	18	В	43	1025	16	Ö	1112	4,615	4,615	100%	18	1	62	25	C	44	1423	23	i	1543
Cottonwood Road on-ramp to Brundage Lane off-ramp	3,585	3,550	99%	38	1	63	19	С	81	2349	38	1	1183	4,855	4,855	100%	39	2	61	27	D	83	3218	53	3	1622
	3,055	3,035	99% 99%	17 12	1	63 62	16 17	B B	98 110	896 664	14 11	1 0	1012 1080	3,975 4,195	3,931 4,123	99% 98%	17 12	1	62 61	21 23	C	100 112	1160 845	19 14	1	1310 1374
Brundage Lane off-ramp to Brundage Lane on-ramp	2 270					02	1.7	D	110	004	1 11	U	1080	4.190	4.123	90%	12		O I	4.3						13/4
Srundage Lane on-ramp to Union Avenue on-ramp Brundage Lane on-ramp to Union Avenue on-ramp Union Avenue on-ramp to Chester Avenue off-ramp	3,270 3,615	3,239 3,555	98%	23	1	61	19	С	133	1385	23	1	1185	4,595	4,505	98%	23	2	60	25	č	136	1754	29	2	1502
Brundage Lane on-ramp to Union Avenue on-ramp Union Avenue on-ramp to Chester Avenue off-ramp Chester Avenue off-ramp to H Street on-ramp	3,615 3,010	3,555 2,990	98% 99%	23 39	1 1	61 63	16	В	172	2026	32	1 1	997	4,595 3,860	4,505 3,778	98%	23 39	2 2	60 62	25 20	C C	136 175	1754 2561	29 41	2	1259
Brundage Lane on-ramp to Union Avenue on-ramp Union Avenue on-ramp to Chester Avenue off-ramp	3,615	3,555	98%	23	1 1 2	61						1 1 2		4,595	4,505		23		60	25	С	136	1754	29		





Table 4-9. PM Peak Hour Measures of Effectiveness for the No-Build Alternative—Freeway Mainline Segments

Table 4-9. PM Peak Hour Measures of Effectiveness	101 (140	Jane	a Aitel	·iative	110		R 2018		gc.iit										YEAR 2	2038					
				TRAVEL	DELAY			LOS	CUMULATIVE	VEHICLE	VEHICLE	VEHICLE	CORSIM SERVED				TRAVEL	DELAY			LOS	CUMULATIVE	VEHICLE		VEHICLE	CORSIM SERVED
FREEWAY SEGMENT	DEMAND (vph)	SERVED (vph)	% SERVED	TIME (sec/veh)	TIME (sec/veh)	SPEED (mph)	DENSITY (veh/ln/mi)	(HCM 2000)	TIME (sec/veh)	MILES (VMT)	HOURS (VHT)	DELAY (VHD)	VOLUMES/LANES (vphl)	DEMAND (vph)	SERVED (vph)	% SERVED	TIME (sec/veh)	TIME (sec/veh)	SPEED (mph)	DENSITY (veh/ln/mi)	(HCM 2000)	TIME (sec/veh)	MILES (VMT)	HOURS (VHT)	DELAY (VHD)	VOLUMES/LANES (vphl)
State Route 99 NB Mainline	7 - 1 /																									
SR 99 NB south end of the network to White Lane off-ramp	4,590	4,604	100%	50	1	63	18	С	50	4043	64	2	1151	6,580	6,570	100%	52	4	60	28	D	52	5698	95	6	1642
White Lane off-ramp to White Lane loop on-ramp White Lane loop on-ramp to White Lane direct on-ramp	3,905 5,360	3,899 5,139	100% 96%	19 7	1	63 57	15 23	B C	69 76	1305 540	21 10	1	975 1285	5,485 7,130	5,360 6,226	98% 87%	21 8					73 81	1795 655	31 14	3 4	1340 1557
White Lane diagonal on-ramp to Ming Avenue off-ramp	5,915	5,875	99%	75	5	61	24	Č	151	7434	123	8	1469	7,740	6,805	88%	152	81	31	57 57	F	233	8911	287	153	1701
Ming Avenue off-ramp to Ming Avenue on-ramp Ming Avenue on-ramp to SR 58 EB/Wible Road off-ramp	5,290 6,535	5,290 6,535	100% 100%	28 17	4 6	58 38	23 38	C E	179 195	2373 1151	41 30	6 11	1331 1640	6,920 8,415	5,857 7.374	85% 88%	76 19					309 328	2515 1293	124 38	83 16	1464 1844
SR 58 EB/Wible Road off-ramp to Wible Road on-ramp	4,630	4,630	100%	32	1	62	19	C	227	2554	41	2	1166	5,915	5,185	88%	32	2	61	21	С	360	2838	47	3	1296
Wible Road on-ramp to SR 58 WB on-ramp SR 58 WB on-ramp to California Avenue off-ramp	5,240	5,190	99%	9 35	1 3	61	21	C D	237 272	830 3797	14 64	1 6	1298 1653	6,525 8,240	5,683	87% 90%	9 36	1	61	23	С	370 405	908 4260	15 73	1 8	1421 1846
California Avenue off-ramp to California Avenue loop on-ramp	6,670 5,900	6,611 5,832	99% 99%	35 16	1	59 62	27 24	C	288	1647	27	1	1458	7,360	7,383 6,598	90%	36 17	1	61	27	D	405 422	1863	30	2	1650
California Avenue loop on-ramp to California Avenue diagonal on-ramp	6,590	6,435	98%	9	1	60	27	D C	297	957	16	1 4	1609	8,100	7,058	87%	9	1	60	29	D	431	1049	17	1	1765 1561
California Avenue diagonal on-ramp to Rosedale Highway off-ramp Rosedale Highway off-ramp to Buck Owens Boulevard/Sillect Avenue off-ramp	6,910 5,060	6,507 4,889	94% 97%	32 16	2 1	59 62	26 20	C	329 345	3397 1389	58 22	1	1446 1222	8,430 6,340	7,026 5,365	83% 85%	32 17	3 1	58 62	28 22	С	463 480	3654 1524	63 25	1	1341
Buck Owens Boulevard/Sillect Avenue off-ramp to Buck Owens Boulevard/Sillect Avenue on-ramp	4,660	4,469	96%	24	1	63	18	В	369	1857	30	1	1117	5,790	4,923	85%	24	1	62	20	С	504	2046	33	1	1231
Buck Owens Boulevard/Sillect Avenue on-ramp to Airport Drive off-ramp Airport Drive off-ramp to SR 99 NB north end of the network	5,280 3,935	5,065 3,764	96% 96%	34 40	1	60 63	21 15	C B	403 443	2879 2657	48 42	1	1266 941	6,540 5,125	5,607 4.390	86% 86%	34 41	2		24 18	B	538 579	3187 3100	53 50	2	1402 1097
State Route 99 SB Mainline	0,000	0,.0.	0070			- 55				200.			<u> </u>	0,120	1,000	0070			02			0.0	0.00			
SR 99 SB north end of the network to Airport Drive on-ramp	3,520	3,520	100%	48	1	64	14	В	48	3013	47	1	883	4,595	4,595	100%	49	TIME (sec/veh) SPEED DENSITY (HCM (sec/veh) (mph) (veh/ln/mi) 2000) 4 60 28 D 2 59 23 C 2 47 33 D 81 31 57 F 51 20 73 F 8 34 48 F 2 61 21 C 1 61 23 C 4 58 31 D 1 61 27 D 1 60 29 D 3 58 28 D 1 62 22 C 1 62 20 C 3 60 24 C		49	3957	63	2	1155		
Airport Drive on-ramp to Rosedale Highway off-ramp Rosedale Highway off-ramp to Rosedale Highway loop on-ramp	5,275 4,600	5,275 4,600	100% 100%	32 25	3	60 62	22 18	C	80 105	2836 1982	47 32	4	1326 1152	6,710 5,615	6,412 5,417	96% 96%	46 26	DELAY TIME SPEED (sec/veh) (mph) DENSITY (HCM 2000)		96 121	3440 2329	83 39	30 3	1603 1354		
Rosedale Highway loop on-ramp to Rosedale Highway diagonal on-ramp	5,905	5,905	100%	25 12	3	46	28	D	116	916	20	5	1511	6,995	6,891	99%	12	DELAY TIME SPEED (mph) (mph)		133	1044	22	6	1723		
Rosedale Highway diagonal on-ramp to California Avenue off-ramp	7,240	7,240	100%	41	7	55	33	D	157	4661	85	15	1867	8,620	8,240	96%	43	9	SPEED		176	5035 1647	97	20	2060	
California Avenue off-ramp to California Avenue on-ramp California Avenue on-ramp to SR 58 EB/Stockdale Highway off-ramp	6,155 7,080	6,155 7,080	100% 100%	14 35	9	59 49	27 36	D E	172 207	1483 3450	25 71	2 19	1593 1804	7,430 8,480	7,076 8,048	95% 95%	21 39	Ü				197 235	1647 3989	41 86	16 28	1769 2012
SR 58 EB/Stockdale Highway off-ramp to SR 58 WB on-ramp	5,105	5,105	100%	41	2	62	21	С	248	3623	58	3	1288	6,385	6,089	95%	46	7				281	4282	78	12	1522
SR 58 WB on-ramp to Real Road on-ramp Real Road on-ramp to Ming Avenue off-ramp	6,535 7,005	6,105 6,961	93% 99%	7 26	3 5	39 52	39 32	E D	254 280	440 2564	11 50	5 10	1526 1740	8,140 8,795	7,219 8,258	89% 94%	10 32	_	26 42			291 323	521 3063	20 74	12 26	1805 2065
Ming Avenue off-ramp to Ming Avenue on-ramp	5,380	5,346	99%	36	1	62	21	C	316	3287	53	2	1337	7,050	6,578	93%	36	2	61	27	D	360	4048	67	4	1644
Ming Avenue on-ramp to White Lane off-ramp White Lane off-ramp to White Lane loop on-ramp	6,025 3,995	6,018 3,995	100% 100%	61 19	4	61 63	24 16	C B	377 396	6184 1340	102 21	7	1504 999	7,885 5,855	7,452 5,522	95% 94%	62 20	5 1				422 441	7664 1851	128 30	11	1863 1381
White Lane loop on-ramp to White Lane diagonal on-ramp	4,490	4,446	99%	10	Ö	62	18	В	406	753	12	1	1112	6,430	6,018	94%	10	1	61	25	С	451	1019	17	1	1505
White Lane diagonal on-ramp to SR 99 SB south end of the network	4,940	4,739	96%	52	2	62	19	С	458	4284	69	3	1185	7,105	6,430	90%	53	3	61	26	D	505	5803	95	5	1608
Westside Parkway EB Mainline														0.400	0.455	000/		0	I	0.4		7	170			4070
Westside Parkway west end of the network to West Beltway off-ramp West Beltway off-ramp to West Beltway loop on-ramp							— NOT AF	PLICABL	E—					2,180 1,225	2,155 1,130	99% 92%	7 14	1			A	21	176 265	4	0	1078 565
West Beltway loop on-ramp to West Beltway diagonal on-ramp											1		1	2,100	1,985	95%	9	ŭ			Α	30	310	5	0	662
West Beltway diagonal on-ramp to Allen Road off-ramp Allen Road off-ramp to Allen Road on-ramp	830 745	783 721	94% 97%	83 32	0	62 65	6	A	83 114	1107 410	18 6	0	261 240	2,245 2,065	2,120 1,967	94% 95%	56 32	2 1			_	86 118	2085 1120	33 18	1 0	707 656
Allen Road on-ramp to Calloway Drive off-ramp	1,840	1,812	98%	58	2	63	10	Α	172	1836	29	1	604	3,675	3,524	96%	59	3	61		_	178	3571	58	3	1175
Calloway Drive off-ramp to Calloway Drive loop on-ramp Calloway Drive loop on-ramp to Calloway Drive diagonal on-ramp	1,405 1,745	1,405 1,710	100% 98%	24 11	0	64 63	7 9	A	197 208	612 324	10 5	0	471 570	2,746 3,127	2,627 2,974	96% 95%	25 11	1 0			_	202 213	1138 563	18 9	0	876 991
Calloway Drive diagonal on-ramp to Coffee Road off-ramp	2,605	2,567	99%	48	2	62	13	В	256	2133	34	2	856	3,889	3,706	95%	49	ŭ	61	20	_	262	3074	51	3	1235
Coffee Road off-ramp to Coffee Road loop on-ramp Coffee Road loop on-ramp to Coffee Road diagonal on-ramp	1,725 2,140	1,716 2,118	99% 99%	23 11	0	64 63	9	Α	278 289	686 401	11	0	572 530	2,184 2,614	2,058 2,469	94% 94%	22 11	0			Α Δ	285 295	823 468	13	0	686 617
Coffee Road diagonal on-ramp to Mohawk Street off-ramp	2,600	2,551	98%	54	1	63	10	Ä	343	2399	38	1	638	3,089	2,920	95%	54	1		11	В	349	2747	44	1	730
Westside Parkway WB Mainline																										
Mohawk Street loop on-ramp to Mohawk Street diagonal ramp	2,605	2,435	93%	14	1	62	13	В	40	602	10	0	812	3,170	2,973	94%	14	1		16	В	40	735	12	0	991
Mohawk Street diagonal ramp to Coffee Road off-ramp Coffee Road off-ramp to Coffee Road loop on-ramp	3,850 2,565	3,502 2,282	91% 89%	42 35	1 0	62 64	13 9	В	83 118	2558 1430	41 22	1 0	778 571	4,585 3,370	4,160 3,199	91% 95%	42 35	1		16 13	B	83 118	3039 2004	49 31	2	1189 1066
Coffee Road loop on-ramp to Coffee Road diagonal on-ramp	2,855	2,533	89%	12	0	64	10	A	130	552	9	0	633	4,350	4,109	94%	13	1	62	17	В	131	895	14	1	1370
Coffee Road diagonal on-ramp to Calloway Drive diagonal off-ramp Calloway Drive diagonal off-ramp to Calloway Drive loop off-ramp	3,420 2,915	3,067 2,598	90% 89%	48 11	2 0	63 61	13 14	B B	178 190	2587 492	41 8	2	767 650	5,375 4,865	5,148 4,614	96% 95%	50 11	3				180 191	4342 874	71 14	4	1287 1154
Calloway Drive loop off-ramp to Calloway Drive on-ramp	1,810	1,653	91%	24	ő	64	9	Α	214	707	11	ő	551	3,850	3,670	95%	25	1	63	20	С	216	1570	25	i	1223
Calloway Drive on-ramp to Allen Road off-ramp Allen Road off-ramp to Allen Road on-ramp	2,360	2,225	94%	58 26	2 0	62 64	11 4	B A	272	2220 373	36 6	1 0	742 266	4,835 2,955	4,628 2,825	96% 96%	59 27	4		0	C	275 302	4612 1323	76 21	5	1543 942
Allen Road on-ramp to West Beltway off-ramp	795 865	795 865	100% 100%	89	1	64 64	6	Â	298 386	1377	21	0	291	3,285	3,138	96%	57	2	62		В	359	3098	50	2	1046
West Beltway off-ramp to West Beltway loop on-ramp West Beltway loop on-ramp to West Beltway diagonal on-ramp							NOT 45	D. 10451	_					1,410 1,955	1,363 1,831	97% 94%	23 6	0			A	382 388	542 183	9	0	682 916
West Beltway diagonal on-ramp to West side Parkway west end of network							— NOT AF	'PLICABL	.E —					1,955 2,305	2,181	94% 95%	6	1			C	388 394	209	4	0	1091
State Route 58 EB Mainline																										
SR 58 west end of the network to SR 99 on-ramp	1,085	1,083	100%	42	2	56	10	Α	42	712	13	1	542	1,195	1,039	87%	42	2	57	9	Α	42	683	12	1	1039
SR 99 on-ramp to H Street off-ramp H Street off-ramp to Chester Avenue on-ramp	3,920 3,480	3,920 3,480	100% 100%	10 36	1 1	58 62	23 19	C	52 88	655 2181	11 35	1 1	1321 1176	4,585 4,110	4,138 3,701	90% 90%	10 36	1			C	52 88	684 2288	12 37	1	1379 1234
Chester Avenue on-ramp to Union Avenue off-ramp	4,165	4,149	100%	27	2	60	23	č	115	1904	32	2	1383	4,950	4,535	92%	28	2	60	25	č	115	2083	35	3	1512
Union Avenue loop on-ramp to Union Avenue loop on-ramp	3,570 3,985	3,567	100% 97%	17 8	1	62 61	19 21	C	132 140	1035 542	17 9	1 0	1189 1291	4,305 5,075	3,967 4,555	92% 90%	17 8	1				132 140	1152 637	19 11	1	1322 1518
Union Avenue loop on-ramp to Union Avenue diagonal on-ramp Union Avenue diagonal on-ramp to Cottonwood Road off-ramp	4,300	3,873 4,217	98%	8 31	2	61	21 23	C	171	2241	36	2	1406	5,075 5,710	4,555 5,226	90% 92%	8 32	2				172	2785	46	3	1518
Cottonwood Road off-ramp to Cottonwood Road on-ramp	3,890	3,794	98%	28	1	62 62	20	C	199	1835	30 29	1	1265	5,110	4,698	92%	28	1	62	25		201	2272	37 37	2	1566
Cottonwood Road on-ramp to SR 58 east end of the network State Route 58 WB Mainline	4,230	4,123	97%	25	<u> </u>	62	22	L	225	1799		<u> </u>	1374	5,595	5,169	92%	26	2	וס	28	טן	226	2255	31		1723
SR 58 east end of the network to Cottonwood Road off-ramp	3,490	3,490	100%	26	1 1	64	18		26	1597	25	I 0	1171	4,090	4,090	100%	26	1	63	22	ا م ا	26	1873	30	1	1373
Cottonwood Road off-ramp to Cottonwood Road on-ramp	3,285	3,268	99%	18	0	63	17	В	43	1004	16	0	1089	3,850	3,830	99%	18	1	63	20		44	1176	19	1	1277
Cottonwood Road on-ramp to Brundage Lane off-ramp Brundage Lane off-ramp to Brundage Lane on-ramp	3,665 3,170	3,659 3,147	100% 99%	38 17	2	62 62	20 17	C B	82 99	2421 929	39 15	2	1220 1049	4,250 3,655	4,226 3,655	99% 100%	39 17	2				82 99	2797 1081	45 17	2	1409 1221
Brundage Lane on-ramp to Union Avenue on-ramp	3,525	3,147	98%	12	1	62	19	C	111	707	11	1	1151	4,050	4,008	99%	17	1			-	99 111	821	17	1	1336
Union Avenue on-ramp to Chester Avenue off-ramp	4,055	3,906	96%	23	1	61	21	C	134	1522	25	1	1302	4,605	4,527	98%	23			25	-	135	1764	29	2	1509
Chester Avenue off-ramp to H Street on-ramp H Street on-ramp to SR 99 NB off-ramp	3,440 3,945	3,352 3,785	97% 96%	39 27	1 2	62 60	18 21	B C	173 200	2272 1688	36 28	1 2	1117 1262	3,960 4,555	3,850 4,323	97% 95%	55 42				E E	190 231	2719 1930	59 50	18 20	1283 1441
SR 99 NB off-ramp to SR 99 SB off-ramp	2,515	2,423	96%	25	1	60	14	В	225	1002	17	1	808	2,840	2,677	94%	70			40	E	301	1191	52	35	892
Bottleneck																										



Table 4-10. AM Peak Hour Measures of Effectiveness for the No-Build Alternative— Merging/Diverging Conditions

morging/21torging con-			YEAR 201	R		YEAR 203	8
	ANALYSIS	SPEED	DENSITY	LOS	SPEED	DENSITY	Los
LOCATION	TYPE	(mph)	(veh/ln/mi)	(HCM 2000)	(mph)	(veh/ln/mi)	(HCM 2000)
State Route 99 NB							
White Lane loop on-ramp	Merging	51	27.6	C	16	54.7	E
White Lane diagonal on-ramp Ming Avenue on to SR 58/Wible Road off	Merging	53	32.1 64.8	D F	16 22	66.4 61.0	F F
Wible Road on-ramp	Merging Merging	26 60	21.2	C	61	16.1	B
SR 58 WB on-ramp	Merging	54	32.1	Ď	50	34.1	Ď
California Avenue loop on-ramp	Merging	59	28.3	D	59	28.7	D
California Avenue diagonal on-ramp	Merging	58	28.3	D	58	29.3	D
Buck Owens Boulevard/Sillect Avenue on-ramp	Merging	61	15.7	В	61	18.5	В
White Lane off-ramp Ming Avenue off-ramp	Diverging	62	21.3	С	28	48.3	E F
Ming Avenue on to SR 58/Wible Road off	Diverging Diverging	60 26	28.4 64.8	D F	11 22	76.0 61.0	F
California Avenue off-ramp	Diverging	58	35.2	Ē	58	34.8	D
Rosedale Highway off-ramp	Diverging	58	24.0	С	58	25.0	С
Buck Owens Boulevard/Sillect Avenue off-ramp	Diverging	60	19.2	В	60	20.1	С
Airport Drive off-ramp	Diverging	58	21.7	С	57	24.7	С
State Route 99 SB							
Airport Drive on-ramp	Merging	60	22.4	С	58	27.7	С
Rosedale Highway loop on-ramp	Merging	55	21.1	C	56	21.1	c
Rosedale Highway diagonal on-ramp California Avenue on-ramp	Merging Merging	54 57	35.3 29.5	E D	51 50	42.9 39.3	E E
SR 58 WB on-ramp	Merging	57 47	29.5 33.7	D	36	39.3 53.3	E
Real Road on-ramp	Merging	55	27.6	Č	48	38.9	Ē
Ming Avenue on-ramp	Merging	60	22.6	C	59	28.7	D
White Lane loop on-ramp	Merging	61	15.5	В	61	20.3	С
White Lane diagonal on-ramp	Merging	61	16.4	В	61	21.4	С
Rosedale Highway off-ramp	Diverging	61	23.8	C	59	29.8	D
California Drive off-ramp SR 58 EB/Stockdale Highway off-ramp	Diverging Diverging	57 58	36.8 31.8	E D	56 55	42.5 39.0	E E
Ming Avenue off-ramp	Diverging	58	30.1	D	55	39.5	Ē
White Lane off-ramp	Diverging	60	19.2	В	60	23.6	C
Westside Parkway EB		•	•			•	
West Beltway loop on-ramp	Merging				61	13.7	В
West Beltway diagonal on-ramp	Merging	_	- NOT APPLICA	ABLE —	62	18.5	В
Allen Road on-ramp	Merging	61	14.4	В	55	32.0	D
Calloway Drive loop on-ramp	Merging	62	13.5	В	60	23.7	С
Calloway Drive diagonal on-ramp	Merging	60	18.4	В	56	27.8	C
Mohawk Street on-ramp	Merging	64	11.4	В	63	14.3	В
West Beltway off-ramp Allen Road off-ramp	Diverging Diverging	65	3.8	Α	47 62	23.9 18.2	C B
Calloway Drive off-ramp	Diverging	62	15.0	B	60	30.7	D
Coffee Road off-ramp	Diverging	61	23.1	Č	60	33.9	D
Coffee Road on-ramp to Mohawk Street/Truxtun Avenue off-ramp	Weaving	63	10.2	В	62	14.4	В
Westside Parkway WB							
Mohawk Street loop on-ramp	Merging	62	9.6	Α	62	10.9	В
Mohawk Street diagonal on-ramp	Merging	62	10.1	В	62	13.0	В
Coffee Road loop on-ramp	Merging	62	6.3	A	62	10.7	В
Coffee Road diagonal on-ramp Calloway Drive on-ramp	Merging	63	6.9	A	62	12.4 14.4	B B
Allen Road on-ramp	Merging Merging	63 63	7.4 4.5	A A	62 63	10.4	В
West Beltway loop on-ramp	Merging		•	•	62	9.3	Ä
West Beltway diagonal on-ramp	Merging	_	NOT APPLICA	ABLE —	60	12.0	В
Coffee Road off-ramp	Diverging	61	10.2	В	61	12.0	В
Calloway Drive diagonal off-ramp	Diverging	63	10.2	В	62	18.3	В
Calloway Drive loop off-ramp	Diverging	62	8.7	A	61	16.0	В
Allen Road off-ramp West Beltway off-ramp	Diverging Diverging	62	6.6 - NOT APPLICA	A	61 62	16.0 10.0	B A
State Route 58 EB	Diverging		-NOT AFFLICE	IDLL —	02	10.0	
	T					05.0	
SR 99 NB and SB on-ramp Chester Avenue on-ramp	Merging Merging	57 59	23.2 25.8	C	57 57	25.3 29.4	C D
Union Avenue loop on-ramp	Merging	62	19.2	В	61	22.3	
Union Avenue diagonal on-ramp	Merging	62	22.0	Č	61	25.4	C C
Cottonwood Road on-ramp	Merging	62	19.4	В	61	22.9	С
H Street off-ramp	Diverging	60	23.4	С	59	25.4	С
Union Avenue off-ramp	Diverging	61	25.1	С	60	28.1	D
Cottonwood Road off-ramp	Diverging	62	22.0	С	61	25.3	С
State Route 58 WB						1	1
Cottonwood Road on-ramp	Merging	63	18.6	В	61	27.0	СС
Brundage Lane on-ramp Union Avenue on-ramp	Merging	62 61	18.0 21.7	B C	61	23.0 27.6	C C
H Street on-ramp	Merging Merging	60	23.0	C	59 54	32.0	D
Cottonwood Road off-ramp	Diverging	64	17.9	В	63	25.0	С
Brundage Lane off-ramp	Diverging	62	20.1	Č	60	29.1	Ď
Chester Avenue off-ramp	Diverging	61	21.8	С	60	27.2	С
SR 99 NB off-ramp	Diverging	60	23.3	Ç	56	31.5	D
SR 99 SB off-ramp	Diverging	59	12.3	В	57	14.7	В

Note: Merge/diverge calculations are based on 1,500 feet from on-/off-ramp. These calculations also include two right-most lanes within 1,500 feet.

Table 4-11. PM Peak Hour Measures of Effectiveness for the No-Build Alternative— Merging/Diverging Conditions

Merging/Diverging Condition	, .		YEAR 2018	3		YEAR 2038	
	ANALYSIS	SPEED	DENSITY	LOS	SPEED	DENSITY	LOS
LOCATION	TYPE	(mph)	(veh/ln/mi)	(HCM 2000)	(mph)	(veh/ln/mi)	(HCM 2000)
State Route 99 NB							
White Lane loop on-ramp	Merging	53	28.4	D	38	41.3	E
White Lane diagonal on-ramp Ming Avenue on to SR 58/Wible Road off	Merging	56 32	31.8 53.7	D E	34 24	52.0 61.7	E F
Wible Road on-ramp	Merging Merging	60	22.9	C	60	18.5	B
SR 58 WB on-ramp	Merging	54	33.4	Ď	51	34.1	D
California Avenue loop on-ramp	Merging	57	33.1	D	59	31.3	D
California Avenue diagonal on-ramp	Merging	56	32.7	D	56	33.0	D
Buck Owens Boulevard/Sillect Avenue on-ramp White Lane off-ramp	Merging Diverging	60 63	22.1 19.8	C B	60 53	23.7 34.2	C D
Ming Avenue off-ramp	Diverging	60	29.5	D	15	81.5	F
Ming Avenue on to SR 58/Wible Road off	Diverging	32	53.7	E	24	61.7	F
California Avenue off-ramp	Diverging	59	35.6	E	59	34.9	D
Rosedale Highway off-ramp Buck Owens Boulevard/Sillect Avenue off-ramp	Diverging Diverging	57 61	26.6 21.1	C C	56 61	27.7 21.3	C C
Airport Drive off-ramp	Diverging	57	28.1	D	56	29.6	D
State Route 99 SB		•		•			
Airport Drive on-ramp	Merging	59	28.3	D	35	51.1	Е
Rosedale Highway loop on-ramp	Merging	41	37.2	E	39	38.2	E
Rosedale Highway diagonal on-ramp	Merging	49	46.9 54.3	E E	45 35	51.6 62.1	E F
California Avenue on-ramp SR 58 WB on-ramp	Merging Merging	39 33	54.3 59.7	F	35 17	62.1 107.5	F
Real Road on-ramp	Merging	43	42.9	E	33	59.2	F
Ming Avenue on-ramp	Merging	59	26.4	C	59	29.3	D
White Lane loop on-ramp White Lane diagonal on-ramp	Merging Merging	60 61	19.5 21.5	B C	60 60	25.1 28.0	C
Rosedale Highway off-ramp	Diverging	60	29.3	D	34	53.7	E
California Drive off-ramp	Diverging	55	44.7	E	49	51.9	Ē
SR 58 EB/Stockdale Highway off-ramp	Diverging	50	46.2	E	48	47.4	E
Ming Avenue off-ramp	Diverging	52	41.2	E	38	57.6	F
White Lane off-ramp	Diverging	58	25.0	С	58	28.0	D
Westside Parkway EB	l Manada a	1			04	10.8	
West Beltway loop on-ramp West Beltway diagonal on-ramp	Merging Merging	_	NOT APPLICA	NBLE —	61 63	10.8	B B
Allen Road on-ramp	Merging	61	12.2	В	59	23.9	Č
Calloway Drive loop on-ramp	Merging	62	11.1	В	61	17.9	В
Calloway Drive diagonal on-ramp	Merging	61	14.6	В	60	20.1	С
Mohawk Street on-ramp West Beltway off-ramp	Merging Diverging	64	9.4	A	64 44	10.9 24.2	B C
Allen Road off-ramp	Diverging	64	3.9	Α	63	12.5	В
Calloway Drive off-ramp	Diverging	63	12.5	В	61	24.1	С
Coffee Road off-ramp	Diverging	61	18.5	В	60	26.4	С
Coffee Road loop on-ramp to Mohawk Street/Truxtun Avenue off-ramp	Weaving	62	10.6	В	62	11.6	В
Westside Parkway WB		00	40.0		00	40.5	
Mohawk Street loop on-ramp Mohawk Street diagonal on-ramp	Merging Merging	62 61	13.6 15.2	B B	62 61	16.5 18.6	B B
Coffee Road loop on-ramp	Merging	62	10.4	В	61	18.5	В
Coffee Road diagonal on-ramp	Merging	63	12.4	В	60	21.6	С
Calloway Drive on-ramp	Merging	62	13.1	В	60	27.4	С
Allen Road on-ramp West Beltway loop on-ramp	Merging Merging	63	5.6	A	62 61	17.1 15.0	B B
West Beltway diagonal on-ramp	Merging	_	NOT APPLICA	BLE —	58	18.8	В
Coffee Road off-ramp	Diverging	61	12.9	В	61	14.2	В
Calloway Drive diagonal off-ramp	Diverging	60	21.0	С	59	33.1	D
Calloway Drive loop off-ramp Allen Road off-ramp	Diverging Diverging	60 61	17.0 12.0	B B	61 60	27.8 31.4	C D
West Beltway off-ramp	Diverging		NOT APPLICA		61	17.1	В
State Route 58 EB		,	2, 2.07				
SR 99 NB and SB on-ramp	Merging	57	23.3	С	57	24.4	С
Chester Avenue on-ramp	Merging	59	26.6	С	58	29.5	D
Union Avenue loop on-ramp	Merging	61	22.6	С	59	27.7	C
Union Avenue diagonal on-ramp Cottonwood Road on-ramp	Merging Merging	61 61	25.6 23.5	C C	59 59	33.1 30.3	D D
H Street off-ramp	Diverging	60	23.5	C	60	25.1	С
Union Avenue off-ramp	Diverging	61	25.9	С	60	28.3	D
Cottonwood Road off-ramp	Diverging	61	25.4	С	60	31.9	D
State Route 58 WB							
Cottonwood Road on-ramp	Merging	62	19.5	В	61	22.9	C
Brundage Lane on-ramp	Merging	62	19.6	B C	61	22.8	C
Union Avenue on-ramp H Street on-ramp	Merging Merging	60 59	24.3 26.0	C	59 35	28.5 45.9	D E
Cottonwood Road off-ramp	Diverging	64	17.0	В	63	20.4	C
Brundage Lane off-ramp	Diverging	62	20.9	С	61	24.3	С
Chester Avenue off-ramp	Diverging	61	24.1	C	60	28.0	C
SR 99 NB off-ramp SR 99 SB off-ramp	Diverging Diverging	60 58	26.2 14	C B	35 19	46.3 46	E E
on to be all famp	Divorging	_ 50			.0	70	_

Note: Merge/diverge calculations are based on 1,500 feet from on-/off-ramp. These calculations also include two right-most mainline lanes within 1,500 feet.

Table 4-12. AM Peak Hour Measures of Effectiveness for the No-Build Alternative—On-Ramp and Off-Ramp Performance

Ramp and	OII-Ra	-						
	DEMAND		R 2018		DE14441D		AR 2038	1
	DEMAND VOLUME	CORSIM SERVED	PERCENTAGE SERVED IN CORSIM	SPEED	VOLUME	CORSIM SERVED	PERCENTAGE SERVED IN CORSIM	SPEED
FREEWAY RAMPS State Route 99 NB Off-ramps	(vph)	VOLUME (vph)	MODEL (%)	(mph)	(vph)	VOLUME (vph)	MODEL (%)	(mph)
White Lane	680	680	100%	62	1,065	935	88%	15
Ming Avenue	515	460	89%	54	790	622	79%	30
SR 58 EB/Wible Road	2,105	2,105	100%	46	2,825	2,206	78%	45
California Avenue	1,145	1,025	90%	52	1,180	974	83%	52
Rosedale Highway Buck Owens Boulevard/Sillect Avenue	2,150 825	1,946 747	91% 91%	53 53	2,440 825	1,999 682	82% 83%	53 53
Airport Drive	1,535	1,417	92%	52	1,750	1,287	74%	52
State Route 99 NB On-ramps	.,,,,,,	.,	3-73		.,,	.,	,,	
White Lane loop	1,590	1,086	68%	44	1,605	520	32%	15
White Lane diagonal	690	658	95%	52	695	493	71%	11
Ming Avenue	1,320	1,320	100%	47	1,585	1,585	100%	43
Wible Road SR 58 WB	590 1,285	551 1,227	93% 95%	50 45	640 1,785	544 1,775	85% 99%	50 37
California Avenue loop	535	514	96%	44	565	460	81%	44
California Avenue diagonal	235	118	50%	51	240	147	61%	50
Buck Owens Boulevard/Sillect Avenue	385	371	96%	33	600	547	91%	33
State Route 99 SB Off-ramps			,		•	T	1	
Rosedale Highway	660	660	100%	53	1,075	1,066	99%	52
California Avenue	1,105	1,105	100%	52	1,200	1,200	100%	52
SR 58 EB/Stockdale Highway Ming Avenue	1,645 1,000	1,642 1,000	100% 100%	52 53	1,890 1,230	1,795 1,230	95% 100%	52 52
White Lane	1,365	1,357	99%	54	1,495	1,436	96%	53
State Route 99 SB On-ramps								
Airport Drive	1,265	1,265	100%	47	1,425	1,425	100%	46
Rosedale Highway loop	900	900	100%	38	1,150	805	70%	41
Rosedale Highway diagonal California Avenue	1,180	1,174 401	99%	36 51	1,690	1,690 720	100%	30 50
SR 58 WB	425 1,285	401 1,215	94% 95%	51	720 1,550	1,550	100% 100%	48
Real Road	390	342	88%	51	510	505	99%	51
Ming Avenue	495	490	99%	52	690	690	100%	51
White Lane loop	180	178	99%	52	210	170	81%	52
White Lane diagonal	350	162	46%	53	460	370	80%	53
Westside Parkway EB Off-ramps		NOT A	PPLICABLE —		000	000	4000/	
West Beltway Allen Road	65	— NOT AF	94%	56	800 195	800 187	100% 96%	50 53
Calloway Drive	505	472	93%	54	1,108	1,087	98%	53
Coffee Road loop	1,225	1,225	100%	51	2,185	2,107	96%	51
Mohawk Street	1,080	1,080	100%	54	1,900	1,900	100%	53
Westside Parkway EB On-ramps	1				4 405	4 4 7 7	1 000/	40
West Beltway loop West Beltway diagonal		— NOT AF	PPLICABLE —		1,185 550	1,177 525	99% 95%	48 53
Allen Road	1,425	1,394	98%	45	1,810	1,810	100%	44
Calloway Drive loop	465	421	91%	47	484	479	99%	47
Calloway Drive diagonal	1,055	1,055	100%	51	914	914	100%	51
Coffee Road loop	315	287	91%	49	385	385	100%	48
Coffee Road diagonal Mohawk Street	355	328	92%	54	550	527	96%	54
Westside Parkway WB Off-ramps			_		_	_	_	
Coffee Road	1,125	1,125	100%	54	1,185	1,185	100%	54
Calloway Drive diagonal	195	195	100%	54	312	294	94%	54
Calloway Drive loop	430	385	90%	54	580	551	95%	54
Allen Road	750	698	93%	54	850	806	95%	52
West Beltway Westside Parkway WB On-ramps		— NOT AF	PPLICABLE —		1,060	1,030	97%	54
Mohawk Street loop	680	671	99%	47	835	835	100%	47
Mohawk Street diagonal	720	661	92%	51	1,005	1,005	100%	51
Coffee Road loop	185	175	95%	51	555	487	88%	50
Coffee Road diagonal	230	228	99%	54	645	601	93%	54
Calloway Drive Allen Road	240 100	240 100	100% 100%	53 50	465 225	454 225	98% 100%	53 50
West Beltway loop	100			1 30	395	369	93%	49
West Beltway diagonal		— NOT AF	PPLICABLE —		290	286	99%	54
State Route 58 EB Off-ramps							· · · · · · · · · · · · · · · · · · ·	
H Street	485	465	96%	54	545	485	89%	52
Union Avenue	855	833	97%	53	915	800	87%	53
Cottonwood Road State Route 58 EB On-ramps	460	445	97%	53	550	504	92%	54
SR 99 NB and SR 99 SB	2,765	2,731	99%	52	3,505	2,967	85%	52
Chester Avenue	555	510	92%	51	595	595	100%	51
Union Avenue loop	245	237	97%	53	300	278	93%	53
Union Avenue diagonal	230	230	100%	53	275	264	96%	53
Cottonwood Road	230	230	100%	52	305	305	100%	52
State Route 58 WB Off-ramps	245	245	1000/	EA	255	224	020/	E E
Cottonwood Road Brundage Lane	245 530	245 517	100% 98%	54 54	255 880	234 880	92% 100%	55 50
Chester Avenue	605	585	97%	53	735	735	100%	53
SR 99 NB	1,285	1,231	96%	53	1,785	1,785	100%	52
SR 99 SB	1,285	1,213	94%	42	1,550	1,550	100%	41
State Route 58 WB On-ramps								
Cottonwood Road	230	217	94%	51	240	240	100%	50
Brundage Lane Union Avenue	215 345	215 327	100% 95%	49 51	220 400	204 391	93% 98%	49 51
H Street	490	327 421	95% 86%	51	400	405	100%	50



Table 4-13. PM Peak Hour Measures of Effectiveness for the No-Build Alternative—On-Ramp and Off-Ramp Performance

- Kamp and	OII-K	amp Perto						
			R 2018				R 2038	
	DEMAND VOLUME	CORSIM SERVED	PERCENTAGE SERVED IN CORSIM	SPEED	DEMAND VOLUME	CORSIM SERVED	PERCENTAGE SERVED IN CORSIM	SPEED
FREEWAY RAMPS	(vph)	VOLUME (vph)	MODEL (%)	(mph)	(vph)	VOLUME (vph)	MODEL (%)	(mph)
State Route 99 NB Off-ramps	005	005	4000/	0.4	4.005	4.005	4000/	- 00
White Lane Ming Avenue	685 625	685 574	100% 92%	61 54	1,095 820	1,095 702	100% 86%	33 32
SR 58 EB/Wible Road	1,905	1,898	100%	47	2,500	2,220	89%	46
California Avenue	770	770	100%	53	880	784	89%	52
Rosedale Highway	1,850	1,850	100%	53	2,090	1,978	95%	53
Buck Owens Boulevard/Sillect Avenue Airport Drive	400 1,345	400 1,329	100% 99%	52 52	550 1,415	453 1,224	82% 87%	53 52
State Route 99 NB On-ramps	1,343	1,329	99%	32	1,410	1,224	0176	32
White Lane loop	1,455	1,435	99%	42	1,645	1,214	74%	40
White Lane diagonal	555	555	100%	53	610	603	99%	45
Ming Avenue	1,245	1,245	100%	48	1,495	1,495	100%	44
Wible Road SR 58 WB	610 1,430	582 1,375	95% 96%	51 43	610 1,715	518 1,666	85% 97%	51 35
California Avenue loop	690	674	98%	44	740	527	71%	45
California Avenue diagonal	320	253	79%	51	330	232	70%	51
Buck Owens Boulevard/Sillect Avenue	620	600	97%	33	750	696	93%	31
State Route 99 SB Off ramps Rosedale Highway	675	675	100%	53	1,095	1,003	92%	15
California Avenue	1,085	1,085	100%	52	1,190	1,168	98%	50
SR 58 EB/Stockdale Highway	1,975	1,975	100%	51	2,095	1,982	95%	51
Ming Avenue	1,625	1,604	99%	50	1,745	1,669	96%	17
White Lane	2,030	2,012	99%	54	2,030	1,925	95%	45
State Route 99 SB On-ramps Airport Drive	1,755	1,755	100%	45	2,115	2,012	95%	30
Rosedale Highway loop	1,755	1,735	100%	34	1,380	1,380	100%	33
Rosedale Highway diagonal	1,335	1,335	100%	34	1,625	1,343	83%	34
California Avenue	925	860	93%	50	1,050	962	92%	49
SR 58 WB Real Road	1,430 470	1,410 426	99% 91%	47 51	1,755	1,631 532	93%	13 50
Ming Avenue	645	426 645	100%	51	655 835	835	81% 100%	51
White Lane loop	495	488	99%	51	575	540	94%	51
White Lane diagonal	450	274	61%	53	675	573	85%	52
Westside Parkway EB Off-ramps		NOT A	201104015		055	000	000/	40
West Beltway Allen Road	85	— NOT AF 82	PPLICABLE — 96%	53	955 180	936 164	98% 91%	49 54
Calloway Drive	435	402	92%	54	929	916	99%	53
Coffee Road loop	880	842	96%	52	1,705	1,638	96%	51
Mohawk Street	1,335	1,332	100%	53	1,545	1,504	97%	54
Westside Parkway EB On-ramps West Beltway loop					875	860	98%	48
West Beltway diagonal		— NOT AF	PPLICABLE —		145	145	100%	54
Allen Road	1,095	1,095	100%	48	1,610	1,581	98%	48
Calloway Drive loop	340	317	93%	47	381	362	95%	47
Calloway Drive diagonal Coffee Road loop	860 415	838 400	97% 96%	52 48	762 430	719 410	94% 95%	53 48
Coffee Road diagonal	460	439	95%	54	475	456	96%	54
Mohawk Street	_	_	_	_	_	_		_
Westside Parkway WB Off-ramps								
Coffee Road	1,285	1,206	94%	54 54	1,215	1,122	92%	54 54
Calloway Drive diagonal Calloway Drive loop	505 1,105	463 941	92% 85%	54 53	510 1,015	510 935	100% 92%	54 54
Allen Road	1,565	1,435	92%	54	1,880	1,796	96%	51
West Beltway			PPLICABLE —		1,875	1,772	95%	53
Westside Parkway WB On-ramps	4.000	4.047	0.70/	1 47	4.050	4.050	4000/	1 47
Mohawk Street loop Mohawk Street diagonal	1,080 1,245	1,047 1,076	97% 86%	47 50	1,250 1,415	1,250 1,366	100% 97%	47 50
Coffee Road loop	290	278	96%	51	980	952	97%	50
Coffee Road diagonal	565	512	91%	53	1,025	1,001	98%	53
Calloway Drive	550	550	100%	53	985	983	100%	52
Allen Road West Beltway loop	70	70	100%	51	330 545	322 531	98% 97%	50 48
West Beltway diagonal		— NOT AF	PPLICABLE —		350	334	95%	53
State Route 58 EB Off-ramps								
H Street	440	423	96%	53	475	435	92%	53
Union Avenue Cottonwood Road	595 410	595 410	100% 100%	53 54	645 600	604 522	94% 87%	54 54
State Route 58 EB On-ramps	410	410	10076	J4	000	JZZ	0170	J4
SR 99 NB and SR 99 SB	2,835	2,835	100%	52	3,390	3,114	92%	52
Chester Avenue	685	651	95%	51	840	840	100%	50
Union Avenue diagonal	415	335	81%	53	770	656	85%	53
Union Avenue diagonal Cottonwood Road	315 340	315 340	100% 100%	52 52	635 485	628 484	99% 100%	52 51
State Route 58 WB Off-ramps	0.10	3.10		, 32	.50	.54	.5070	
Cottonwood Road	205	205	100%	54	240	240	100%	54
Brundage Lane	495	495	100%	53	595	565	95%	52
Chester Avenue SR 99 NB	615	576 1 375	94%	53 53	645	620	96%	53
SR 99 NB SR 99 SB	1,430 1,430	1,375 1,407	96% 98%	53 41	1,715 1,755	1,672 1,652	97% 94%	49 13
State Route 58 WB On-ramps	, .50	.,			,,,,,,,	.,		
Cottonwood Road	380	380	100%	50	400	394	99%	50
Brundage Lane	355	325	92%	49	395	368	93%	49
Union Avenue H Street	530 505	470 443	89% 88%	51 51	555 595	522 595	94% 100%	51 22
11 011001	505	7+0	00 /0	J I	JJJ	535	10070	~~



Table 4-14. Ramp Termini and Intersection Level of Service for the No-build Alternative (1 of 3)

			YEA	R 2018			YEAR	2038	
			AM PEAK		PM PEAK	I	AM PEAK		PM PEAK
			AVG DELAY		AVG DELAY		AVG DELAY		AVG DELAY
INTERSECTION	SIGNAL CONTROL	LOS	(sec/veh)	LOS	(sec/veh)	LOS	(sec/veh)	LOS	(sec/veh)
I-5 SB ramps/Stockdale Highway	NB/SB two-way stop	Α	4.1	Α	7.3	Α	6.4	F	92.0
. ,	Imp-Signal*			I/A		В	12.2	В	11.6
2. I-5 NB ramps/Stockdale Highway	NB/SB two-way stop	Α	4.6	Α	8.4	Α	4.0	В	14.0
3. SR 43 (Enos Lane)/Rosedale Highway	4-way stop	В	13.3	D	28.3	D	25.8	F	77.0
o. or is (2nds 2ans)/residuals riighway	Imp-Signal		N	I/A		С	24.5	С	31.9
4. SR 43 (Enos Lane)/Stockdale Highway	4-way stop	F	45.9	F	66.2	F	>150	F	>150
4. OK 45 (Ellos Earle)/Otockdale Flighway	Imp-Signal	D	37.1	D	39.2	С	24.2	С	23.0
5. SR 43 (Enos Lane)/I-5 NB ramps	EB/WB two-way stop	Α	5.4	В	11.5	Α	3.4	В	10.7
6. SR 43 (Enos Lane)/I-5 SB ramps	EB/WB two-way stop	Α	7.1	С	16.1	Α	5.7	С	19.8
7. Stockdale Highway/Nord Road	Signal	D	40.0	С	28.6	С	31.8	С	29.1
Stockdale Highway/Wegis Avenue	NB/SB two-way stop	Α	7.1	В	10.4	F	>150	F	>150
6. Stockdale Highway/Wegis Avenue	Imp-Signal*		N	I/A		С	21.5	С	23.2
9. Stockdale Highway/Heath Road	Stop (existing) Signal (future)	С	21.1	С	20.5	С	29.2	С	27.6
10. Stockdale Highway/Westside Parkway	Signal	Α	5.6	Α	6.4	Α	7.6	Α	9.0
11. West Beltway/Westside Parkway WB ramp	Signal		Does not e	exist in	2018	Α	6.5	В	14.7
12. West Beltway/Westside Parkway EB ramp	Signal		Does not	exist in	2018	В	11.2	В	18.2
13. Allen Road/Rosedale Highway	Signal*	С	33.2	D	43.3	D	40.7	D	47.1
14. Allen Road/Brimhall Road	Signal	С	25.6	С	30.4	С	25.9	С	33.5
15. Allen Road/Westside Parkway WB ramps	Signal	В	12.7	С	24.8	В	15.0	С	28.0
16. Allen Road/Westside Parkway EB ramps	Signal	В	15.7	Α	5.1	В	10.2	Α	3.7
17. Allen Road/San Juan Avenue	Signal*	С	22.3	С	20.3	С	22.6	С	28.8
18. Allen Road/Stockdale Highway	Signal	С	26.5	С	28.0	С	29.8	С	32.5
19. Calloway Drive/Rosedale Highway	Signal*	D	40.3	D	40.6	D	49.4	D	54.8
20. Calloway Drive/Brimhall Road	Signal	С	25.7	С	29.2	С	29.9	С	28.5
21. Calloway Drive/Westside Parkway WB ramps	Signal	Α	7.9	Α	8.6	В	10.8	С	20.5
22. Calloway Drive/Westside Parkway EB ramps	Signal	В	16.7	Α	7.8	В	18.4	Α	8.9
23. Calloway Drive/Stockdale Highway	Signal	D	40.8	С	34.1	D	52.3	D	39.7
24. Coffee Road/Rosedale Highway*	Signal*	Е	69.2	F	87.6	F	81.7	F	92.6



Table 4-14. Ramp Termini and Intersection Level of Service for the No-build Alternative (2 of 3)

Table 1 1 1 Kamp Termin and Interception			YEA	R 2018	, ,		YEAR	2038	
		,	AM PEAK		PM PEAK	-	AM PEAK		PM PEAK
			AVG DELAY		AVG DELAY		AVG DELAY		AVG DELAY
INTERSECTION	SIGNAL CONTROL	LOS	(sec/ve h)	LOS	(sec/veh)	LOS	(sec/veh)	LOS	(sec/veh)
25. Coffee Road/Brimhall Road	Signal	С	26.9	С	33.9	С	27.3	D	43.2
26. Coffee Road/Westside Parkway WB ramps	Unsignalized		N	I/A			N.	/A	
27. Coffee Road/Westside Parkway EB ramps	Signal	С	27.3	В	13.4	С	28.7	В	19.0
28. Coffee Road/Truxtun Avenue	Signal	В	18.0	В	18.1	С	20.5	С	24.5
29. Coffee Road/Stockdale Highway	Signal*	D	35.5	D	53.5	Е	56.3	F	99.8
30. Mohawk Street/Rosedale Highway	Stop (existing)* Signal (future)*	Е	57.0	F	111.9	F	103.6	F	123.2
31. Mohawk Street/WSP WB ramps	Signal		١	I/A			N.	/A	
32. Mohawk Street/WSP EB ramps	Signal	С	26.4	В	14.3	С	24.4	В	19.5
33. Mohawk Street/Truxtun Avenue	Signal	С	27.1	С	24.7	С	31.1	Е	59.8
34. Mohawk Street/California Avenue	Signal*	C	30.7	Т	88.7	F	106.3	F	162.3
35. Stockdale Highway/California Avenue	Signal*	Е	64.7	D	49.0	F	92.6	F	99.9
36. Airport Drive/State Road–SR 204 off-ramp	Signal	C	20.4	C	22.9	C	20.2	C	21.9
37. Airport Drive/SR 99 NB ramp	Signal	Α	7.1	В	11.3	Α	7.1	В	10.6
38. Buck Owens Boulevard/Rio Mirada Drive	Signal	С	21.1	С	24.3	С	29.1	С	28.6
39. SR 99 NB ramps/Buck Owens Boulevard	Signal	D	41.2	D	43.8	D	37.5	D	46.2
40. Rosedale Highway/Camino Del Rio Court	Signal	C	34.6	D	44.8	C	20.9	D	49.9
41. Rosedale Highway/SR 99 SB ramps	Signal	В	16.1	С	21.4	С	22.7	D	37.6
42. Rosedale Highway/SR99 NB ramps	Signal	С	25.8	С	27.2	С	26.6	С	30.0
43. 24th Street/Oak Street	Signal	С	26.5	С	30.3	D	37.5	С	29.5
44. Truxtun Avenue/Empire Drive	Signal*	С	32.8	С	33.8	С	21.4	D	47.6
45. Truxtun Avenue/Oak Street	Signal	D	35.5	D	52.7	Е	61.9	Е	73.9
46. California Avenue/Chester Lane	Signal	С	23.3	С	29.2	С	23.7	С	30.1
47. California Avenue/SR 99 SB ramps	Signal	D	43.6	Е	61.0	D	54.6	Е	76.1
48. California Avenue/SR 99 NB ramps	Signal	С	26.7	С	28.6	С	28.5	D	51.5
49. California Avenue/Oak Street	Signal	С	25.3	Е	56.0	С	28.3	Е	61.5
50. Stockdale Highway/Stine Road	Signal*	D	40.0	Е	75.8	F	89.2	F	>150
51. Stockdale Highway/Real Road	Signal	Е	59.1	Е	58.2	D	48.3	F	94.0
52. Stockdale Highway/SR 99 SB ramp	Signal	В	14.3	В	10.7	В	16.8	В	18.0



Table 4-14. Ramp Termini and Intersection Level of Service for the No-build Alternative (3 of 3)

			YEAI	R 2018	,		YEAR	2038	
			AM PEAK		PM PEAK	F	AM PEAK		PM PEAK
INTERSECTION	SIGNAL CONTROL	LOS	AVG DELAY (sec/veh)	LOS	AVG DELAY (sec/veh)	LOS	AVG DELAY (sec/veh)	LOS	AVG DELAY (sec/veh)
53. Brundage Lane/Oak Street	Signal	С	27.4	D	42.4	С	31.9	D	40.3
54. Real Road/SR 58	Signal	С	23.6	C	23.8	С	21.8	D	39.3
55. Wible Road/SR 99 NB ramps	Signal	В	14.3	C	31.4	В	15.5	С	28.4
56. Ming Avenue/New Stine Road	Signal*	D	47.2	D	47.7	D	46.9	Ш	70.3
57. Ming Avenue/Real Road	Signal	С	24.7	C	32.2	С	25.3	Ш	64.5
58. Ming Avenue/SR 99 SB ramps	Signal	Α	2.5	Α	2.8	Α	3.2	В	15.6
59. Ming Avenue/Wible Road	Signal	С	21.6	C	23.4	С	23.2	О	24.5
60. Ming Avenue/SR 99 NB ramps	Signal	С	24.9	C	30.3	D	36.4	О	28.3
61. Ming Avenue/Castro Lane	Signal	С	23.1	C	26.6	С	25.3	D	38.9
62. White Lane/Wible Road	Signal*	Е	55.5	Е	77.1	F	82.7	F	131.6
63. White Lane/SR 99 SB ramps	Signal*	С	20.2	F	95.5	С	28.7	F	96.8
64. White Lane/SR 99 NB ramps	Signal*	Α	9.3	Α	9.8	В	20.0	В	19.7
65. White Lane/Hughes Lane	Signal*	D	36.3	D	45.2	D	37.3	С	29.5
66. H Street/Brundage Lane	Signal	С	26.1	D	38.7	С	29.2	Е	68.0
67. H Street/SR 58 WB ramp	Signal	В	17.6	Е	66.2	С	20.6	Е	64.3
68. H Street/SR 58 EB ramp	Signal	D	39.7	С	29.7	D	40.0	O	29.0
69. H Street/Ming Avenue	Signal*	С	30.3	D	43.3	С	33.7	D	37.0
70. Chester Avenue/Brundage Lane	Signal	С	22.1	С	26.5	С	23.2	Е	68.0
71. Chester Avenue/SR 58 WB ramp	Signal	С	23.4	С	24.6	С	26.0	С	28.5
72. Chester Avenue/SR 58 EB ramp	Signal	С	32.8	С	25.5	С	30.7	С	25.7
73. Chester Avenue/Ming Avenue	Signal*	D	39.4	D	36.6	С	34.1	D	48.5
74. Union Avenue/Brundage Lane	Signal	С	29.4	С	25.7	D	46.8	D	43.4
75. Brundage Lane/SR 58 WB ramps	Signal	С	34.5	С	28.7	С	23.4	D	41.0
76. Union Avenue/SR 58 EB ramps	Signal	В	13.8	В	12.4	В	18.0	В	15.8
77. Cottonwood Road-MLK/Brundage Lane	Signal	С	23.4	С	24.8	С	28.8	С	27.5
78. Cottonwood Road-Brundage Lane/SR 58 WB ramps	Signal	С	26.0	D	35.7	С	27.3	С	32.9
79. Cottonwood Road/SR 58 EB ramps	Signal	В	16.9	В	14.1	В	15.8	В	15.7

*LOS summary based on SYNCHRO 6

Source: Parsons



4.4 Traffic Performance of Build Alternative A

Alternative A proposes to connect the Westside Parkway to State Route 58 east near Cottonwood Road by means of a new freeway. Alternative A would begin at the Westside Parkway in between the Mohawk Street interchange and the Coffee Road interchange and turn in a southeasterly direction. It would then span the Kern River, Truxtun Avenue, Carrier Canal, and Stockdale Highway before joining the existing State Route 58 east at its existing terminus near the State Route 58/State Route 99 interchange. Improvements on State Route 58 would continue to extend through the H Street and Chester Avenue interchange to Cottonwood Road. Figure 4-7 illustrates the proposed alignment of build alternative A and the limits of new construction on State Route 58, State Route 99 and the Westside Parkway.

State Route 58 would maintain its existing connections to State Route 99 by means of freeway to freeway connectors. The existing westbound State Route 58 to northbound State Route 99 connector, southbound State Route 99 to eastbound State Route 58 connector, and northbound State Route 99 to eastbound State Route 58 would be preserved with modifications. New branch connectors would be constructed for the eastbound State Route 58 to southbound State Route 99, and northbound State Route 99 to westbound State Route 58 movements.

Auxiliary lanes would be provided on State Route 99 to accommodate the additional traffic from these branch connectors. The limits of improvements on State Route 99 would extend from the interchange at State Route 58 to the Wilson Road overcrossing. All ramps in this vicinity would have to be realigned to provide for the additional lanes. The Wible Road on- and off-ramps south of the existing State Route 99/State Route 58 interchange would be removed to accommodate the northbound State Route 99 on ramp from Ming Avenue. The Stockdale Avenue off ramp from southbound State Route 99 to the eastbound State Route 58 connector would be removed. Local access from Real Road to State Route 58 and to southbound State Route 99 would also be removed. The configuration of these freeway segments, insofar as the number of lanes, interchange on- and off-ramps and traffic demand volumes, are illustrated on Figure 4-8.

The new freeway would bisect existing business parks and residential neighborhoods. The following roads would cross under or over the proposed freeway alignment:

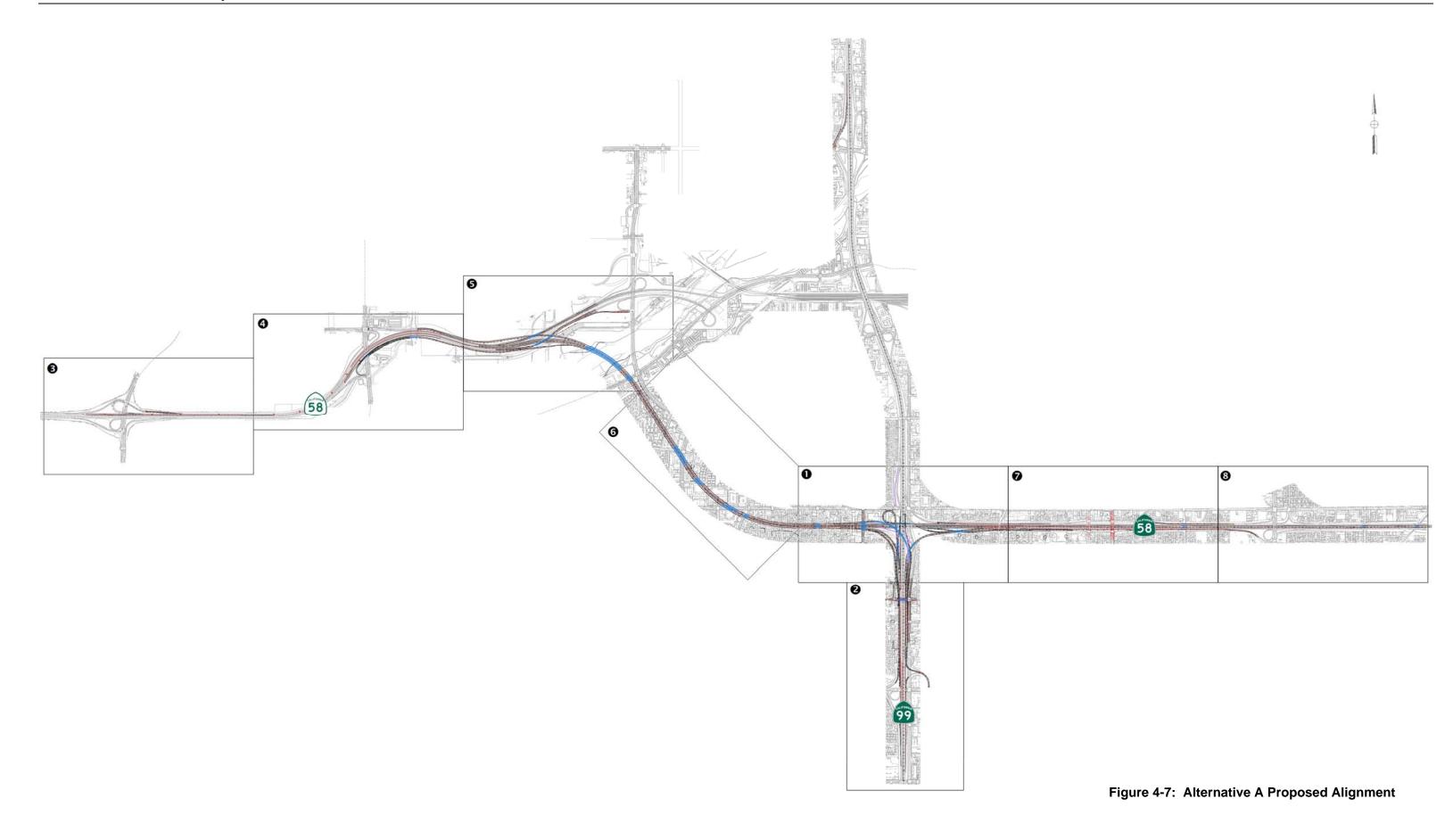
- Truxtun Avenue
- Lennox Avenue and California Avenue
- Business Center Drive

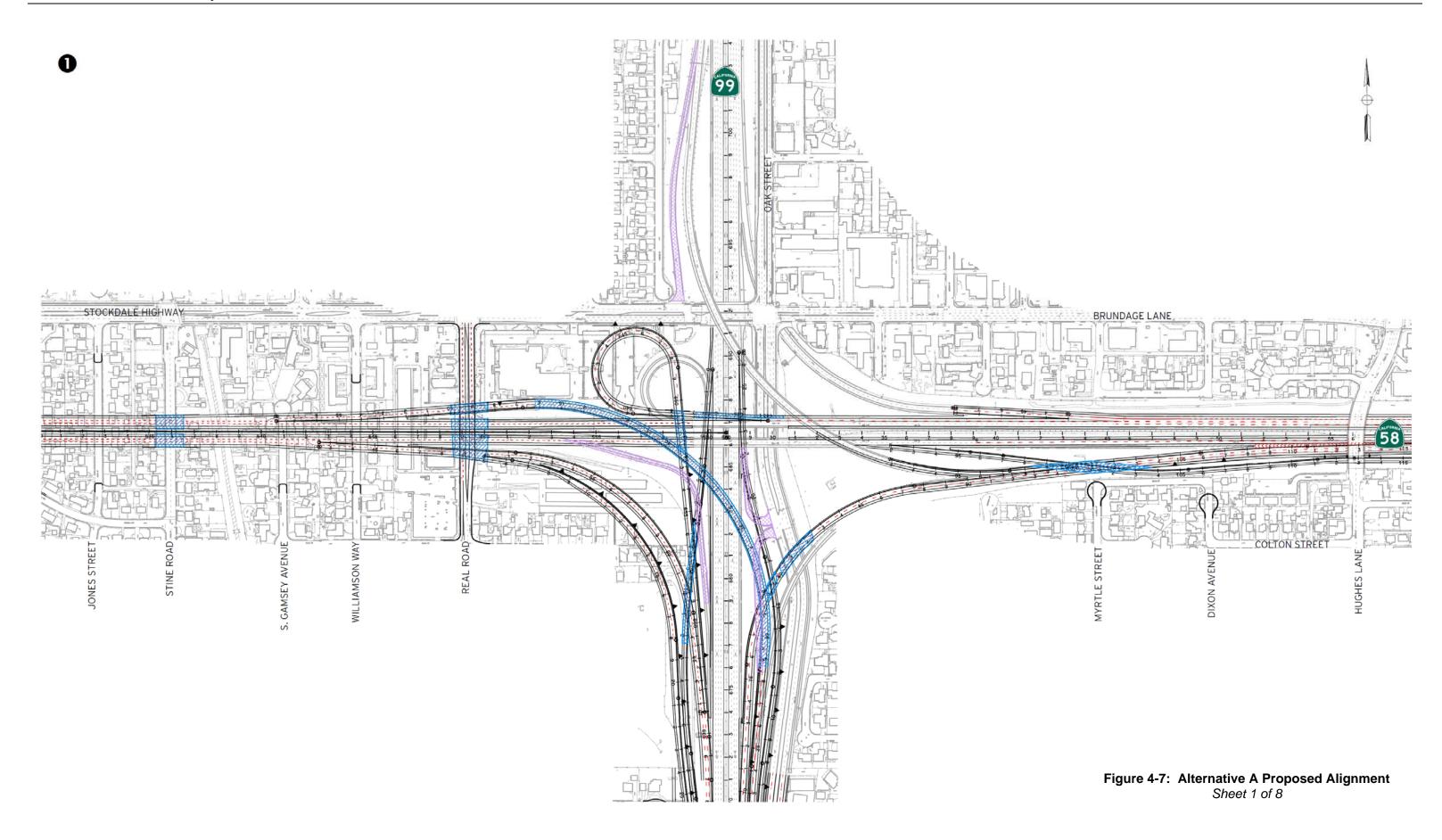
- Stockdale Highway and Montclair Street
- Stine Road
- South Real Road

The potential closure of Frazier Avenue, Westwood Way, McDonald Way, Curran Street, Griffith Street, Jones Street and Williamson Way would modify existing circulation.

To provide an overall indication of the impact of constructing the Westside Parkway to State Route 58 east connector along the alternative A alignment, a computer generated map was provided previously as Figure 3-9 which illustrates the difference between the 2038 alternative A and no-build daily traffic volumes, as assigned by the travel forecast model. Given this reassignment or rearrangement of travel patterns, Figure 4-9 illustrates the resulting location and severity of traffic congestion along Westside Parkway, Rosedale Highway/24th Street, State Route 99 and connecting roadway lengths.

Insofar as specific freeway performance data, Table 4-15 presents AM peak hour measures of effectiveness for mainline segments, while Table 4-16 reports PM peak hour results as simulated







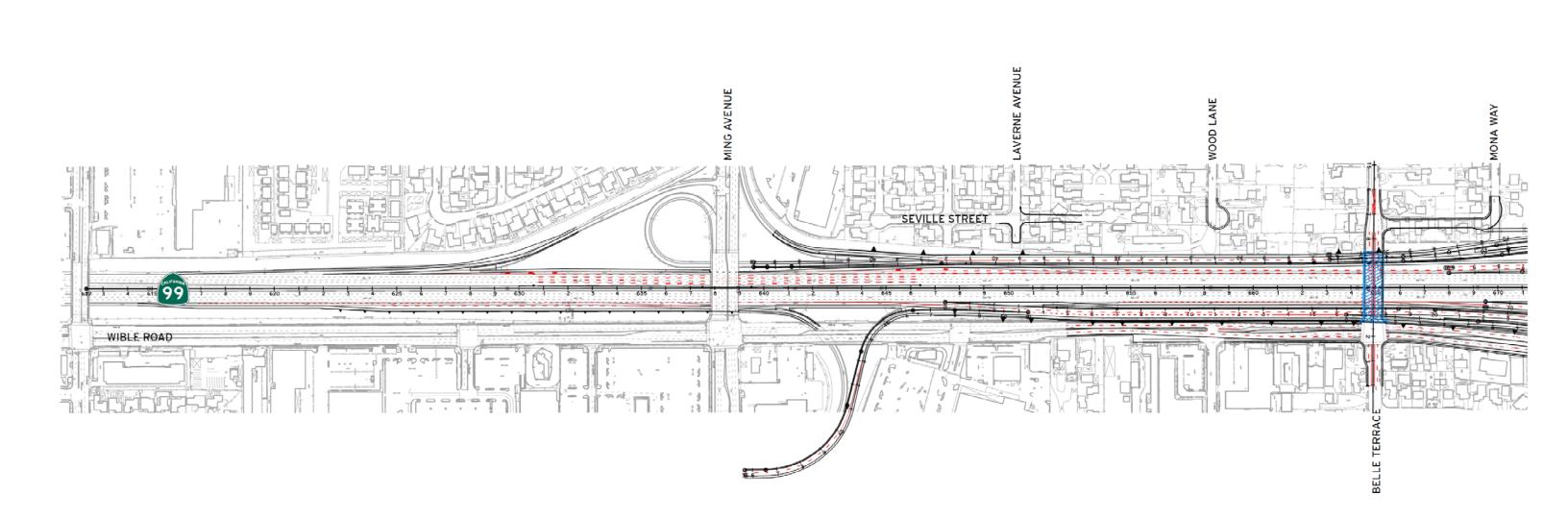


Figure 4-7: Alternative A Proposed Alignment Sheet 2 of 8

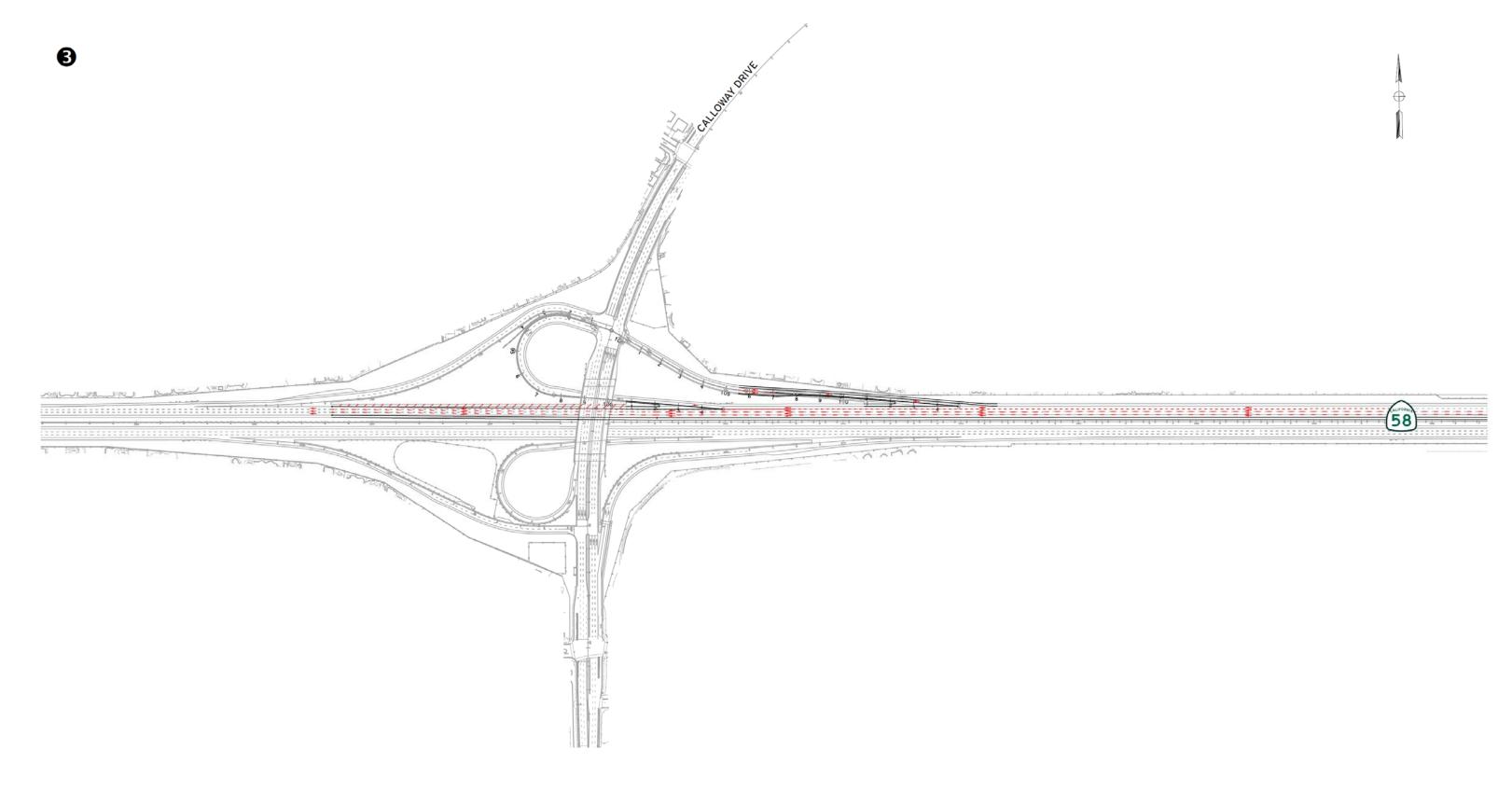
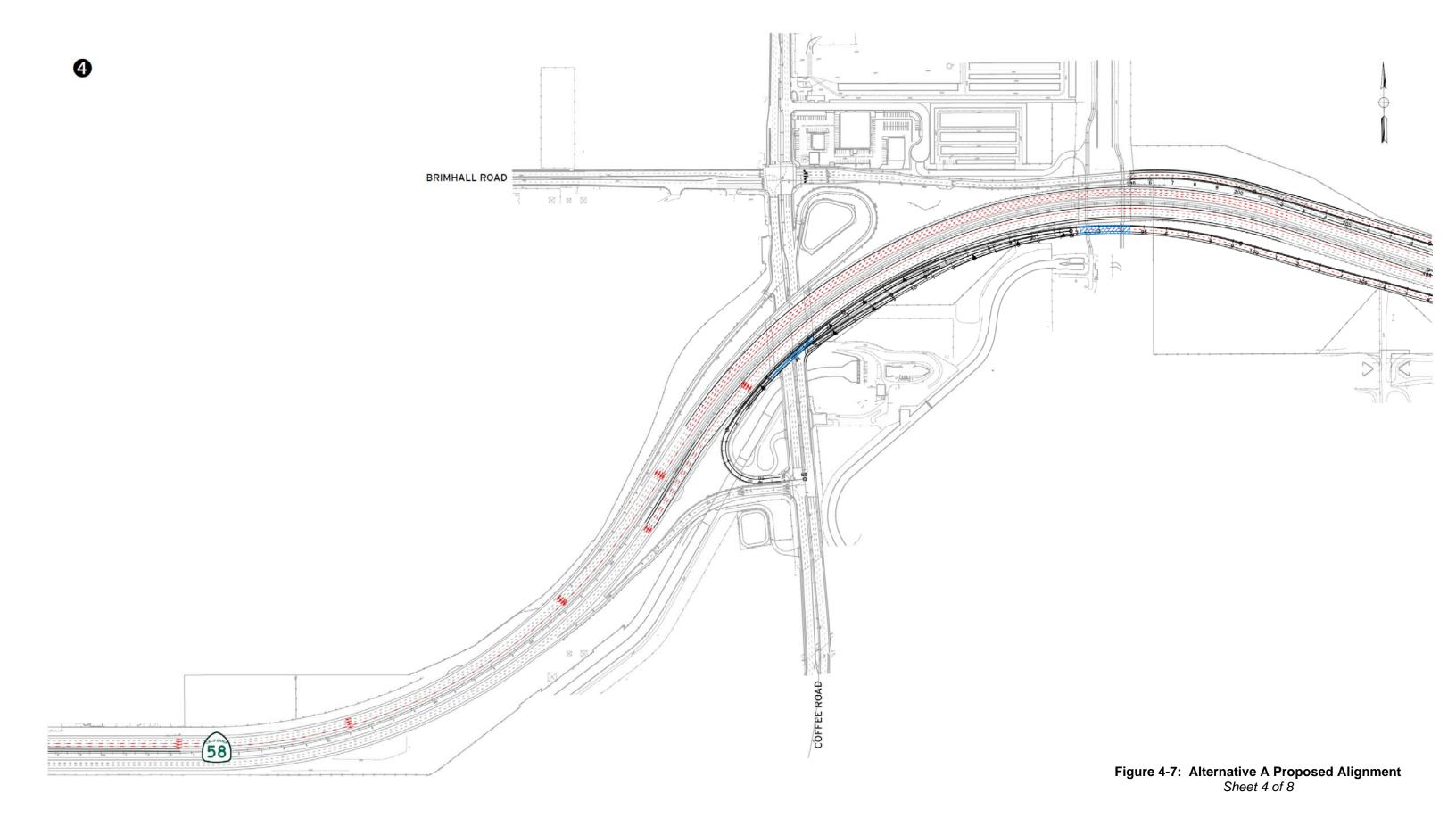
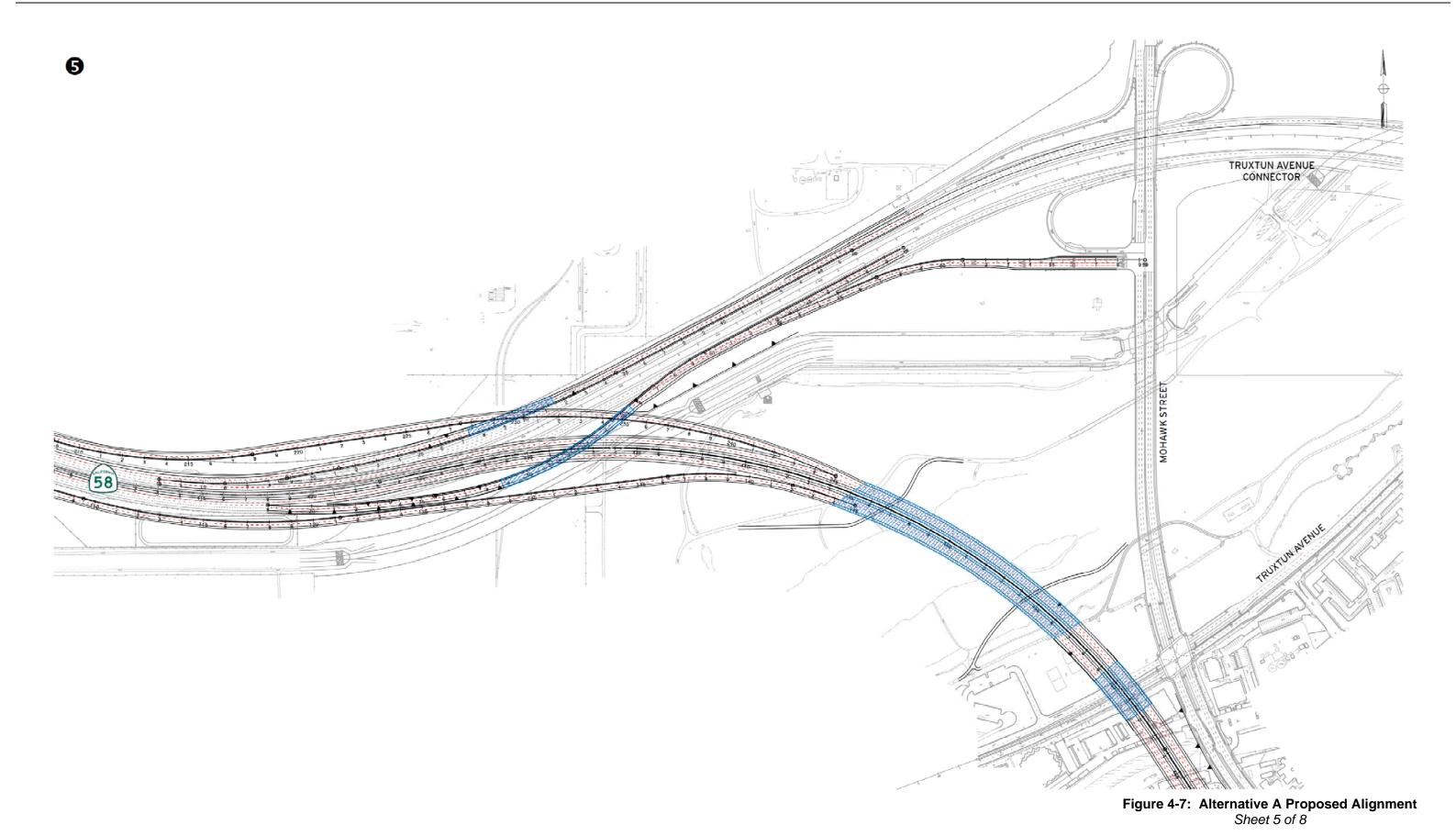


Figure 4-7: Alternative A Proposed Alignment Sheet 3 of 8





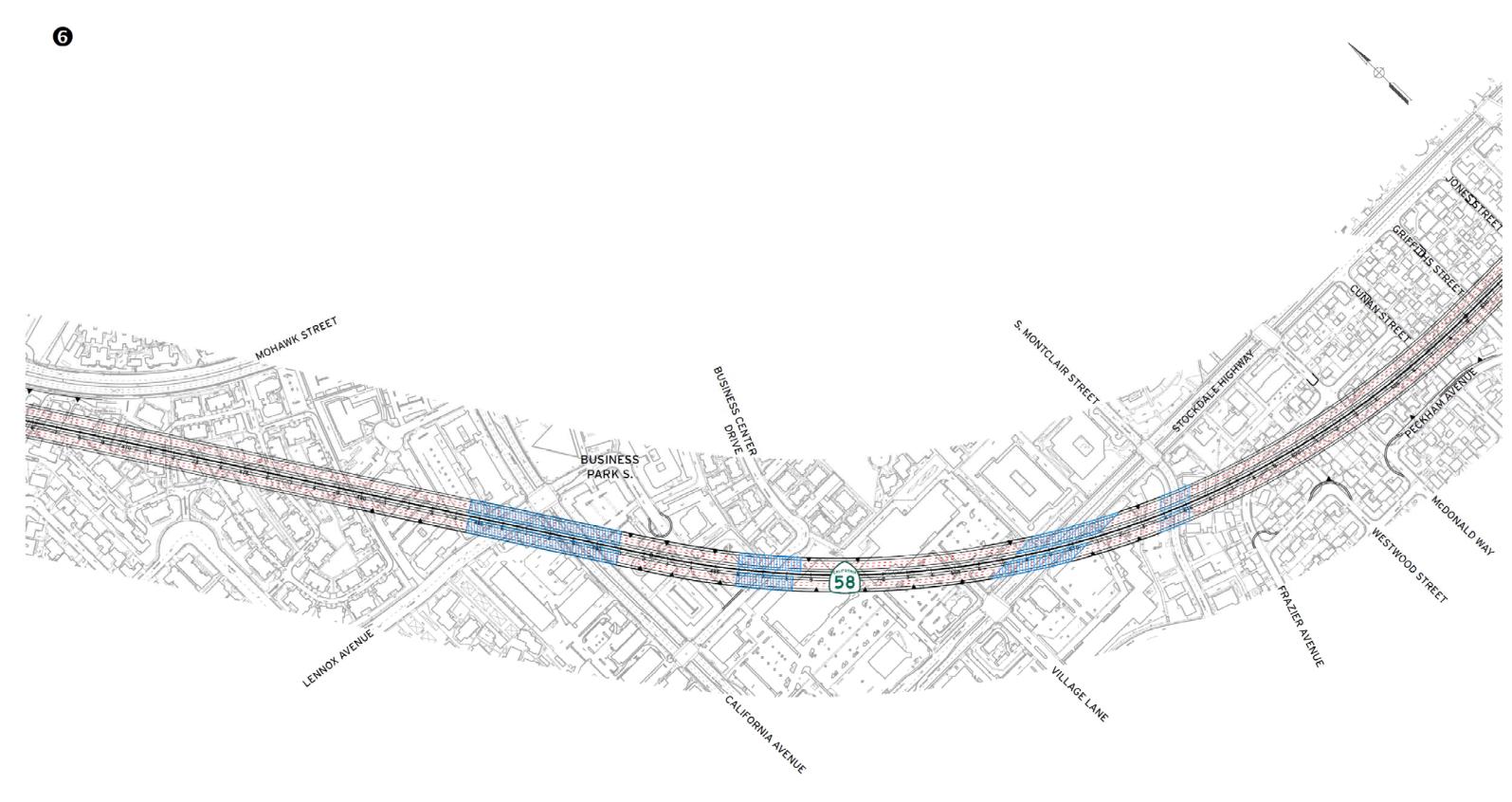


Figure 4-7: Alternative A Proposed Alignment Sheet 6 of 8



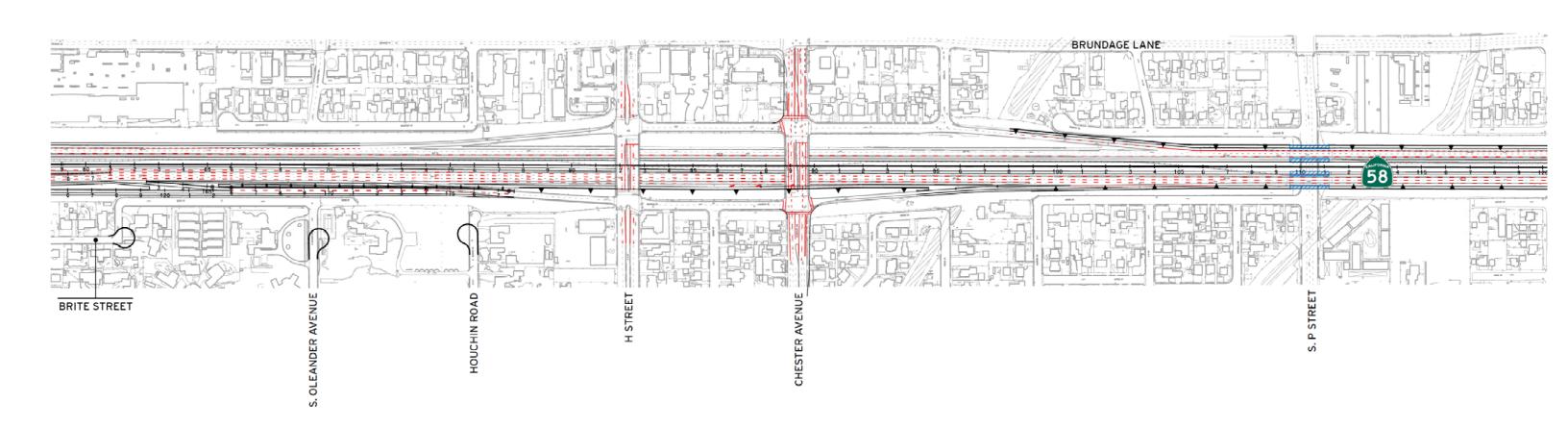
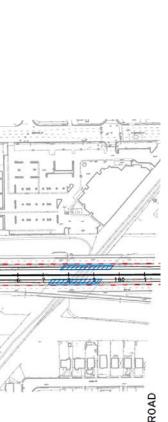


Figure 4-7: Alternative A Proposed Alignment Sheet 7 of 8





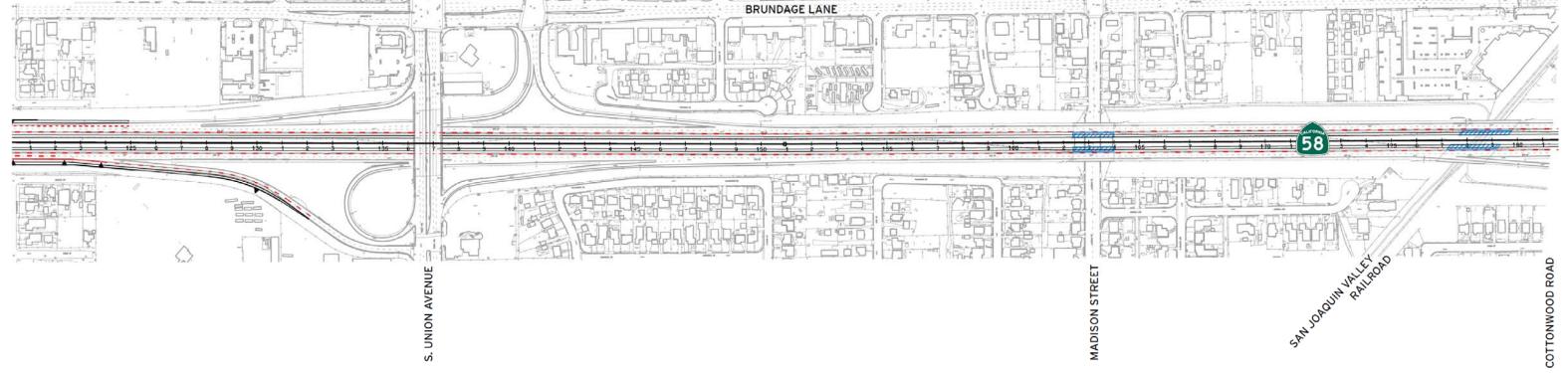


Figure 4-7: Alternative A Proposed Alignment Sheet 8 of 8

A. State Route 58 and Westside Parkway

State Route 58 West	hound	MAINLINE		V	VEST BELTV	VAY		MAINLINE		ALLEN ROA	D	MAINLINE		CALL	OWAY D	RIVE		MAINLINE	C	OFFEE ROAL		MAINLINE
tate Route 36 West	bound	WAINLINE	ON	MAINLINE	ON	MAINLINE	OFF	WAINLINE	ON	MAINLINE	OFF	MAINLINE	ON	MAINLINE	OFF	MAINLINE	OFF	MAINLINE	ON	MAINLINE	ON	MAINLINE
	AM Peak	1,745	305	1,440	320	1,120	1,500	2,620	350	2,270	1,150	3,420	500	2,920	910	3,830	650	4,480	300	4,180	150	
2038 ALTERNATIVE A	PM Peak	2,280	345	1,935	425	1,510	2,385	3,895	370	3,525	2,230	5,755	880	4,875	1,350	6,225	995	7,220	550	6,670	240	
	ADT	28,754	5,750	23,004	5,970	17,034	24,925	41,959	3,500	38,459	25,735	64,194	8,285	55,909	17,700	73,609	10,525	84,134	5,530	78,604	2,885	
					<u> </u>		ris .	1		ALLEN						CALLOWAY		÷				
esta Davita 50 Fact	haund	MAINLING		V	VEST BELTY	VAY		MAINLINE		ALLEN ROA	D	MAINI INF		CAL	LOWAY	DRIVE		MAINI INF	C	OFFEE ROAL		From
State Route 58 East	bound	MAINLINE	OFF	MAINLINE	ON	MAINLINE	ON	MAINLINE	OFF	MAINLINE	ON	MAINLINE	OFF	MAINLIN	E ON	MAINLINI	E ON	MAINLINE		OFF		MAINLINE
	AM Peak	2,230	745	1,485	1,525	3,010	290	3,300	380	2,920	2,000	4,920	960	3,960	940	4,900	1,150	6,050		1,025		5,025
038 ALTERNATIVE A	PM Peak	2,220	870	1,350	1,340	2,690	275	2,965	310	2,655	1,875	4,530	800	3,730	1,040		1,110	5,880		825		5,055
	ADT	28.605	11,090	17,515	18,820	36,335	3,450	39,785	4,595	35,190	24,660	59,850	11,800	4 8,050	11,555	59,605	15,275	74.880	1	11,735		63,145



MAINLINE	MOHAWK STREET/	TRUXTUN	AVENUE		STATE ROUTE 99 SYSTEM INTERCHANGE						H ST/CHESTER AVE				UNION AVENUE					COTTONWOOD ROAD				State Route 58	
	MOHAWK/ TRUXTUN ON	TRUXTUN ON MAINLINE COFFEE OFF		MAINLINE	ON	MAINLINE	OFF	MAINLINE	OFF	MAINLINE	ON	ON MAINLINE OFF		MAINLINE	ON	MAINLINE	E ON	MAINLINE OFF		MAINLINE	ON	MAINLINE	OFF	MAINLINE	
4,030	2,160	1,870	1,950	3,820	1,500	2,320	1,200	3,520	1,185	4,705	635	4,070	1,030	5,100	565	4,535	365	4,170	1,180	5,350	315	5,035	235	5,270	AM Peak 2038
6,430	3,780	2,650	1,575	4,225	1,630	2,595	1,225	3,820	1,120	4,940	660	4,280	1,115	5,395	770	4,625	360	4,265	990	5,255	530	4,725	305	5,030	PM Peak ALTERNATIVE
75,719	42,215	33,504	23,775	57,279	22,400	34,879	14,850	49,729	16,050	65,779	12,410	53,369	16,925	70,294	9,644	60,650	4,980	55,670	13,220	68,890	5,810	63,080	3,205	66,285	

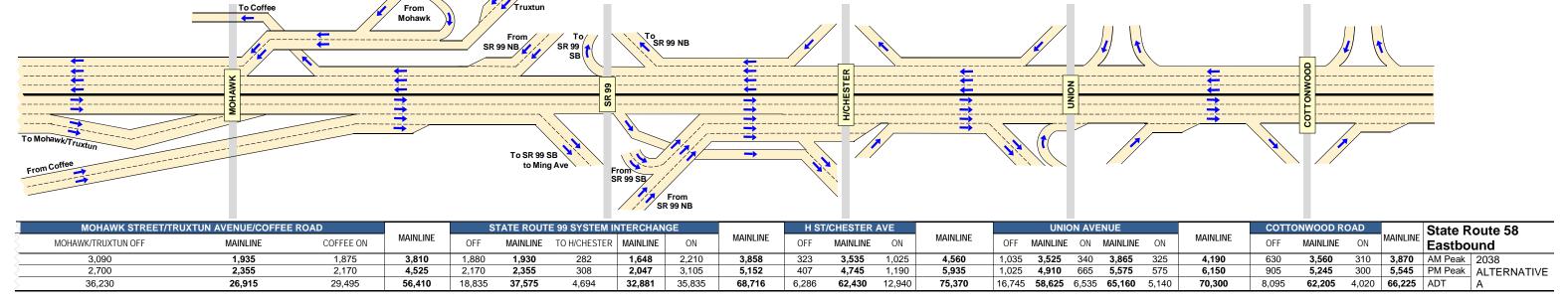


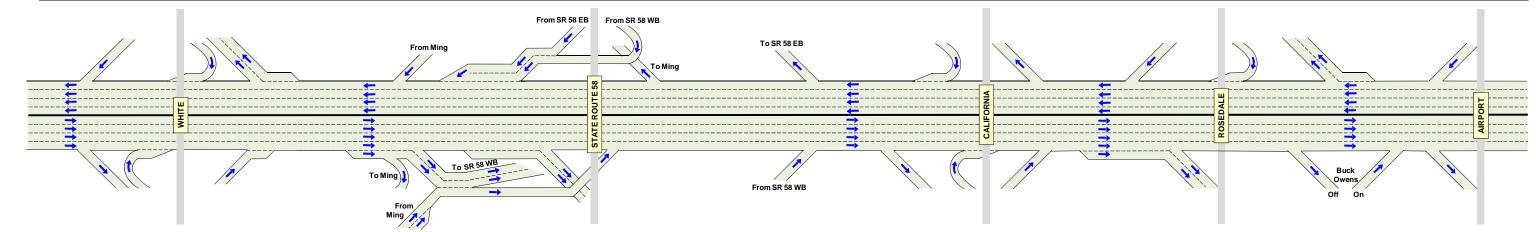
Figure 4-8: Year 2038 Alternative A Freeway Lane Configuration and Forecast Traffic Volumes (1 of 2)

B. State Route 99



NOT TO SCA

State Route Southboun	e 99	MAINLINE		WHITE LANE				MAINLINE	MING AVENUE/STATE ROUTE 58								CAL	LIFORNIA AVENI	MAAINII INIT		ROSE	DALE HIGH	MAINLINE	AIRPORT DRIVE	MAINLINE			
	nd	WAINLINE	ON	MAINLINE	ON	MAINLINE	OFF	MAINLINE	MING ON	MAINLINE	SR 58 ON	MAINLINE	MING OFF	MAINLINE	SR 58 EB OFF	MAINLINE	ON	MAINLINE	OFF	MAINLINE	ON	MAINLINE	ON	MAINLINE	OFF	IVIAINLINE	ON	WAINLINE
2038	AM Peak	5,565	445	5,120	260	4,860	1,670	6,530	785	5,745	2,525	3,220	935	4,155	955	5,110	560	4,550	1,495	6,045	835	5,210	1,100	4,110	1,375	5,485	1,450	4,035
ALTERNATIVE	PM Peak	7,560	645	6,915	550	6,365	2,070	8,435	845	7,590	2,740	4,850	1,205	6,055	1,440	7,495	940	6,555	1,655	8,210	1,340	6,870	1,375	5,495	1,310	6,805	2,070	4,735
A	ADT	95,510	9,505	86,005	7,510	78,495	20,875	99,370	10,715	88,655	25,815	62,840	15,435	78,275	15,715	93,990	10,325	83,665	21,800	105,465	13,125	92,340	16,000	76,340	15,300	91,640	24,105	67,535



State Route 99 Northbound	99	MANINI INIT	-	WHITE LANE					II INE	MING AVENUE/STATE ROUTE 58									MAINILINE	CALIFORNIA AVENUE						ROSEI	DALE HIGHWAY	//BUCK	MAINU INIT	AIRPORT DRIVE			
	d	MAINLINE	OFF	MAINLINE	ON	MAINL	INE ON	IVIAII	ILINE	MING OFF	MAINLINE	SR 58 WB OFF	MAINLINE	SR 58 EB OFF	MAINLINE	MING ON	MAINLINE	SR 58 WB ON	MAINLINE	OFF	MAINLINE	ON	MAINLINE	ON	MAINLINE	OFF	MAINLINE	OFF	MAINLINE	ON	IVIAIIVLIIVE	OFF	MAINLINE
2038	AM Peak	7,075	1,125	5,950	1,640	7,59	90 695	8,2	285	915	7,370	1,125	6,245	1,255	4,990	1,000	5,990	1,185	7,175	1,150	6,025	1,050	7,075	480	7,555	1,965	5,590	795	4,795	720	5,515	1,740	3,775
ALTERNATIVE	PM Peak	6,655	1,080	5,575	1,655	7,23	30 685	7,9	915	850	7,065	1,130	5,935	1,665	4,270	850	5,120	1,120	6,240	710	5,530	1,375	6,905	900	7,805	1,800	6,005	400	5,605	790	6,395	1,365	5,030
A	ADT	95,000	11,675	83,325	18,885	102,2	210 8,79	0 111	,000	11,300	99,700	15,500	84,200	20,120	64,080	9,590	73,670	16,050	89,720	5,800	83,920	16,90	0 100,820	7,200	108,020	22,255	85,765	6,875	78,890	10,800	89,690	21,750	67,940

Figure 4-8: Year 2038 Alternative A Freeway Lane Configuration and Forecast Traffic Volumes (2 of 2)

by the CORSIM traffic model. Segments experiencing LOS E or F performance in the design year of 2038 are highlighted.

With respect to 2038 AM peak hour conditions, Table 4-15 indicates that only one short segment of the freeway system, northbound State Route 99 between the White Lane loop on-ramp and the White Lane diagonal on-ramp, will operate at LOS E. Of the 74 freeway segments analyzed, 61 will operate at LOS C or better, 12 at LOS D, and one at LOS E, as mentioned above. Speeds will range between 55 and 64 mph and nearly all of the demand volumes will be served.

During the PM peak hour, traffic operations will be nearly as good as AM peak hour conditions. Of the 74 freeway segments analyzed, 50 will operate at LOS C or better, 21 segments will operate at LOS D, and three segments will operate at LOS E or F.

- Southbound State Route 99 between the Rosedale Highway loop on-ramp and the Rosedale Highway diagonal on-ramp (LOS E)
- Southbound State Route 99 between the Rosedale Highway diagonal on-ramp and the California Avenue off-ramp (LOS F)
- Southbound State Route 99 between the Ming Avenue on-ramp and the White Lane off-ramp (LOS F).

The percentage of demand traffic volumes served will fall below 90 percent in two reaches of State Route 99.

- Northbound between the California loop on-ramp and the Airport Drive off-ramp (85 to 88 percent served)
- Southbound between the Ming Avenue on-ramp and south of the White Lane diagonal on-ramp (86 to 89 percent served).

Along the Westside Parkway/State Route 58, speeds will range between 58 and 63 mph. Along State Route 99, speeds, for the most part, will range between 51 and 63 mph.

Motorists will experience slower speeds, below 50 mph, on four segments of southbound State Route 99 during the PM peak hour.

In addition to analyzing freeway mainline segments, freeway on-ramp and off-ramp performance and merge/diverging conditions of the freeways were analyzed using the same CORSIM traffic simulation model. These results are reported in Tables 4-17 through 4-20. In cases where a low percentage of on-ramp traffic is served, such as the White Lane loop on-ramp to northbound State Route 99, traffic would be expected to spill back onto local streets until such time that motorists divert to alternate routes.

Traffic level of service at ramp termini and study area intersections are reported in Table 4-21 for build alternative A conditions. The proposed traffic lane geometric configurations and demand traffic volumes are illustrated on Figure 3-11. For the ramp terminal and other study intersections reported in Table 4-21, all but eight intersections perform at LOS D or better during year 2038 AM peak hour demand conditions; while 16 of 79 intersections experience LOS E or F conditions during the PM peak hour. This performance is improved from the no-build condition, as discussed in Chapter 6.

Detailed level of service input parameters and output statistics for each study intersection are provided in Volume II (part 1 of 2) of this report.

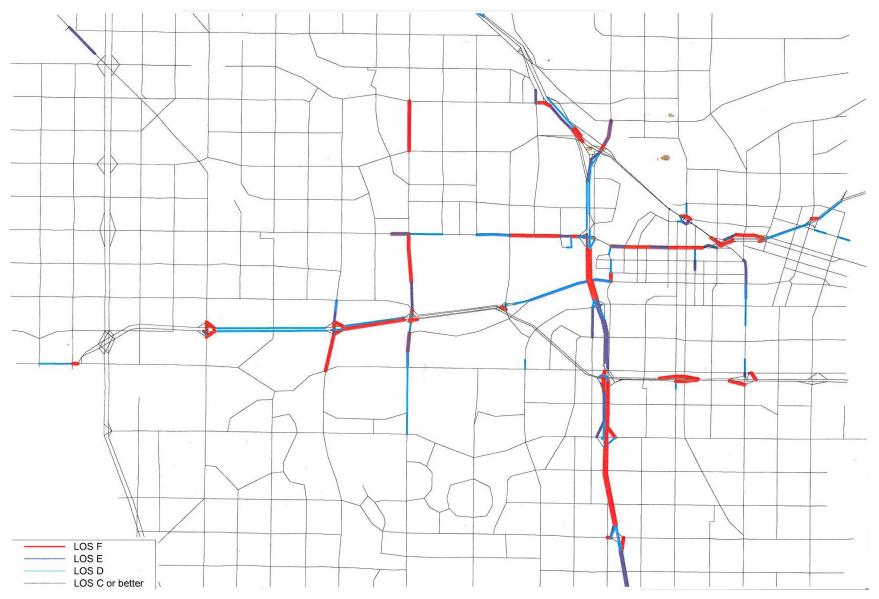


Figure 4-9: Locations of Congestion for Alternative A

Table 4-15. AM Peak Hour Measures of Effectiveness for Alternative A—Freeway Mainline Segments (1 of 2)

								YEAR 2018												Υ	EAR 2038					
FREEWAY SEGMENT	DEMAND (vph)	SERVED (vph)	% SERVED	TRAVEL TIME (sec/veh)	DELAY TIME (sec/veh)	SPEED (mph)	DENSITY (veh/ln/mi)	LOS (HCM 2000)	CUMULATIVE TIME (sec/veh)	VEHICLE MILES (VMT)	VEHICLE HOURS (VHT)	VEHICLE DELAY (VHD)	CORSIM SERVED VOLUMES/ LANES (vphl)	DEMAND (vph)	SERVED (vph)	% SERVED	TRAVEL TIME (sec/veh)	DELAY TIME (sec/veh)	SPEED (mph)	DENSITY (veh/ln/mi)	LOS (HCM 2000)	CUMULATIVE TIME (sec/veh)	VEHICLE MILES (VMT)	VEHICLE HOURS (VHT)	VEHICLE DELAY (VHD)	CORSIM SERVED VOLUMES/ LANE (vphl)
State Route 99 NB Mainline														<u>'</u>												
SR 99 NB south end of the network to White Lane off-ramp	5,070	5,070	100%	50	1	63	20	С	50	4462	71	2	1271	7,075	7,075	100%	55	6	59	31	D	55	6310	107	11	1772
White Lane off-ramp to White Lane loop on-ramp	4,390	4,390	100%	19	1	63	18	В	69	1473	24	1	1100	5,950	5,848	98%	20	1	61	24	С	74	1959	32	2	1462
White Lane loop on-ramp to White Lane direct on-ramp	6,020	5,845	97%	7	1	53	28	D	77	614	12	2	1461	7,590	7,162	94%	8	2	47	38	E	82	755	16	5	1791
White Lane direct on-ramp to Ming Avenue off-ramp	6,724	6,623	98%	77	5	60	25	С	154	8566	142	9	1325	8,285	8,015	97%	80	8	58	32	D	162	10357	178	18	1603
Ming Avenue off-ramp to C-D (SR 58 WB) off-ramp	5,985	5,985	100%	11	1	61	20	С	165	1166	19	1	1220	7,370	7,370	100%	11	1	60	25	С	174	1411	24	2	1475
SR 58 WB off-ramp to SR 58 EB off-ramp	5,140	5,140	100%	27	1	61	19	С	192	2362	39	2	1152	6,245	6,117	98%	27	2	60	23	С	201	2783	46	3	1359
SR 58 EB off-ramp to Ming Avenue on-ramp	4,114	4,114	100%	25	1	63	17	В	217	1824	29	1	1051	4,990	4,990	100%	25	1	62	20	С	226	2167	35	1 1	1249
Ming Avenue on-ramp to SR 58 on-ramp	4,915	4,915	100%	8	1	61	20	С	224	634	10	1	1238	5,990	5,888	98%	8	1	60	25	С	234	754	13	1 1	1472
SR 58 on-ramp to California Avenue off-ramp	6,014	6,014	100%	39	3	59	25	С	264	3951	67	6	1523	7,175	7,163	100%	40	5	58	30	D	274	4660	81	9	1791
California Avenue off-ramp to California Avenue loop on-ramp	5,044	5,041	100%	16	1	62	20	С	280	1424	23	1	1260	6,025	6,025	100%	16	1	62	24	С	291	1701	28	1	1506
California Avenue loop on-ramp to California Avenue direct on-ramp	5,904	5,691	96%	9	1	60	24	С	289	846	14	1	1423	7,075	6,780	96%	9	1	58	29	D	300	1008	17	2	1695
California Avenue direct on-ramp to Rosedale Highway off-ramp	6,273	5,842	93%	32	2	60	23	С	320	3060	51	3	1298	7,555	6,881	91%	33	3	57	28	D	333	3572	62	6	1529
Rosedale Highway off-ramp to Buck Owens Boulevard/Sillect Avenue off-ramp	4,394	4,227	96%	16	1	63	17	В	337	1201	19	1	1057	5,590	5,192	93%	17	1	61	21	С	349	1475	24	1 1	1298
Buck Owens Boulevard/Sillect Avenue off-ramp to Buck Owens Boulevard/Sillect Avenue on-ramp	3,824	3,722	97%	24	1	63	15	В	361	1547	25	1	931	4,795	4,432	92%	24	1	63	18	В	373	1842	29	1 1	1108
Buck Owens Boulevard/Sillect Avenue on-ramp to Airport Drive off-ramp	4,224	4,088	97%	34	2	61	17	В	394	2324	38	2	1022	5,515	5,124	93%	35	3	59	22	С	408	2914	49	4	1281
Airport Dr off-ramp to SR 99 NB north end of the network	2,719	2,609	96%	40	1	63	10	Α	434	1842	29	1	652	3,775	3,556	94%	41	1	63	14	В	448	2512	40	1 1	889
State Route 99 SB Mainline																										
SR 99 SB north end of the network to Airport Drive on-ramp	3,210	3,210	100%	48	1	64	13	В	48	2749	43	1	806	4,035	4,035	100%	48	1	64	16	В	48	3460	54	1	1014
Airport Drive on-ramp to Rosedale Highway off-ramp	4,490	4,490	100%	31	2	62	18	В	79	2393	39	2	998	5,485	5,450	99%	32	3	60	22	С	80	2919	49	4	1211
Rosedale Highway off-ramp to Rosedale Highway loop on-ramp	3,830	3,830	100%	25	1	63	15	В	104	1650	26	1	959	4,110	4,110	100%	25	1	63	16	В	105	1769	28	1 1	1028
Rosedale Highway loop on-ramp to Rosedale Highway direct on-ramp	4,775	4,775	100%	10	1	57	18	С	113	727	13	1	1200	5,210	5,210	100%	10	1	55	21	С	115	796	14	2	1313
Rosedale Highway direct on-ramp to California Avenue off-ramp	5,510	5,510	100%	37	3	59	23	С	150	3361	57	5	1385	6,045	6,045	100%	38	5	57	26	D	153	3676	65	8	1518
California Avenue off-ramp to California Avenue on-ramp	4,340	4,340	100%	13	0	62	17	В	164	1011	16	1	1085	4,550	4,550	100%	13	1	62	19	С	167	1074	17	1 1	1154
California Avenue on-ramp to SR 58 EB off-ramp	4,805	4,805	100%	27	2	61	19	С	191	2218	37	2	1202	5,110	5,110	100%	28	2	60	21	С	194	2399	40	3	1299
SR 58 EB off-ramp to Ming Avenue off-ramp	3,900	3,825	98%	28	1	62	15	В	219	1856	30	1	956	4,155	4,155	100%	28	1	62	17	В	223	2053	33	1 1	1058
Ming Avenue off-ramp to SR 58 WB on-ramp	3,029	2,995	99%	22	1	63	12	В	241	1141	18	0	749	3,220	3,220	100%	22	1	63	13	В	244	1257	20	1 1	825
SR 58 WB on-ramp to Ming Avenue on-ramp	4,614	4,530	98%	51	2	63	14	В	292	4048	65	2	1007	5,745	5,745	100%	52	3	62	18	В	296	5174	84	4	1287
Ming Avenue on-ramp to White Lane off-ramp	5,129	5,041	98%	60	3	61	20	С	352	5172	84	4	1260	6,530	6,516	100%	63	6	59	27	D	360	6749	115	11	1629
White Lane off-ramp to White Lane loop on-ramp	3,709	3,625	98%	19	0	63	14	В	371	1215	19	0	806	4,860	4,841	100%	19	1	63	19	С	379	1623	26	1	1076
White Lane loop on-ramp to White Lane direct on-ramp	3,884	3,791	98%	10	0	63	15	В	381	642	10	0	948	5,120	4,953	97%	10	0	62	20	С	389	839	13	1	1238
White Lane direct on-ramp to SR 99 SB south end of the network	4,254	4,008	94%	52	2	63	16	В	433	3626	58	2	1002	5,565	5,193	93%	52	2	62	21	С	441	4696	75	3	1298
State Route 99 NB Collector-Distributor	•				•								•									•				
SR 99 NB C-D on-ramp to Westside Parkway C-D off-ramp	2,010	2,010	100%	18	1	53	13	В	26	546	10	0	823	2,500	2,500	100%	18	1	53	16	В	26	674	13	0	1017
State Route 99 SB Collector-Distributor																										
Westside Parkway C-D on-ramp to SR 99 SB C-D on-ramp	1,585	1,547	98%	7	0	47	11	А	15	145	3	0	773	2,525	2,525	100%	7	1	45	19	С	16	235	5	0	1263
Bottleneck									1		•											1				

Bottlene Queue

Table 4-15. AM Peak Hour Measures of Effectiveness for Alternative A—Freeway Mainline Segments (2 of 2)

							<u>Y</u>	EAR 2018												YE	EAR 2038					
				TRAVEL	DELAY				CUMULATIVE	VEHICLE	VEHICLE	VEHICLE	CORSIM SERVED				TRAVEL	DELAY				CUMULATIVE	VEHICLE	VEHICLE	VEHICLE	CORSIM SERVED
FREEWAY SEGMENT	DEMAND (vph)	SERVED (vph)		TIME (sec/veh)	TIME (sec/veh)	SPEED (mph)	DENSITY (veh/ln/mi)	LOS (HCM 2000)	TIME (sec/veh)	MILES (VMT)	HOURS (VHT)	DELAY (VHD)	VOLUMES/ LANES (vphl)	DEMAND (vph)	SERVED (vph)	% SERVED	TIME	TIME (sec/veh)	SPEED (mph)	DENSITY (veh/ln/mi)	LOS (HCM 2000)	TIME	MILES (VMT)	HOURS (VHT)	DELAY (VHD)	VOLUMES/ LANES (vphl)
Westside Parkway EB Mainline																										
Westside Parkway west end of the network to West Beltway off-ramp														2,230	2,230	100%	6	2	47	24	С	6	186	4	1	1140
West Beltway off-ramp to West Beltway loop on-ramp							— NOT	APPLICABLE	—					1,485	1,485	100%	14	1	61	12	В	20	349	6	0	743
West Beltway loop on-ramp to West Beltway direct on-ramp	4.045	1 4 000	000/	l 00	1 2	l c4	۱ ،	1 ^	l 00	1405	22		504	3,010	2,796	93%	9 57	1 2	61	15 16	B B	29 86	436	7 49	0	932 1024
West Beltway direct on-ramp to Allen Road off-ramp Allen Road off-ramp to Allen Road on-ramp	1,015 956	1,009 956	99% 100%	83 32	3	61 64	5	Α Δ	83 115	1425 559	23 9	0	504 327	3,300 2,920	3,073 2,699	93% 92%	33	1	62 63	14	В	119	3023 1537	24	2	900
Allen Road on-ramp to Calloway Drive off-ramp	2,665	2,665	100%	59	3	62	14	В	174	2706	44	2	889	4,920	4,665	95%	61	4	60	26	c	180	4745	79	6	1555
Calloway Drive off-ramp to Calloway Drive loop on-ramp	2,140	2,138	100%	24	0	64	11	В	198	926	14	0	713	3,960	3,796	96%	25	1	63	20	С	204	1645	26	1	1265
Calloway Drive loop on-ramp to Calloway Drive direct on-ramp	2,880	2,833	98%	11	1	61	15	В	209	536	9	1	944	4,900	4,653	95%	11	1	59	26	D	216	882	15	1	1551
Calloway Drive direct on-ramp to Coffee Road off-ramp	3,940	3,907	99%	49	3	61	21	С	258	3273	53	3	1302	6,050	5,867	97%	51	5	59	32	D	267	4956	84	8	1956
Coffee Road off-ramp to Truxtun Avenue/Mohawk Street off-ramp Truxtun Avenue off-ramp to Coffee Road on-ramps	3,299 909	3,223 886	98% 97%	58 31	2	62 65	13 7	В	316 347	3214 494	51 o	2	921 443	5,025 1,935	4,836 1,839	96%	59 32	3	61 63	21 15	C B	326 358	4821 1026	79 16	4 0	1382 919
Coffee Road on-ramps to SR 99 SB and Ming Avenue C-D off-ramp	2,311	2,254	98%	108	3	63	12	В	455	494	68	2	751	3,810	3,700	95% 97%	32 111	6	61	20	C	468	6972	114	6	1233
SR 99 SB off-ramp (and C-D Ming Avenue) to H Street off-ramp	1,181	1,134	96%	23	0	63	9	A	478	451	7	0	567	1,930	1,836	95%	23	1	63	15	В	491	729	12	0	918
H Street off-ramp to SR 99 NB and SB on-ramp	921	872	95%	30	0	64	7	Α	507	463	7	0	436	1,648	1,564	95%	30	1	63	12	В	522	830	13	0	782
SR 99 NB and SB on-ramp to Chester Avenue on-ramp	2,486	2,486	100%	46	4	61	9	Α	554	1959	32	2	718	3,535	3,408	96%	47	4	60	12	В	569	2662	45	4	974
Chester Avenue on-ramp to Union Avenue off-ramp	3,236	3,236	100%	27	1	62	13	В	580	1490	24	1	927	4,560	4,401	97%	27	2	61	18	С	596	2022	33	2	1257
Union Avenue off-ramp to Union Avenue loop on-ramp	2,521	2,521	100%	17	0	63	13	В	597	736	12	0	845	3,525	3,405	97%	17	1	63	18	С	613	989	16	1	1135
Union Avenue loop on-ramp to Union Avenue direct on-ramp Union Avenue direct on-ramp to Cottonwood Road off-ramp	2,841 3,185	2,797 3,165	98% 99%	31	0	62 62	15 17	В	605 636	391 1681	6 27	0	932 1055	3,865 4,190	3,698 4,044	96% 97%	8 31	0 2	62 62	20 22	C	621 652	518 2149	8 35	0 2	1233 1348
Cottonwood Road off-ramp to Cottonwood Road on-ramp	2,771	2,746	99%	28	1	63	14	B	663	1328	21	1 1	915	3,560	3,445	97%	28	1	63	18	C	680	1666	27	1	1148
Cottonwood Road on-ramp to SR 58 east end of the network	3,071	3,061	100%	25	1	63	16	В	688	1336	21	1	1020	3,870	3,739	97%	25	1	62	20	c	705	1631	26	1	1246
Westside Parkway WB Mainline	<u> </u>				<u> </u>	I.		<u>I</u>													I.		<u> </u>	<u> </u>		
SR 58 east end of the network to Cottonwood Road off-ramp	3,475	3,475	100%	26	1	64	18	С	26	1589	25	0	1165	5,270	5,270	100%	26	1	63	28	D	26	2411	38	1	1768
Cottonwood Road off-ramp to Cottonwood Road on-ramp	3,220	3,215	100%	18	1	63	17	В	43	988	16	0	1072	5,035	5,035	100%	18	1	62	27	D	44	1555	25	1	1688
Cottonwood Road on-ramp to Brundage Lane off-ramp	3,480	3,453	99%	38	1	62	18	С	81	2285	37	1	1151	5,350	5,350	100%	44	7	56	33	D	88	3657	65	10	1785
Brundage Lane off-ramp to Brundage Lane on-ramp	2,811	2,793	99%	17	1	63	15	В	98	824	13	0	931	4,170	4,082	98%	17	1	61	22	С	105	1205	20	1	1361
Brundage Lane on-ramp to Union Avenue on-ramp	3,195	3,121	98%	12	1	62	17	В	110	639	10	1	1040	4,535	4,409	97%	12	1	61	24	С	117	904	15	1	1470
Union Avenue on-ramp to Chester Avenue off-ramp	3,761 2,785	3,717 2,785	99% 100%	23 37	1	62 63	15 15	В	133 170	1446 1826	24 29	1	1062 935	5,100 4,070	4,963 3,952	97% 97%	23 38	1 2	61 62	20 21	C C	140 178	1931 2573	32 42	2 2	1418 1317
Chester Avenue off-ramp to H Street on-ramp H Street on-ramp to SR 99 NB off-ramp	3,480	3,468	100%	24	1	61	14	B	195	1443	29	1 1	991	4,070	4,612	98%	25	1	61	19	C	203	1920	32	2	1317
SR 99 NB off-ramp to SR 99 SB off-ramp	2,381	2,381	100%	21	1	62	13	В	216	870	14	0	796	3,520	3,444	98%	21	1	61	19	C	224	1253	20	1	1148
SR 99 SB off-ramp to SR 99 NB on-ramp	1,356	1,356	100%	23	1	64	11	Α	239	570	9	0	692	2,320	2,218	96%	24	1	62	18	В	248	913	15	1	1109
SR 99 NB on-ramp to Coffee Road off-ramp	2,566	2,566	100%	105	3	63	14	В	344	4741	76	3	867	3,820	3,753	98%	107	6	61	20	С	355	6842	111	6	1251
Coffee Road off-ramp to Mohawk Street/Truxtun Avenue on-ramp	896	896	100%	23	0	64	7	Α	367	374	6	0	460	1,870	1,843	99%	23	1	63	15	В	378	748	12	0	921
Mohawk Street/Truxtun Avenue on-ramp to Coffee Road loop on-ramp	2,516	2,473	98%	53	1	64	10	A	420	2301	36	1	618	4,030	3,966	98%	53	2	63	16	В	431	3688	59	2	991
Coffee Road loop on-ramp to Coffee Road direct on-ramp	2,646	2,584	98%	12 48	0	64 63	10 11	A	432	563 2357	9 37	0	646 700	4,180	4,101	98%	12 48	0 2	63	16	B B	444	893	14	0 2	1025 1087
Coffee Road direct on-ramp to Calloway Drive direct off-ramp Calloway Drive direct off-ramp to Calloway Drive loop off-ramp	2,901 2,251	2,800 2,185	97% 97%	11	0	61	9	A A	480 491	414	7	0	700 546	4,480 3,830	4,348 3,689	97% 96%	11	0	62 62	17 15	В	492 503	3653 699	59 11	0	922
Calloway Drive loop off-ramp to Calloway Drive on-ramp	1,301	1,234	95%	24	0	64	6	A	515	528	8	0	411	2,920	2,800	96%	24	1	63	15	В	528	1197	19	1	933
Calloway Drive on-ramp to Allen Road off-ramp	1,621	1,552	96%	57	2	63	8	Α	572	1545	25	1	517	3,420	3,287	96%	58	3	62	18	В	586	3270	53	3	1096
Allen Road off-ramp to Allen Road on-ramp	536	536	100%	26	0	65	3	Α	598	262	4	0	187	2,270	2,168	96%	27	1	64	11	В	612	1016	16	0	723
Allen Road on-ramp to West Beltway off-ramp	656	656	100%	89	1	64	5	Α	687	1091	17	0	230	2,620	2,492	95%	57	2	62	13	В	669	2460	39	1	831
West Beltway off-ramp to West Beltway loop on-ramp														1,120	1,090	97%	22	0	64	9	Α	692	433	7	0	545
West Beltway loop on-ramp to West Beltway direct on-ramp							— NOT A	PPLICABLE	_					1,440	1,318	92%	6	0	62	11	A	698	131	2	0	659
West Beltway direct on-ramp to Westside Parkway west end of the network														1,745	1,600	92%	6	0	60	13	В	703	154	3	0	800
Westside Parkway EB Collector-Distributor 1	1 000	1 000	4000/	1 40	1 .	10	1 00		1 10	1 404		1 0		1 045	0.45	1000/	40	. 1		1 40		1 40	1 440	1 0 1		
Begin C-D (Coffee Road loop on-ramp to Coffee Road direct on-ramp)	830 1,400	830 1,378	100% 98%	12 62	1 2	49 53	22 21	C C	12 73	134 1248	3 24	0	833 689	915 1,875	915 1,875	100% 100%	12 62	1 2	49 53	19 18	C B	12 73	149 1699	3 32	0	928 938
Coffee Road direct on-ramp to end of C-D network (Westside Parkway EB) Westside Parkway EB Collector-Distributor 2	1,400	1,3/8	96%	02		- U.S	41		13	1248	24	_ '	009	1,0/5	1,0/5	100%	02	۷	აა	10	_ B	13	1099	ა∠		930
*	2 200	2 252	000/	20	1	52	20		20	066	10	1 1	0/1	3,000	3 030	080/	20	1 1	52	22		20	1242	24	1 1	1212
Begin C-D to Mohawk Street off-ramp Mohawk Street off-ramp to Mohawk Street on-ramp	2,390 1,185	2,353 1,182	98% 100%	28 26	0	53 54	20 11	В	28 54	966 462	18 9	0	941 591	3,090 1,580	3,030 1,536	98% 97%	28 26	1	52 54	23 14	В	28 55	1243 600	24 11	0	1212 768
Westside Parkway EB Collector-Distributor 3	1,100	1,102	1.5078							702	3		551	1,000	1,000	J1 /0	20		J-7				, 500]	J	, 00
Westside Parkway EB C-D on-ramp to SR 99 SB off-ramp	1,130	1,106	98%	6	1 0	52	17	B	6	90	2	0	553	1,880	1,851	98%	6	0	52	15	В	6	151	2	0 1	926
SR 99 SB off-ramp to SR 99 SB C-D on-ramp	570	557	98%	26	1	47	8	A	32	188	4	0	557	555	546	98%	26	1	47	7	A	32	184	4	0	546
SR 99 SB C-D on-ramp to Ming Avenue	1,440	1,373	95%	28	1	48	18	c	60	515	11	0	687	1,490	1,468	99%	28	1	48	15	В	60	551	11	0	734
Westside Parkway WB Collector-Distributor																										
Truxtun Avenue on-ramp to Mohawk Street loop on-ramp	780	749	96%	14	0	64	13	В	14	187	3	0	374	800	780	98%	14	0	64	6	Α	14	195	3	0	390
Mohawk Street loop on-ramp to Mohawk Street direct on-ramp	1,225	1,180		15	1	60	22	С	29	292	5	0	590	1,305	1,270	97%	14	1	62	10	Α	28	314	5	0	635
Mohawk Street direct on-ramp to Westside Parkway on-ramp (end of C-D network)	1,620	1,555		17	1	59	24	С	46	442	7	1	778	2,160	2,127	98%	17	1	61	14	В	45	604	10	1	1063
Bottleneck															•		•									



Table 4-16. PM Peak Hour Measures of Effectiveness for Alternative A—Freeway Mainline Segments (1 of 2)

							Y	EAR 2018												Υ	'EAR 2038					
	DEMAND	SERVED	%	TRAVEL TIME	DELAY TIME	SPEED	DENSITY	LOS	CUMULATIVE TIME	VEHICLE MILES	HOURS	VEHICLE DELAY	CORSIM SERVED VOLUMES/		SERVED	%	TRAVEL TIME	DELAY TIME	SPEED	DENSITY	LOS	CUMULATIVE TIME	VEHICLE MILES	HOURS	VEHICLE DELAY	VOLUMES/
FREEWAY SEGMENT	(vph)	(vph)	SERVED	(sec/veh)	(sec/veh)	(mph)	(veh/ln/mi)	(HCM 2000)	(sec/veh)	(VMT)	(VHT)	(VHD)	LANES (vphl)	(vph)	(vph)	SERVED	(sec/veh)	(sec/veh)	(mph)	(veh/ln/mi)	(HCM 2000)	(sec/veh)	(VMT)	(VHT)	(VHD)	LANES (vphl)
State Route 99 NB Mainline																										
SR 99 NB south end of the network to White Lane off-ramp	4,665	4,665	100%	50	1	63	18	С	50	4106	65	2	1169	6,655	6,655	100%	51	2	62	27	D	51	5839	94	4	1664
White Lane off-ramp to White Lane loop on-ramp	3,975	3,975	100%	19	1	63	16	В	69	1334	21	1	996	5,575	5,566	100%	20	1	62	23	С	70	1864	30	2	1392
White Lane loop on-ramp to White Lane direct on-ramp	5,460	5,249	96%	7	1	56	24	С	76	552	10	1	1312	7,230	6,686	92%	7	2	51	33	D	78	703	14	3	1672
White Lane direct on-ramp to Ming Avenue off-ramp	6,065	5,919	98%	76	4	61	22	С	152	7657	125	7	1184	7,915	7,395	93%	84	12	56	30	D	161	9530	172	24	1479
Ming Avenue off-ramp to C-D (SR 58 WB) off-ramp	5,231	5,224	100%	11	1	61	17	В	163	998	16	1	1045	7,065	6,781	96%	13	2	53	26	С	175	1310	25	4	1356
SR 58 WB off-ramp to SR 58 EB off-ramp	4,401	4,332	98%	27	1	62	16	В	190	1976	32	1	963	5,935	5,456	92%	27	2	60	21	С	202	2484	42	3	1212
SR 58 EB off-ramp to Ming Avenue on-ramp	3,246	3,246	100%	25	1	63	13	В	215	1415	22	1	815	4,270	4,065	95%	25	1	63	16	В	227	1765	28	1	1016
Ming Avenue on-ramp to SR 58 on-ramp	3,951	3,915	99%	7	0	62	16	В	222	501	8	0	979	5,120	4,836	94%	8	0	61	20	С	234	619	10	1	1209
SR 58 on-ramp to California Avenue off-ramp	4,891	4,891	100%	38	2	61	19	С	260	3191	52	3	1234	6,240	6,016	96%	39	3	60	24	С	273	3897	65	5	1504
California Avenue off-ramp to California Avenue loop on-ramp	4,261	4,261	100%	16	1	63	17	В	276	1215	19	1	1076	5,530	5,308	96%	16	1	62	21	С	290	1499	24	1	1327
California Avenue loop on-ramp to California Avenue direct on-ramp	5,166	4,956	96%	9	1	60	21	С	285	737	12	1	1239	6,905	6,108	88%	9	1	57	27	D	299	911	16	2	1527
California Avenue direct on-ramp to Rosedale Highway off-ramp	5,826	5,384	92%	32	2	60	21	С	317	2833	48	3	1196	7,805	6,598	85%	36	6	53	30	D	335	3459	66	11	1466
Rosedale Highway off-ramp to Buck Owens Boulevard/Sillect Avenue off-ramp	4,066	3,787	93%	16	0	63	15	В	333	1076	17	0	947	6,005	5,209	87%	16	1	62	21	С	352	1480	24	1	1302
Buck Owens Boulevard/Sillect Avenue off-ramp to Buck Owens Boulevard/Sillect Avenue on-ramp	3,701	3,410	92%	24	1	63	13	В	357	1417	22	1	853	5,605	4,859	87%	24	1	63	19	С	375	2019	32	1	1215
Buck Owens Boulevard/Sillect Avenue on-ramp to Airport Drive off-ramp	4,416	4,124	93%	34	2	61	17	В	391	2343	39	2	1031	6,395	5,618	88%	34	3	60	24	С	410	3193	53	4	1404
Airport Dr off-ramp to SR 99 NB north end of the network	3,051	2,839	93%	40	1	63	11	В	431	2004	32	1	710	5,030	4,446	88%	41	2	62	18	В	450	3140	50	2	1112
State Route 99 SB Mainline																										
SR 99 SB north end of the network to Airport Drive on-ramp	3,550	3,550	100%	48	1	64	14	В	48	3042	48	1	892	4,735	4,735	100%	48	1	63	19	С	48	4062	64	2	1189
Airport Drive on-ramp to Rosedale Highway off-ramp	5,314	5,280	99%	32	3	60	21	С	80	2824	47	4	1173	6,805	6,805	100%	35	6	55	30	D	84	3763	68	11	1534
Rosedale Highway off-ramp to Rosedale Highway loop on-ramp	4,604	4,585	100%	25	1	62	18	С	105	1972	32	1	1146	5,495	5,495	100%	26	2	60	24	С	110	2438	41	3	1416
Rosedale Highway loop on-ramp to Rosedale Highway direct on-ramp	5,938	5,924	100%	11	2	50	26	С	116	897	18	4	1481	6,870	6,870	100%	13	5	42	36	E	123	1059	25	9	1733
Rosedale Highway direct on-ramp to California Avenue off-ramp	7,044	7,012	100%	40	6	55	31	D	155	4272	77	11	1753	8,210	8,151	99%	54	20	40	51	F	177	4868	122	45	2038
California Avenue off-ramp to California Avenue on-ramp	5,649	5,640	100%	14	1	62	23	С	169	1312	21	1	1410	6,555	6,442	98%	14	1	60	27	D	191	1500	25	2	1611
California Avenue on-ramp to SR 58 EB off-ramp	6,570	6,462	98%	29	3	57	27	D	198	2989	52	6	1615	7,495	7,202	96%	29	3	58	30	D	220	3329	58	6	1800
SR 58 EB off-ramp to Ming Avenue off-ramp	5,195	5,102	98%	28	1	62	21	С	226	2475	40	2	1276	6,055	5,814	96%	28	2	61	24	С	248	2820	46	2	1454
Ming Avenue off-ramp to SR 58 WB on-ramp	4,075	4,009	98%	22	1	63	16	В	248	1527	24	1	1002	4,850	4,617	95%	22	1	62	18	С	270	1758	28	1	1154
SR 58 WB on-ramp to Ming Avenue on-ramp	6,130	6,037	98%	52	2	62	18	С	300	5392	87	4	1341	7,590	7,109	94%	67	17	49	28	D	337	6495	131	33	1580
Ming Avenue on-ramp to White Lane off-ramp	6,780	6,685	99%	63	6	59	27	D	363	6898	117	11	1671	8,435	7,514	89%	115	57	34	56	F	452	8095	240	120	1878
White Lane off-ramp to White Lane loop on-ramp	4,765	4,678	98%	19	1	63	19	С	382	1568	25	1	1040	6,365	5,629	88%	20	1	61	23	С	471	1887	31	2	1251
White Lane loop on-ramp to White Lane direct on-ramp	5,275	5,140	97%	10	0	62	21	С	392	870	14	1	1285	6,915	6,129	89%	10	1	61	25	С	481	1038	17	1	1532
White Lane direct on-ramp to SR 99 SB south end of the network	5,755	5,449	95%	52	2	62	22	С	444	4926	79	3	1362	7,560	6,506	86%	53	3	61	27	D	534	5879	96	5	1627
State Route 99 NB Collector-Distributor	_																									_
SR 99 NB C-D on-ramp to Westside Parkway C-D off-ramp	1,925	1,925	100%	18	0	53	12	В	26	517	10	0	779	2,480	2,480	100%	18	1	53	16	В	26	677	13	0	1021
State Route 99 SB Collector-Distributor																										
Westside Parkway C-D on-ramp to SR 99 SB C-D on-ramp	1,925	1,925	100%	7	0	47	15	В	15	191	4	0	1023	2,740	2,662	97%	8	1	42	21	С	16	251	6	1	1331
	1					-			1					1	1											

Bottlene



Table 4-16. PM Peak Hour Measures of Effectiveness for Alternative A—Freeway Mainline Segments (2 of 2)

Table 4-16. PM Peak Hour Measures of Effectivenes				, .				YEAR 2018	(2 0)											Y	EAR 2038					
				TRAVEL	DELAY				CUMULATIVE	VEHICLE	VEHICLE	VEHICLE	CORSIM SERVED				TRAVEL	DELAY				CUMULATIVE	VEHICLE		VEHICLE	CORSIM SERVED
FREEWAY SEGMENT	DEMAND (vph)	SERVED (vph)	% SERVED	TIME (sec/veh)	TIME (sec/veh)	SPEED (mph)	DENSITY (veh/ln/mi)	LOS (HCM 2000)	TIME (sec/veh)	MILES (VMT)	HOURS (VHT)	DELAY (VHD)	VOLUMES/ LANES (vphl)	DEMAND (vph)	SERVED (vph)	% SERVED	TIME (sec/veh)	TIME (sec/veh)	SPEED (mph)	DENSITY (veh/ln/mi)	LOS (HCM 2000)	TIME (sec/veh)	MILES (VMT)	HOURS (VHT)	DELAY (VHD)	VOLUMES/ LANES (vphl)
Westside Parkway EB Mainline								1																		
Westside Parkway west end of the network to West Beltway off-ramp														2,220	2,217	100%	7	2	44	25	С	7	181	4	1	1109
West Beltway off-ramp to West Beltway loop on-ramp							— NОТ	APPLICABLE	_					1,350	1,267	94%	14	1	60	11	Α	21	297	5	0	634
West Beltway loop on-ramp to West Beltway direct on-ramp														2,690	2,586	96%	9	1	61	14	В	30	404	7	0	862
West Beltway direct on-ramp to Allen Road off-ramp	895	872	97%	83	3	61	7	Α	83	1232	20	1	436	2,965	2,835	96%	57	2	63	15	В	87	2788	44	1	945
Allen Road off-ramp to Allen Road on-ramp	810	810	100%	32	0	64	4	A	115	464	7	0	272	2,655	2,543	96%	32	1	63	13	В	119	1448	23	1	848
Allen Road on-ramp to Calloway Drive off-ramp	2,091 1,615	1,982	95%	58 24	2 0	63 64	10 8	A	173	2007	32 10	0	661 505	4,530	4,351	96% 97%	60 25	4	61 63	24 19	C	179	4423 1563	73 25	5	1450 1202
Calloway Drive off-ramp to Calloway Drive loop on-ramp Calloway Drive loop on-ramp to Calloway Drive direct on-ramp	2,331	1,516 2,161	94% 93%	11	1	61	12	В	197 208	656 409	7	0	720	3,730 4,770	3,606 4,558	96%	12	1	58	26	D	204 216	863	25 15	1	1519
Calloway Drive direct on-ramp to Coffee Road off-ramp	3,305	3,163	96%	48	2	62	16	В	256	2631	42	2	1054	5,880	5,703	97%	50	4	60	31	D	266	4776	79	6	1901
Coffee Road off-ramp to Truxtun Avenue/Mohawk Street off-ramp	2,710	2,604	96%	57	1	63	11	Α	313	2599	41	1	744	5,055	4,869	96%	58	3	62	21	С	324	4857	79	3	1391
Truxtun Avenue off-ramp to Coffee Road on-ramps	950	889	94%	31	0	64	7	Α	345	496	8	0	444	2,355	2,296	97%	32	1	63	18	С	356	1280	20	1	1148
Coffee Road on-ramps to SR 99 SB and Ming Avenue C-D off-ramp	2,600	2,543	98%	109	4	62	13	В	453	4793	77	3	848	4,525	4,438	98%	111	7	61	24	С	467	8361	137	8	1479
SR 99 SB off-ramp (and C-D Ming Avenue) to H Street off-ramp	1,180	1,180	100%	23	0	63	9	Α	476	470	7	0	590	2,355	2,309	98%	23	1	62	19	С	490	917	15	1	1154
H Street off-ramp to SR 99 NB and SB on-ramp	890	865	97%	30	0	64	7	Α	506	459	7	0	432	2,047	1,986	97%	30	1	63	16	В	521	1053	17	1	993
SR 99 NB and SB on-ramp to Chester Avenue on-ramp	3,010	2,906	97%	47	4	60	10	A	553	2270	38	3	830	4,745	4,275	90%	49	6	58	16	В	570	3357	58	7	1221
Chester Avenue on-ramp to Union Avenue off-ramp	3,999	3,836	96%	27	1	61	16	В	580	1762	29	1	1096	5,935	5,410	91%	28	2	60	23	С	598	2487	42	3	1546
Union Avenue off-ramp to Union Avenue loop on-ramp Union Avenue loop on-ramp to Union Avenue direct on-ramp	3,219 3,780	3,079 3,569	96% 94%	17 8	0	63 61	16 19	B C	597 605	894 499	14 8	0	1026 1190	4,910 5,575	4,517 5,086	92% 91%	17 9	1 1	62 59	24 29	C	614 623	1312 712	21 12	1	1506 1695
Union Avenue direct on-ramp to Onion Avenue direct on-ramp Union Avenue direct on-ramp to Cottonwood Road off-ramp	4,195	4,016	96%	31	1	62	22	C	636	2134	35	2	1339	6,150	5,727	93%	32	3	59 59	32	ם	655	3047	51	4	1909
Cottonwood Road off-ramp to Cottonwood Road on-ramp	3,755	3,598	96%	28	1	63	19	C	664	1740	28	1	1199	5,245	4,893	93%	28	1	62	26	D	683	2366	38	2	1631
Cottonwood Road on-ramp to SR 58 east end of the network	4,145	3,981	96%	25	1	62	21	c	689	1737	28	1	1327	5,545	5,172	93%	26	1	61	28	D	709	2256	37	2	1724
Westside Parkway WB Mainline	, -	- /			ı			<u> </u>										l								
SR 58 east end of the network to Cottonwood Road off-ramp	3,790	3,790	100%	26	1	64	20	С	26	1736	27	1	1272	5,030	5,030	100%	26	1 1	63	27	D	26	2302	37	1	1688
Cottonwood Road off-ramp to Cottonwood Road on-ramp	3,531	3,528	100%	18	1	63	19	C	43	1084	17	1	1176	4,725	4,717	100%	18	1	62	25	C	44	1449	23	1	1572
Cottonwood Road on-ramp to Brundage Lane off-ramp	3,941	3,932	100%	39	2	62	21	C	82	2600	42	2	1311	5,255	5,219	99%	39	3	60	29	D	83	3451	57	4	1740
Brundage Lane off-ramp to Brundage Lane on-ramp	3,041	3,040	100%	17	1	63	16	В	99	897	14	0	1013	4,265	4,222	99%	17	1	62	23	С	101	1246	20	1	1407
Brundage Lane on-ramp to Union Avenue on-ramp	3,386	3,354	99%	12	1	62	18	С	111	688	11	1	1118	4,625	4,545	98%	12	1	60	25	С	113	931	15	1	1515
Union Avenue on-ramp to Chester Avenue off-ramp	4,116	4,116	100%	23	1	61	17	В	134	1603	26	1	1177	5,395	5,323	99%	23	1	60	22	С	136	2071	34	2	1521
Chester Avenue off-ramp to H Street on-ramp	3,216	3,216	100%	37	1	63	17	В	171	2112	34	1	1081	4,280	4,249	99%	38	2	62	23	С	174	2767	45	2	1416
H Street on-ramp to SR 99 NB off-ramp	4,092	4,070	99%	25	1	61	17	В	196	1695	28	1	1163	4,940	4,901	99%	25	2	60	20	С	199	2041	34	2	1400
SR 99 NB off-ramp to SR 99 SB off-ramp	3,151	3,120	99%	21	1	62	17 15	В	217	1137	18 12	1	1040	3,820	3,795	99%	21	1	61	21	C	220	1379	23 17	1	1265
SR 99 SB off-ramp to SR 99 NB on-ramp SR 99 NB on-ramp to Coffee Road off-ramp	1,902 3,122	1,854 3,068	97% 98%	23 106	4	63 62	16	В	241 346	763 5591	90	0	927 1023	2,595 4,225	2,581 4,290	99% 102%	24 108	7	62 61	21 23	C	244 352	1063 7819	128	8	1291 1430
Coffee Road off-ramp to Mohawk Street/Truxtun Avenue on-ramp	1,267	1,254	99%	23	1	63	10	A	369	509	8	0	627	2,650	2,650	100%	24	1	62	22	C	375	1091	18	1	1343
Mohawk Street/Truxtun Avenue on-ramp to Coffee Road loop on-ramp	3,866	3,707	96%	53	1	63	15	В	422	3448	55	1	927	6,430	6,355	99%	54	2	62	26	C	429	5911	95	4	1589
Coffee Road loop on-ramp to Coffee Road direct on-ramp	4,136	3,924	95%	13	0	63	16	В	435	855	14	0	981	6,670	6,556	98%	13	1	61	27	D	442	1428	23	1	1639
Coffee Road direct on-ramp to Calloway Drive direct off-ramp	4,551	4,328	95%	49	2	62	17	В	484	3644	59	3	1082	7,220	7,073	98%	50	4	60	29	D	493	5967	99	7	1768
Calloway Drive direct off-ramp to Calloway Drive loop off-ramp	3,666	3,484	95%	11	0	62	14	В	495	660	11	0	871	6,225	6,091	98%	11	1	61	25	С	504	1154	19	1	1523
Calloway Drive loop off-ramp to Calloway Drive on-ramp	2,431	2,285	94%	24	0	63	12	В	519	977	15	0	762	4,875	4,764	98%	25	1	62	26	С	529	2037	33	2	1588
Calloway Drive on-ramp to Allen Road off-ramp	2,916	2,800	96%	58	3	62	14	В	577	2792	45	2	933	5,755	5,589	97%	63	8	58	32	D	591	5615	97	12	1863
Allen Road off-ramp to Allen Road on-ramp	1,101	1,055	96%	26	0	64	5	A	604	494	8	0	352	3,525	3,454	98%	27	1	63	18	С	618	1618	26	1	1151
Allen Road on-ramp to West Beltway off-ramp	1,166	1,117	96%	89	1	64	8	A	693	1758	28	0	372	3,895 1,510	3,792	97% 97%	58	0	61 63	21 12	C	676 699	3747 583	61	0	1264 734
West Beltway off-ramp to West Beltway loop on-ramp West Beltway loop on-ramp to West Beltway direct on-ramp							— NO7	APPLICABLE	_						1,468 1,842		23 6	0	61	15	В	705	184	3	0	73 4 921
West Beltway direct on-ramp to Westside Parkway west end of the network							-1101	ATT LIOABLE						2,280	2,184	96%	6	1	59	19	C	711	210	4	0	1092
Westside Parkway EB Collector-Distributor 1														2,200	2,.0.	0070		<u> </u>				<u> </u>	1 2.0			.002
Begin C-D (Coffee Road loop on-ramp to Coffee Road direct on-ramp)	905	899	009/	12	1 1	1 40	22	С	12	144	١ ،	1 0	899	1 100	1 100	1009/	12	1 1	40	22	C	12	170	1 4 1	0	1106
Coffee Road direct on-ramp to end of C-D network (Westside Parkway EB)	1,650	1,650	99% 100%	12 62	2	49 53	22 21	C	12 73	1499	28	1	828	1,100 2,170	1,100 2,155	100% 99%	12 62	2	49 53	23 20	C	12 74	178 1952	37	1	1078
Westside Parkway EB Collector-Distributor 2	1,030	1,030	10076	02		33	21		13	1433	20		020	2,170	2,100	3370	02		55	20		/4	1932	31	'	1076
•	4.700	4.700	000/	-00	1 4	T 50	00		1 00	700	10		000	0.700	0.000	070/	1 00		50	1 00	0	1 00	1070	00	4	4040
Begin C-D to Mohawk Street off-ramp	1,760	1,726	98%	28	0	53 54	20	СВ	28	708	13	1	690 471	2,700 1,300	2,608 1,250	97%	28	1	53	20	В	28	1070	20 9	1 0	1043
Mohawk Street off-ramp to Mohawk Street on-ramp Mohawk Street on-ramp to Truxtun Avenue off-ramp (end of C-D network)	825 935	825 935	100% 100%	26 32	1	54	11 12	В	54 87	368 432	7	0	471 472	1,300	1,250	96% 96%	26 33	1	54 51	12 12	B B	54 87	489 571	11	0	625 624
	930	333	100 /0	JZ	<u>'</u>	1 31	12		01	432			412	1,300	1,441	30 /0		<u> </u>	J 1	14	ь	01	311		U	024
Westside Parkway EB Collector-Distributor 3	4.400	4.050	050/	-	_ ^		47			144			070	0.470	0.407	070/			F4 1	47	-		470		0 1	4050
Westside Parkway EB C-D on-ramp to SR 99 SB off-ramp SR 99 SB off-ramp to SR 99 SB C-D on-ramp	1,420 615	1,356	95%	6 26	0	52 47	17 8	B A	β 33	111 191	4	0	678 567	2,170	2,107	97%	96	4	51 47	17 8	, B	6 32	173 212	3	0	1053 626
SR 99 SB C-D on-ramp to Sk 99 SB C-D on-ramp SR 99 SB C-D on-ramp to Ming Avenue	1,735	567 1,668	92% 96%	26	1	48	8 18	C	32 60	191 625	13	0	567 834	655 1,860	626 1,805	96% 97%	26 34	7	47	23	C	66	756	5 17	4	903
	1,730	1,000	3070	20		+0	10		1 00	020	13	U	034	1,000	1,000	3170	. 34		44		U	00	100	1 1/		a03
Westside Parkway WB Collector-Distributor	4.400	4.404	0.407	4.4	_ ^	C4	40			204			500	4 740	1.050	070/	44		C4 1	40	_	1 44	444		0 1	000
Truxtun Avenue on-ramp to Mohawk Street loop on-ramp	1,190	1,124	94%	14 15	0	64	13	B	14	281	8	0	562 955	1,710	1,653	97%	14	0	64	13	В	14	414	1 11	0	826 1310
Mohawk Street loop on-ramp to Mohawk Street direct on-ramp Mohawk Street direct on-ramp to Westside Parkway on-ramp (end of C-D network)	2,014 2,599	1,909 2,459	95% 95%	15 17	1	60 59	22 24	C	29 46	472 699	12	1	955 1230	2,735 3,780	2,620 3,672	96% 97%	15 17	1 2	60 59	22 25	C	29 46	648 1043	11 18	1 2	1310 1836
monant choose an out on ramp to visitione i answay on ramp (choose on out of the work)	۷,555	∠,⊤∪∂	JJ /0		<u> </u>	55	∠+		, , ,	000	14	'	1200	3,700	0,012	31 /0			JJ		U	_ - U	10+3	10	_	1000



PARSONS

Table 4-17. AM Peak Hour Measures of Effectiveness for Alternative A—Merging/Diverging Conditions

Merging/biverging conditions			VEAD 201	n		VEAD 2020	,
	ANALYSIS	SPEED	YEAR 201	LOS	SPEED	YEAR 2038 DENSITY	LOS
LOCATION	TYPE	(mph)	(veh/ln/mi)	(HCM 2000)	(mph)	(veh/ln/mi)	(HCM 2000)
State Route 99 NB	1		,				
White Lane loop on-ramp	Merging	48	35.9	E E	39	49.8	E E
White Lane diagonal on-ramp Ming Avenue (from C-D) on-ramp	Merging Merging	56 58	36.5 23.0	C	51 57	45.0 26.9	C
SR 58 on-ramp	Merging	56	28.2	D	53	33.7	D
California Avenue loop on-ramp	Merging	58	29.6	D	56	35.4	E
California Avenue diagonal on-ramp	Merging	58	29.3	D	54	35.4	E
Buck Owens Boulevard/Sillect Avenue on-ramp	Merging	61	17.5	В	59	22.7	С
White Lane off-ramp	Diverging	62	21.8	С	59	32.1	D
Ming Avenue off-ramp	Diverging	60	19.1	В	57	23.1	С
Westside Parkway (C-D) off-ramp SR 58 EB off-ramp	Diverging	61 61	21.1 16.9	C B	59 61	25.2 19.0	C B
California Avenue off-ramp	Diverging Diverging	58	34.9	D	56	40.4	E
Rosedale Highway off-ramp	Diverging	58	24.1	C	55	28.9	D
Buck Owens Boulevard/Sillect Avenue off-ramp	Diverging	61	19.2	В	60	23.4	C
Airport Drive off-ramp	Diverging	57	23.7	С	55	30.3	D
State Route 99 SB							
Airport Drive on-ramp	Merging	61	22.4	С	59	28.2	D
Rosedale Highway loop on-ramp	Merging	54	21.9	С	52	24.8	С
Rosedale Highway diagonal on-ramp California Avenue on-ramp	Merging	57 59	31.6 25.4	D C	55 58	36.4 27.0	E C
SR 99 SB C-D on-ramp (Westside Parkway EB and SR 58 WB and H Street)	Merging Merging	62	12.8	В	61	17.5	В
Ming Avenue on-ramp	Merging	59	26.6	C	56	37.5	E
White Lane loop on-ramp	Merging	61	17.0	В	60	21.7	С
White Lane diagonal on-ramp	Merging	61	18.2	В	60	23.2	С
Rosedale Highway off-ramp	Diverging	61	21.4	С	59	28.0	С
California Avenue off-ramp	Diverging	57	35.0	D	54	40.8	E
SR 58 EB off-ramp	Diverging	60	27.5	С	59	29.1	D
SR 99 SB C-D off-ramp (Ming Avenue) White Lane off-ramp	Diverging Diverging	61 60	20.4 21.0	C	60 55	22.7 29.4	C D
Westside Parkway/State Route 58 EB	Diverging	00	21.0		33	29.4	l D
West Beltway loop on-ramp	Merging				61	15.3	В
West Beltway diagonal on-ramp	Merging	- N	NOT APPLICA	ABLE —	62	18.4	В
Allen Road on-ramp	Merging	60	18.5	В	55	32.6	D
Calloway Drive loop on-ramp	Merging	60	18.9	В	58	29.3	D
Calloway Drive diagonal on-ramp	Merging	57	22.9	С	51	34.3	D
SR 99 NB and SB on-ramp	Merging	63	8.1	A	62	10.5	В
Union Avenue loop on-ramp Union Avenue diagonal on-ramp	Merging Merging	61 61	17.4 20.4	B C	61 60	22.1 25.3	C C
Cottonwood Road on-ramp	Merging	61	17.8	В	61	21.0	C
West Beltway off-ramp	Diverging		NOT APPLICA	L	47	24.4	C
Allen Road off-ramp	Diverging	65	5.1	Α	62	19.1	В
Calloway diagonal off-ramp	Diverging	62	18.6	В	60	30.4	D
Coffee Road off-ramp	Diverging	61	25.3	С	60	35.7	E
Westside Parkway EB C-D off-ramp (Mohawk Street and Truxtun Avenue)	Diverging	61	16.4	В	60	22.6	С
H Street off-ramp Cottonwood Road off-ramp	Diverging	63	8.9	A C	63	14.6	B C
Coffee Road C-D on-ramp to SR 99 SB C-D off-ramp (SR 99 SB and Ming Avenue)	Diverging Weaving	61 63	20.1 11.8	В	61 61	25.0 19.8	В
Chester Avenue on-ramp to Union Avenue off-ramp	Weaving	61	15.0	В	60	19.8	В
Westside Parkway/State Route 58 WB	Trouring	- 01	10.0		_ 00	10.0	
Brundage Lane on-ramp (Cottonwood Road)	Merging	63	18.1	В	60	30.6	D
Brundage Lane on-ramp (Union Avenue)	Merging	62	17.9	В	60	24.7	C
Truxtun Avenue/Mohawk Street (C-D) on-ramp	Merging	63	10.8	В	63	16.4	В
Coffee Road loop on-ramp	Merging	63	12.2	В	62	18.0	В
Calloway Drive on-ramp	Merging	62	10.4	В	61	20.5	С
Allen Road on-ramp	Merging	63	4.5	A	62	14.3	B B
West Beltway loop on-ramp West Beltway diagonal on-ramp	Merging Merging	-1	NOT APPLICA	ABLE —	62 60	10.6 13.3	В
Brundage Lane off-ramp (Cottonwood Road)	Diverging	64	17.0	В	63	27.3	С
Brundage Lane off-ramp (Union Avenue)	Diverging	62	19.9	В	59	32.0	D
SR 99 SB off-ramp	Diverging	62	12.8	В	61	18.7	В
Allen Road off-ramp	Diverging	61	8.2	Α	60	22.1	С
West Beltway off-ramp	Diverging		NOT APPLICA		62	13.5	В
Union Avenue on-ramp to Chester Avenue off-ramp	Weaving	61	16.0	В	60	21.0	С
H Street on-ramp to SR 99 NB off-ramp	Weaving	61	16.0	В	60	20.0	C
SR 99 NB on-ramp to Coffee Road off-ramp	Weaving	63 60	14.0	B B	61 62	20.0	C B
Coffee Road Direct on-ramp to Calloway Drive loop off-ramp	Weaving		17			19	ם

Note: Merge/diverge calculations are based on 1,500 feet from on/off ramp. These calculations also include two right-most mainline lanes within 1,500 feet.



Table 4-18. PM Peak Hour Measures of Effectiveness for Alternative A—Merging/Diverging Conditions

Merging/Diverging Conditions							
	ANALYSIS	SPEED	YEAR 2018 DENSITY	LOS	SPEED	YEAR 2038 DENSITY	LOS
LOCATION	TYPE	(mph)		(HCM 2000)	(mph)	(veh/ln/mi)	(HCM 2000)
State Route 99 NB							
White Lane loop on-ramp	Merging	51	29.7	D	44	41.2	E
White Lane diagonal on-ramp	Merging	58	31.8	D	54	40.2	E
Ming Avenue (from C-D) on-ramp	Merging	60 59	18.2 22.2	B C	59 57	21.1 26.5	C C
SR 58 on-ramp California Avenue loop on-ramp	Merging Merging	58 58	27.4	C	53	34.9	D
California Avenue diagonal on-ramp	Merging	57	28.6	D	46	40.6	E
Buck Owens Boulevard/Sillect Avenue on-ramp	Merging	59	20.0	С	59	25.2	С
White Lane off-ramp	Diverging	63	19.8	В	61	30.3	D
Ming Avenue off-ramp	Diverging	60	17.5	В	50	24.7	С
Westside Parkway (C-D) off-ramp	Diverging	60	18.7	В	50	27.6	С
SR 58 EB off-ramp	Diverging	61	14.8	В	59	18.0	В
California Avenue off-ramp	Diverging	60 59	27.7 23.0	C C	59 52	32.3 29.0	D D
Rosedale Highway off-ramp Buck Owens Boulevard/Sillect Avenue off-ramp	Diverging Diverging	61	23.0 17.9	В	60	29.0	C
Airport Drive off-ramp	Diverging	58	24.4	C	56	30.3	D
State Route 99 SB			ı				ı
Airport Drive on-ramp	Merging	58	28.4	D	51	41.0	Е
Rosedale Highway loop on-ramp	Merging	45	32.6	D	34	46.7	E
Rosedale Highway diagonal on-ramp	Merging	52	43.0	E	32	73.5	F
California Avenue on-ramp	Merging	53	36.7	E	53	35.7	E
SR 99 SB C-D on-ramp (Westside Parkway EB and SR 58 WB and H Street)	Merging	61	16.6	В	52	23.3	С
Ming Avenue on-ramp White Lane loop on-ramp	Merging Merging	57 60	34.9 22.8	D C	29 60	70.4 21.6	F C
White Lane diagonal on-ramp	Merging	60	24.7	C	60	24.6	C
Rosedale Highway off-ramp	Diverging	60	26.1	C	55	33.0	D
California Avenue off-ramp	Diverging	53	45.8	Ē	33	76.2	F
SR 58 EB off-ramp	Diverging	57	37.6	E	56	36.2	E
SR 99 SB C-D off-ramp (Ming Avenue)	Diverging	60	26.5	С	60	27.3	С
White Lane off-ramp	Diverging	56	28.6	D	22	65.8	F
Westside Parkway/State Route 58 EB	NA	ı			1 04	440	
West Beltway loop on-ramp West Beltway diagonal on-ramp	Merging	^	IOT APPLICA	ABLE —	61 62	14.2 17.2	B B
Allen Road on-ramp	Merging Merging	61	13.1	В	56	30.2	D
Calloway Drive loop on-ramp	Merging	60	14.7	В	56	29.8	D
Calloway Drive diagonal on-ramp	Merging	60	17.8	В	55	31.5	D
SR 99 NB and SB on-ramp	Merging	62	9.7	Α	61	13.6	В
Union Avenue loop on-ramp	Merging	60	22.6	С	57	31.6	D
Union Avenue diagonal on-ramp	Merging	60	25.8	С	57	37.2	E
Cottonwood Road on-ramp	Merging	61	23.3	C	60	28.8	D
West Beltway off-ramp	Diverging	64	IOT APPLICA 4.3	ABLE — A	44 62	25.4 17.6	C B
Allen Road off-ramp Calloway Drive off-ramp	Diverging Diverging	63	4.3 13.8	В	61	28.2	D
Coffee Road off-ramp	Diverging	62	21.2	C	60	34.1	D
Westside Parkway EB C-D off-ramp (Mohawk Street and Truxtun Avenue)	Diverging	62	12.3	В	61	20.8	C
H Street off-ramp	Diverging	63	9.3	Α	63	18.4	В
Cottonwood Road off-ramp	Diverging	61	25.3	С	59	37.9	E
Coffee Road C-D on-ramp to SR 99 SB C-D off-ramp (SR 99 SB and Ming Avenue)	Weaving	62	13.4	В	61	23.9	С
Chester Avenue on-ramp to Union Avenue off-ramp	Weaving	61	17.7	В	59	23.7	С
Westside Parkway/State Route 58 WB	NA		04.0		1 00	00.0	
Brundage Lane on-ramp (Cottonwood Road) Brundage Lane on-ramp (Union Avenue)	Merging	62 61	21.8 19.4	C B	60 60	29.8 26.6	D C
Truxtun Avenue/Mohawk Street (C-D) on-ramp	Merging Merging	63	16.9	В	62	27.5	C
Coffee Road loop on-ramp	Merging	62	19.3	В	61	30.2	D
Calloway Drive on-ramp	Merging	61	18.3	В	57	33.8	D
Allen Road on-ramp	Merging	63	6.6	Α	61	19.9	В
West Beltway loop on-ramp	Merging	^	IOT APPLICA	ABLE —	61	15.0	В
West Beltway diagonal on-ramp	Merging				59	18.5	В
Brundage Lane off-ramp (Cottonwood Road)	Diverging	64	18.9	В	63	26.2	С
Brundage Lane off-ramp (Union Avenue)	Diverging	61	24.0	С	60	31.7	D
SR 99 SB off-ramp Allen Road off-ramp	Diverging Diverging	61 61	16.9 14.8	B B	61 59	20.6 25.3	C C
West Beltway off-ramp	Diverging	-			60	25.3	C
Union Avenue on-ramp to Chester Avenue off-ramp	Weaving	61	18.0	В	60	23.0	С
H Street on-ramp to SR 99 NB off-ramp	Weaving	61	18.0	В	60	21.0	Č
SR 99 NB on-ramp to Coffee Road off-ramp	Weaving	62	16.0	В	61	23.0	С
Coffee Road diagonal on-ramp to Calloway Drive loop off-ramp	Weaving	60	22	С	58	31	D

Note: Merge/diverge calculations are based on 1,500 feet from on/off ramp. These calculations also include two right-most mainline lanes within 1,500 feet.



Table 4-19. AM Peak Hour Measures of Effectiveness for Alternative A—On-Ramp and Off-Ramp Performance (1 of 2)

		YE	AR 2018			YEA	R 2038	
FREEWAY RAMPS	DEMAND VOLUME (vph)	CORSIM SERVED VOLUME (vph)	PERCENTAGE SERVED IN CORSIM MODEL (%)	SPEED (mph)	DEMAND VOLUME (vph)	CORSIM SERVED VOLUME (vph)	PERCENTAGE SERVED IN CORSIM MODEL (%)	SPEED (mph)
State Route 99 NB Off-ramps								
White Lane	680	669	98%	61	1,125	1,125	100%	34
Ming Avenue	740	715	97%	53	915	904	99%	52
SR 58 WB	845	841	100%	54	1,125	1,125	100%	54
SR 58 EB	1,025	1,025	100%	54	1,255	1,228	98%	54
California Avenue	970	970	100%	52	1,150	1,134	99%	52
Rosedale Highway	1,880	1,772	94%	54	1,965	1,941	99%	53
Buck Owens Boulevard/Sillect Avenue	570	508	89%	53	795	755	95%	52
Airport Drive	1,505	1,477	98%	52	1,740	1,570	90%	51
State Route 99 NB On-ramps								
White Lane loop	1,630	1,630	100%	41	1,640	1,640	100%	41
White Lane diagonal	705	672	95%	52	695	674	97%	52
Ming Avenue (C-D on)	800	800	100%	51	1,000	1,000	100%	51
SR 58	1,100	1,074	98%	44	1,185	1,164	98%	42
California Avenue loop	860	709	82%	44	1,050	850	81%	44
California Avenue diagonal	370	309	84%	51	480	384	80%	51
Buck Owens Boulevard/Sillect Avenue	400	378	95%	34	720	719	100%	31
State Route 99 SB Off-ramps								
Rosedale Highway	660	660	100%	54	1,375	1,370	100%	54
California Avenue	1,170	1,170	100%	51	1,495	1,457	97%	43
SR 58 EB	905	905	100%	53	955	955	100%	53
SR 99 SB C-D (Ming Avenue)	870	819	94%	53	935	927	99%	53
White Lane	1,420	1,399	99%	54	1,670	1,659	99%	47
State Route 99 SB On-ramps								
Airport Drive	1,280	1,280	100%	47	1,450	1,445	100%	46
Rosedale Highway loop	945	945	100%	37	1,100	1,100	100%	34
Rosedale Highway diagonal	735	725	99%	40	835	828	99%	39
California Avenue	465	463	100%	50	560	560	100%	51
SR 99 SB C-D (SR 58 WB and H Street)	1,585	1,544	97%	51	2,525	2,520	100%	50
Ming Avenue on-ramp to SR 99 NB C-D on-ramp	515	515	100%	51	785	767	98%	50
White Lane loop	175	171	98%	53	260	255	98%	52
White Lane diagonal	370	344	93%	53	445	404	91%	53
State Route 99 NB Collector-Distribu	utor Off-/0	On-ramps						
Ming Avenue C-D on-ramp	1,165	1,165	100%	31	1,375	1,375	100%	30
SR 99 NB C-D on-ramp	845	841	100%	54	1,125	1,125	100%	54
Westside Parkway C-D off-ramp	1,500	1,455	97%	54	1,500	1,500	100%	54
State Route 99 SB Collector-Distribu	utor Off-/0	On-ramps						
SR 58 WB on-ramp	1,025	1,002	98%	39	1,200	1,200	100%	39
	560			44				

Table 4-19. AM Peak Hour Measures of Effectiveness for Alternative A—On-Ramp and Off-Ramp Performance (2 of 2)

Off-Ramp Perio	Jilliand	. ,	AR 2018			VEA	IR 2038	
	DEMAND	CORSIM	PERCENTAGE		DEMAND	CORSIM	PERCENTAGE	
FREEWAY RAMPS	VOLUME (vph)	SERVED VOLUME (vph)	SERVED IN CORSIM MODEL (%)	SPEED (mph)	VOLUME (vph)	SERVED VOLUME (vph)	SERVED IN CORSIM MODEL (%)	SPEED (mph)
Westside Parkway/State Route 58 E	B Off-ran	ıps						
West Beltway		— NOT A	PPLICABLE —		745	743	100%	50
Allen Road	60	60	100%	55	380	374	98%	53
Calloway Drive	525	525	100%	54	960	954	99%	53
Coffee Road	640	640	100%	53	1,025	1,006	98%	53
Mohawk Street/Truxtun Avenue C-D	2,390	2,371	99%	53	3,090	3,031	98%	52
SR 99 SB/Ming Avenue C-D	1,130	1,107	98%	54	1,880	1,852	99%	53
H Street	260	256	98%	54	282	268	95%	54
Union Avenue Cottonwood Road	715 415	709 415	99% 100%	54 54	1,035 630	988 605	95% 96%	54 53
			100%	54	030	605	90%	- 55
Westside Parkway/State Route 58 E	b Un-ran	ips			4 505	4.400	000/	40
West Beltway loop		— NOT A	PPLICABLE —		1,525	1,406	92%	48
West Beltway diagonal Allen Road	1 710	1 707	100%	42	290	282	97% 100%	54 44
Calloway Drive loop	1,710 740	1,707 737	100%	42	2,000 940	1,990 933	99%	47
Calloway Drive loop Calloway Drive diagonal	1,060	1,057	100%	51	1,150	1,130	98%	50
Coffee Road (C-D)	1,400	1,379	99%	53	1,130	1,874	100%	52
SR 99 NB and SR 99 SB	1,565	1,565	100%	52	1,887	1,857	98%	52
Chester Avenue	750	734	98%	51	1,025	1,008	98%	51
Union Avenue loop	320	287	90%	53	340	325	96%	52
Union Avenue diagonal	345	345	100%	52	325	325	100%	52
Cottonwood Road	300	300	100%	52	310	309	100%	52
Westside Parkway/State Route 58 W	/B Off-rar	nps		ı		I.	•	
Cottonwood Road	255	253	99%	54	235	215	91%	54
Brundage Lane	670	667	100%	52	1,180	1,180	100%	19
Chester Avenue	975	919	94%	54	1,030	1,009	98%	54
SR 99 NB	1,100	1,078	98%	54	1,185	1,167	98%	54
SR 99 SB	1,025	1,001	98%	52	1,200	1,200	100%	52
Coffee Road	1,670	1,670	100%	53	1,950	1,910	98%	53
Calloway Drive diagonal	650	611	94%	54	650	649	100%	54
Calloway Drive loop	950	948	100%	53	910	884	97%	53
Allen Road	1,085	988	91%	54	1,150	1,112	97%	52
West Beltway			PPLICABLE —		1,500	1,403	94%	54
Westside Parkway/State Route 58 W	/B On-rar	nps						
Cottonwood Road	260	244	94%	51	315	303	96%	50
Brundage Lane	385	343	89%	49	365	350	96%	49
Union Avenue	565	565	100%	51	565	532	94%	51
H Street	695	659	95%	50	635	635	100%	50
SR 99 NB/Ming Avenue	1,210	1,210	100%	44	1,500	1,500	100%	41
Truxtun Avenue/Mohawk Street C-D	1,620	1,551	96%	63	2,160	2,127	98%	62
Coffee Road loop	130	125	95%	51	150	150	100%	51
Coffee Road diagonal	255	242	82%	54	300	282	94%	54
Calloway Drive	320	320 120	100%	52 50	500 350	500	100%	53 50
Allen Road West Beltway loop	120	120	100%	50	350 320	338 299	97% 93%	50 49
West Beltway diagonal		— NOT A	PPLICABLE —		305	299	97%	54
Westside Parkway EB Collector-Dis	tributor 4	Off-/On row	ne		1 303	230	37 70	J-4
•			1	44	045	045	1000/	44
Coffee Road loop on-ramp Coffee Road diagonal on-ramp	830 570	830 542	100% 95%	41 53	915 960	915 949	100% 99%	41 52
				J 33	300	348	3370	J2
Westside Parkway EB Collector-Dis					0.00-	0.00:	2001	
Westside Parkway C-D on-ramp	2,390	2,371	99%	53	3,090	3,031	98%	52
Mohawk Street off-ramp	1,205	1,160	96%	49	1,510	1,497	99%	49
Mohawk Street on-ramp Truxtun Avenue off-ramp	1,185	— 1,185	— 100%	— 45	1,580	— 1,529	97%	— 45
Westside Parkway EB Collector-Dis		· · · · · · · · · · · · · · · · · · ·	l	1 40	1,360	1,329	3170	40
			•	F0	4.000	4.050	000/	F0
Westside Parkway EB C-D on-ramp	1,130	1,107	98%	53	1,880	1,852	99%	53
SR 99 SB C-D off-ramp	560 870	549 816	98%	48 47	1,325	1,305	98%	49 47
SR 99 SB C-D on-ramp			94%	4/	935	926	99%	4/
Westside Parkway WB Collector-Dis		•				ı	1	
Truxtun Avenue on-ramp	780	751	96%	63	800	780	98%	63
Mohawk Street loop on-ramp	445	444	100%	47	505	502	99%	47
Mohawk Street direct on-ramp	395	365	92%	51	855	840	98%	51



Table 4-20. PM Peak Hour Measures of Effectiveness for Alternative A—On-Ramp and Off-Ramp Performance (1 of 2)

On-Kamp r end			AR 2018			YEA	IR 2038	
FREEWAY RAMPS	DEMAND VOLUME (vph)	CORSIM SERVED VOLUME (vph)	PERCENTAGE SERVED IN CORSIM MODEL (%)	SPEED (mph)	DEMAND VOLUME (vph)	CORSIM SERVED VOLUME (vph)	PERCENTAGE SERVED IN CORSIM MODEL (%)	SPEED (mph)
State Route 99 NB Off-ramps								
White Lane	690	676	98%	62	1,080	1,053	98%	60
Ming Avenue	835	835	100%	52	850	802	94%	52
SR 58 WB	830	830	100%	54	1,130	1,130	100%	54
SR 58 EB	1,155	1,105	96%	54	1,665	1,609	97%	53
California Avenue	630	626	99%	52	710	709	100%	51
Rosedale Highway	1,760	1,729	98%	53	1,800	1,673	93%	53
Buck Owens Boulevard/Sillect Avenue	365	365	100%	52	400	365	91%	53
Airport Drive	1,365	1,278	94%	52	1,365	1,275	93%	52
State Route 99 NB On-ramps								
White Lane loop	1,485	1,485	100%	44	1,655	1,476	89%	44
White Lane diagonal	605	592	98%	53	685	665	97%	52
Ming Avenue (C-D on)	705	705	100%	52	850	846	100%	51
SR 58	940	940	100%	45	1,120	1,101	98%	43
California Avenue loop	905	728	80%	45	1,375	908	66%	44
California Avenue diagonal	660	499	76%	51	900	666	74%	50
Buck Owens Boulevard/Sillect Avenue	715	715	100%	31	790	783	99%	31
State Route 99 SB Off-ramps								
Rosedale Highway	710	710	100%	54	1,310	1,292	99%	53
California Avenue	1,395	1,373	98%	44	1,655	1,655	100%	30
SR 58 EB	1,375	1,346	98%	52	1,440	1,375	95%	52
SR 99 SB C-D (Ming Avenue)	1,120	1,100	98%	53	1,205	1,199	100%	52
White Lane	2,015	2,009	100%	52	2,070	1,855	90%	9
State Route 99 SB On-ramps								
Airport Drive	1,765	1,765	100%	46	2,070	2,070	100%	40
Rosedale Highway loop	1,335	1,335	100%	34	1,375	1,350	98%	29
Rosedale Highway diagonal	1,105	1,068	97%	37	1,340	1,225	91%	32
California Avenue	920	838	91%	51	940	825	88%	50
SR 99 SB C-D (SR 58 WB and H Street)	2,055	2,041	99%	51	2,740	2,740	100%	47
Ming Avenue on-ramp to SR 99 NB C-D on-ramp	650	650	100%	51	845	845	100%	28
White Lane loop	510	504	99%	52	550	542	99%	51
White Lane diagonal	480	287	60%	53	645	555	86%	53
State Route 99 NB Collector-Distribu	utor Off-/0	On-ramps						
Ming Avenue C-D on-ramp	1,095	1,095	100%	33	1,350	1,337	99%	32
SR 99 NB C-D on-ramp	830	830	100%	54	1,130	1,130	100%	54
Westside Parkway C-D off-ramp	1,220	1,220	100%	54	1,630	1,630	100%	53
State Route 99 SB Collector-Distribu	utor Off-/0	On-ramps						
SR 58 WB on-ramp	1,225	1,195	98%	39	1,225	1,213	99%	39
Westside Parkway C-D on-ramp	1,515	1,457	96%	44	1,515	1,465	97%	41

Table 4-20. PM Peak Hour Measures of Effectiveness for Alternative A—On-Ramp and Off-Ramp Performance (2 of 2)

Off-Ramp Perio	Jilliani		AR 2018			VEA	.R 2038	
	DEMAND VOLUME	CORSIM SERVED	PERCENTAGE SERVED IN CORSIM	SPEED	DEMAND VOLUME	CORSIM SERVED	PERCENTAGE SERVED IN CORSIM	SPEED
FREEWAY RAMPS	(vph)	VOLUME (vph)	MODEL (%)	(mph)	(vph)	VOLUME (vph)	MODEL (%)	(mph)
Westside Parkway/State Route 58 E West Beltway	b On-ran	•	PPLICABLE —		070	964	000/	40
Allen Road	85	— NOT A	95%	54	870 310	861 294	99% 95%	49 54
Calloway Drive	475	469	99%	54	800	752	94%	53
Coffee Road	595	547	92%	53	825	802	97%	53
Mohawk Street/Truxtun Avenue C-D	1,760	1,728	98%	54	2,700	2,611	97%	53
SR 99 SB/Ming Avenue C-D	1,420	1,354	95%	53	2,170	2,112	97%	52
H Street	290	290	100%	54	308	308	100%	54
Union Avenue	780	757	97%	54	1,025	994	97%	54
Cottonwood Road Westside Parkway/State Route 58 E	440	424	96%	54	905	845	93%	53
West Beltway loop	b Oli-Iali	ips			1,340	1,320	99%	48
West Beltway loop West Beltway diagonal		— NOT A	PPLICABLE —		275	270	98%	54
Allen Road	1,280	1,170	91%	47	1,875	1,833	98%	46
Calloway Drive loop	715	690	97%	47	1,040	1,040	100%	47
Calloway Drive diagonal	975	960	98%	52	1,110	1,055	95%	52
Coffee Road (C-D)	1,650	1,650	100%	53	2,170	2,155	99%	52
SR 99 NB and SR 99 SB	2,120	2,041	96%	52	2,698	2,553	95%	51
Chester Avenue	990	952	96%	50	1,190	1,162	98%	50
Union Avenue loop	560	536	96%	53	665	655	98%	52
Union Avenue diagonal	415	415	100%	52	575	572	99%	52
Cottonwood Road	390	390	100%	52	300	300	100%	52
Westside Parkway/State Route 58 W	B Off-rar	nps						
Cottonwood Road	260	260	100%	54	305	305	100%	54
Brundage Lane	900	894	99%	52	990	990	100%	44
Chester Avenue	900	883	98%	51	1,115	1,074	96%	52
SR 99 NB	940	940	100%	54	1,120	1,103	98%	54
SR 99 SB	1,250	1,250	100%	52	1,225	1,213	99%	52
Coffee Road	1,855	1,812	98%	53	1,575	1,575	100%	54
Calloway Drive diagonal	885	841	95%	53	995	975	98%	53
Calloway Drive loop	1,235	1,198	97%	53	1,350	1,325	98%	52
Allen Road	1,815	1,749	96%	54	2,230	2,120	95%	51
West Beltway	(D. O.,		PPLICABLE —		2,385	2,308	97%	46
Westside Parkway/State Route 58 W		•			1		1	
Cottonwood Road	410	408	100%	50	530	505	95%	50
Brundage Lane	345	336	97%	49	360	351	98%	49
Union Avenue H Street	730	730	100%	51	770	759 647	99%	51 50
SR 99 NB/Ming Avenue	875 1,220	816 1,217	93% 100%	50 44	660 1,630	1,630	98% 100%	37
Truxtun Avenue/Mohawk Street C-D	2,599	2,457	95%	62	3,780	3,672	97%	61
Coffee Road loop	270	231	86%	51	240	222	93%	51
Coffee Road diagonal	415	403	97%	54	550	513	93%	54
Calloway Drive	482	482	100%	52	880	857	97%	52
Allen Road	65	65	100%	51	370	356	96%	50
West Beltway loop		NOT A	DDL ICADI E		425	425	100%	49
West Beltway diagonal		=NOT A	PPLICABLE —		345	331	96%	53
Westside Parkway EB Collector-Dist	tributor 1	Off-/On-ram	nps					
Coffee Road loop on-ramp	1,100	1,100	100%	41	1,100	1,100	100%	41
Coffee Road diagonal on-ramp	1,070	1,070	100%	53	1,070	1,053	98%	52
Westside Parkway EB Collector-Dist	tributor 2	Off-/On-ram	nps					
Westside Parkway C-D on-ramp	2,700	2,610	97%	53	2,700	2,611	97%	53
Mohawk Street off-ramp	1,400	1,377	98%	49	1,400	1,355	97%	49
Mohawk Street on-ramp	_	_	_	_	_	_	_	_
Truxtun Avenue off-ramp	1,300	1,211	93%	45	1,300	1,247	96%	45
Westside Parkway EB Collector-Dis	tributor 3	Off-/On-ram	nps					
Westside Parkway EB C-D on-ramp	2,170	2,094	96%	53	2,170	2,112	97%	52
SR 99 SB C-D off-ramp	1,515	1,462	97%	48	1,515	1,474	97%	46
SR 99 SB C-D on-ramp	1,205	1,116	93%	47	1,205	1,196	99%	46
Westside Parkway WB Collector-Dis	tributor (Off-/On-ramp	os					
Truxtun Avenue on-ramp	1,190	1,123	94%	63	1,710	1,653	97%	63
Mohawk Street loop on-ramp	824	808	98%	47	1,025	1,000	98%	47
Mohawk Street direct on-ramp	585	526	90%	51	1,045	1,014	97%	50



Table 4-21. Ramp Termini and Intersection Level of Service for Alternative A (1 of 3)

Table 4-21. Ramp Termini and intersection				R 2018			YEAF	2038	
			AM PEAK		PM PEAK	I	AM PEAK		PM PEAK
	0.00.00		AVG DELAY		AVG DELAY		AVG DELAY		AVG DELAY
INTERSECTION	SIGNAL CONTROL	LOS	(sec/veh)	LOS	(sec/veh)	LOS	(sec/veh)	LOS	(sec/veh)
I. I-5 SB ramps/Stockdale Highway	NB/SB two-way stop	Α	4.7	Α	8.8	A	5.3	F	66.3
	Imp-Signal*			I/A		В	11.4	В	11.2
2. I-5 NB ramps/Stockdale Highway	NB/SB two-way stop	Α	6.4	Α	8.4	Α	7.2	В	16.8
3. SR 43 (Enos Lane)/Rosedale Highway	4-way stop	В	14.4	D	32.0	D	28.5	F	81.9
	Imp-Signal		N	I/A		С	26.8	С	32.6
4. SR 43 (Enos Lane)/Stockdale Highway	4-way stop	F	52.9	F	86.3	F	>150	F	>150
4. ON 40 (Ellos Eallo)/Otockdale Highway	Imp-Signal	С	20.5	С	23.6	В	15.5	С	22.1
5. SR 43 (Enos Lane)/I-5 NB ramps	EB/WB two-way stop	Α	5.5	В	11.1	Α	4.3	В	10.1
6. SR 43 (Enos Lane)/I-5 SB ramps	EB/WB two-way stop	Α	8.1	С	15.8	Α	4.1	С	21.2
7. Stockdale Highway/Nord Road	Signal	D	39.5	С	29.6	С	34.5	С	32.4
Stockdale Highway/Wegis Avenue	NB/SB two-way stop	Α	7.4	В	10.5	F	>150	F	>150
8. Stockdale Highway/Wegis Avenue	Imp-Signal*		Ν	l/A		С	31.5	С	30.7
9. Stockdale Highway/Heath Road	Stop (existing) Signal (future)	В	14.9	В	18.0	С	23.8	С	26.7
10. Stockdale Highway/Westside Parkway	Signal	Α	7.4	В	18.0	Α	9.0	Α	8.3
11. West Beltway/Westside Parkway WB ramp	Signal		Does not e	exist in	2018	Α	8.5	В	17.2
12. West Beltway/Westside Parkway EB ramp	Signal		Does not e	exist in	2018	Α	8.3	В	15.3
13. Allen Road/Rosedale Highway	Signal*	С	34.4	D	41.4	D	36.1	D	48.0
14. Allen Road/Brimhall Road	Signal	С	24.9	С	31.9	С	23.5	С	34.4
15. Allen Road/Westside Parkway WB ramps	Signal	В	14.0	С	29.4	В	17.5	С	29.3
16. Allen Road/Westside Parkway EB ramps	Signal	В	19.9	Α	8.2	В	10.9	В	11.5
17. Allen Road/San Juan Avenue	Signal*	С	23.3	С	20.6	С	22.7	С	26.5
18. Allen Road/Stockdale Highway	Signal	С	24.4	С	26.0	С	31.1	С	33.0
19. Calloway Drive/Rosedale Highway	Signal*	D	47.3	D	39.7	D	53.2	Е	58.6
20. Calloway Drive/Brimhall Road	Signal	С	26.4	С	30.5	С	29.8	С	35.0
21. Calloway Drive/Westside Parkway WB ramps	Signal	В	18.3	В	15.8	В	19.2	С	28.3
22. Calloway Drive/Westside Parkway EB ramps	Signal	В	15.6	Α	7.4	В	11.2	Α	7.5
23. Calloway Drive/Stockdale Highway	Signal	D	47.6	С	31.4	D	42.9	D	37.0
24. Coffee Road/Rosedale Highway*	Signal*	D	46.1	F	92.7	Е	64.0	F	83.7



Table 4-21. Ramp Termini and Intersection Level of Service for Alternative A (2 of 3)

Table 4-21. Namp Termin and Intersection				R 2018			YEAR	2038	
			AM PEAK	F	PM PEAK	1	AM PEAK		PM PEAK
INTERSECTION	SIGNAL CONTROL	LOS	AVG DELAY (sec/veh)	LOS	AVG DELAY (sec/veh)	LOS	AVG DELAY (sec/veh)	LOS	AVG DELAY (sec/veh)
25. Coffee Road/Brimhall Road	Signal	С	22.4	С	26.4	С	32.0	С	31.6
26. Coffee Road/Westside Parkway WB ramps	Unsignalized		N	l/A			N.	/A	
27. Coffee Road/Westside Parkway EB ramps	Signal	В	16.9	Α	7.2	В	18.2	В	11.0
28. Coffee Road/Truxtun Avenue	Signal	В	13.3	В	15.3	В	12.4	В	18.5
29. Coffee Road/Stockdale Highway	Signal*	D	37.7	D	52.7	Е	64.7	F	92.6
30. Mohawk Street/Rosedale Highway	Stop (existing)* Signal (future)*	D	50.4	F	87.0	Ш	64.5	F	106.9
31. Mohawk Street/WSP WB ramps	Signal		N	I/A			N,	/A	
32. Mohawk Street/WSP EB ramps	Signal	С	21.5	В	16.4	В	11.8	В	13.8
33. Mohawk Street/Truxtun Avenue	Signal	С	26.8	С	27.1	С	26.4	С	25.5
34. Mohawk Street/California Avenue	Signal*	С	30.8	Е	62.3	D	36.1	Е	65.4
35. Stockdale Highway/California Avenue	Signal*	D	39.7	Е	58.5	D	42.4	Е	74.2
36. Airport Drive/State Road-SR 204 off-ramp	Signal	С	23.6	С	22.3	С	22.2	С	22.6
37. Airport Drive/SR 99 NB ramp	Signal	Α	9.3	В	12.0	Α	9.5	В	11.6
38. Buck Owens Boulevard/Rio Mirada Drive	Signal	С	23.4	С	23.4	С	21.7	С	25.0
39. SR 99 NB ramps/Buck Owens Boulevard	Signal	С	34.4	D	43.7	D	41.7	D	43.4
40. Rosedale Highway/Camino Del Rio Court	Signal	С	27.1	D	38.5	С	30.4	D	49.4
41. Rosedale Highway/SR 99 SB ramps	Signal	В	17.6	В	16.8	С	21.2	С	21.8
42. Rosedale Highway/SR99 NB ramps	Signal	С	25.9	С	28.9	С	33.2	D	42.9
43. 24th Street/Oak Street	Signal	С	32.1	С	32.2	С	25.9	D	38.4
44. Truxtun Avenue/Empire Drive	Signal*	С	29.9	С	27.0	С	29.4	С	28.1
45. Truxtun Avenue/Oak Street	Signal	С	30.3	D	44.3	D	52.0	D	46.3
46. California Avenue/Chester Lane	Signal	С	20.8	D	53.6	С	20.2	F	130.7
47. California Avenue/SR 99 SB ramps	Signal	D	43.5	Е	65.9	Е	57.8	F	90.3
48. California Avenue/SR 99 NB ramps	Signal	С	25.6	С	25.8	С	32.5	С	25.9
49. California Avenue/Oak Street	Signal	С	27.8	D	41.0	C	29.7	Е	58.6
50. Stockdale Highway/Stine Road	Signal*	С	25.4	D	52.5	С	30.7	F	89.5
51. Stockdale Highway/Real Road	Signal	D	39.9	D	40.6	D	36.0	D	53.1
52. Stockdale Highway/SR 99 SB ramp	Signal			Interse	ction does not	exist i	n Alternative	4	



Table 4-21. Ramp Termini and Intersection Level of Service for Alternative A (3 of 3)

Table 4-21. Ramp Termini and intersection E			,	R 2018			YEAR	2038	
			AM PEAK		PM PEAK	F	AM PEAK		PM PEAK
INTERSECTION	SIGNAL CONTROL	LOS	AVG DELAY (sec/veh)	LOS	AVG DELAY (sec/veh)	LOS	AVG DELAY (sec/veh)	LOS	AVG DELAY (sec/veh)
53. Brundage Lane/Oak Street	Signal	С	23.6	С	28.1	С	25.9	С	30.4
54. Real Road/SR 58	Signal		ı	Intersed	ction does not	exist ii	n Alternative A	4	
55. Wible Road/SR 99 NB ramps	Signal		ı	Intersed	ction does not	exist ii	n Alternative A	4	
56. Ming Avenue/New Stine Road	Signal*	С	34.4	D	37.0	D	42.1	D	44.4
57. Ming Avenue/Real Road	Signal	С	30.4	С	30.2	С	30.6	С	29.9
58. Ming Avenue/SR 99 SB ramps	Signal	Α	7.6	Α	2.8	Α	8.8	В	15.7
59. Ming Avenue/Wible Road	Signal	В	15.6	С	25.9	С	25.8	С	28.2
60. Ming Avenue/SR 99 NB ramps	Signal	С	24.4	C	33.1	C	25.1	С	33.6
61. Ming Avenue/Castro Lane	Signal	С	30.0	С	29.9	С	30.1	D	39.8
62. White Lane/Wible Road	Signal*	Е	64.8	Е	71.9	F	83.6	F	138.7
63. White Lane/SR 99 SB ramps	Signal*	С	20.6	F	90.0	С	29.0	F	88.8
64. White Lane/SR 99 NB ramps	Signal*	В	10.7	В	12.3	C	23.9	С	20.1
65. White Lane/Hughes Lane	Signal*	D	36.6	D	45.2	D	42.6	D	48.3
66. H Street/Brundage Lane	Signal	С	24.2	D	40.0	O	26.2	D	53.8
67. H Street/SR 58 WB ramp	Signal	С	23.9	D	45.4	В	15.7	D	35.1
68. H Street/SR 58 EB ramp	Signal	С	24.6	С	21.0	С	23.2	С	25.7
69. H Street/Ming Avenue	Signal*	С	30.0	D	40.5	D	35.0	D	42.1
70. Chester Avenue/Brundage Lane	Signal	С	31.1	D	40.5	С	26.2	D	53.8
71. Chester Avenue/SR 58 WB ramp	Signal	В	19.4	С	24.9	O	23.9	С	27.2
72. Chester Avenue/SR 58 EB ramp	Signal	С	25.7	В	17.2	С	29.9	С	31.1
73. Chester Avenue/Ming Avenue	Signal*	D	36.0	D	42.8	D	36.4	D	47.6
74. Union Avenue/Brundage Lane	Signal	С	32.7	С	28.5	D	42.7	D	47.8
75. Brundage Lane/SR 58 WB ramps	Signal	С	32.9	С	22.4	O	24.8	D	41.8
76. Union Avenue/SR 58 EB ramps	Signal	В	13.9	В	12.5	В	19.9	В	18.9
77. Cottonwood Road-MLK/Brundage Lane	Signal	С	23.1	С	25.9	С	25.2	С	27.6
78. Cottonwood Road-Brundage Lane/SR 58 WB ramps	Signal	С	23.1	С	27.3	С	23.3	С	33.1
79. Cottonwood Road/SR 58 EB ramps	Signal	В	11.2	В	19.2	В	11.0	В	18.8

*LOS summary based on SYNCHRO 6

Source: Parsons



4.5 Traffic Performance of Build Alternative B

Alternative B proposes to connect the east end of the Westside Parkway to State Route 58 east near Cottonwood Road by means of a new freeway. This proposed alternative would begin at the Mohawk Street interchange and turn in a southeasterly direction. As illustrated on Figure 4-10, it would span the Kern River, Truxtun Avenue, Carrier Canal, California Avenue and Stockdale Highway before joining the existing State Route 58 east at its existing terminus near the State Route 58/State Route 99 interchange. Improvements on State Route 58 would continue to extend through the H Street and Chester Avenue interchange to Cottonwood Road.

State Route 58 would maintain its existing connections to State Route 99 by means of freeway to freeway connectors. The existing westbound State Route 58 to northbound State Route 99 connector, southbound State Route 99 to eastbound State Route 58 connector, and northbound State Route 99 to eastbound State Route 58 would be preserved with modifications. New branch connectors would be constructed for the eastbound State Route 58 to southbound State Route 99, and northbound State Route 99 to westbound State Route 58 movements.

Auxiliary lanes would be provided on State Route 99 to accommodate the additional traffic from these branch connectors. The limits of improvements on State Route 99 would extend from the interchange at State Route 58 to the Wilson Road overcrossing. All ramps in this vicinity would have to be realigned to provide for the additional lanes. The Wible Road on- and off-ramps south of the existing State Route 99/State Route 58 interchange would be removed to accommodate the northbound State Route 99 on ramp from Ming Avenue. The Stockdale Avenue off-ramp from southbound State Route 99 to the eastbound State Route 58 connector would be removed. Local access from Real Road to State Route 58 and to southbound State Route 99 would also be removed.

The traffic lane configuration of the freeway segments along State Route 58, the Westside Parkway, and State Route 99 are illustrated on Figure 4-11. Also depicted are the relative locations of freeway on- and off-ramps along with traffic demand volumes in the opening year (2018) and the design year.

The alternative B new freeway would bisect existing business parks and residential neighborhoods. The following roads are proposed to cross over or under the proposed freeway alignment:

- Truxtun Avenue
- Commerce Drive
- California Avenue
- Marella Way

- La Mirada Drive
- Stockdale Highway and Stine Road
- South Real Road

The potential closure of Monclair Street, Woodlake Drive, Kensington Avenue, Hillsborough Drive, Kentfield Drive, Joseph Drive, Dunlap Street, Ford Avenue, and Williamson Way would modify existing circulation.

Figure 3-12, shown previously, illustrated the difference between 2038 alternative B and no-build daily traffic volumes, as assigned by the travel forecast model. The bandwidths illustrated in red depicted roadways which receive additional volumes of traffic as a result of constructing the freeway-to-freeway Centennial Project connector. Bandwidths illustrated in

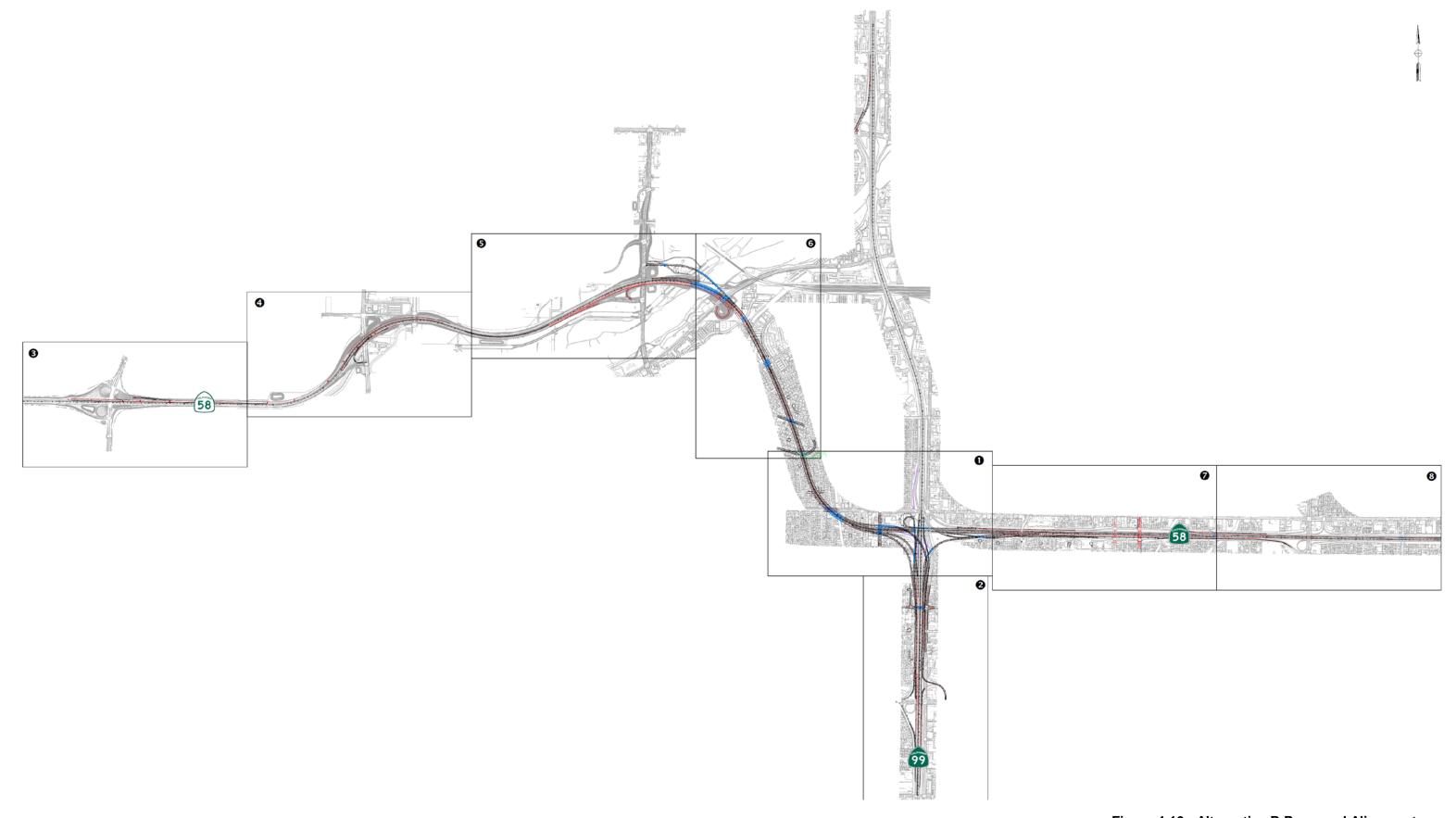


Figure 4-10: Alternative B Proposed Alignment

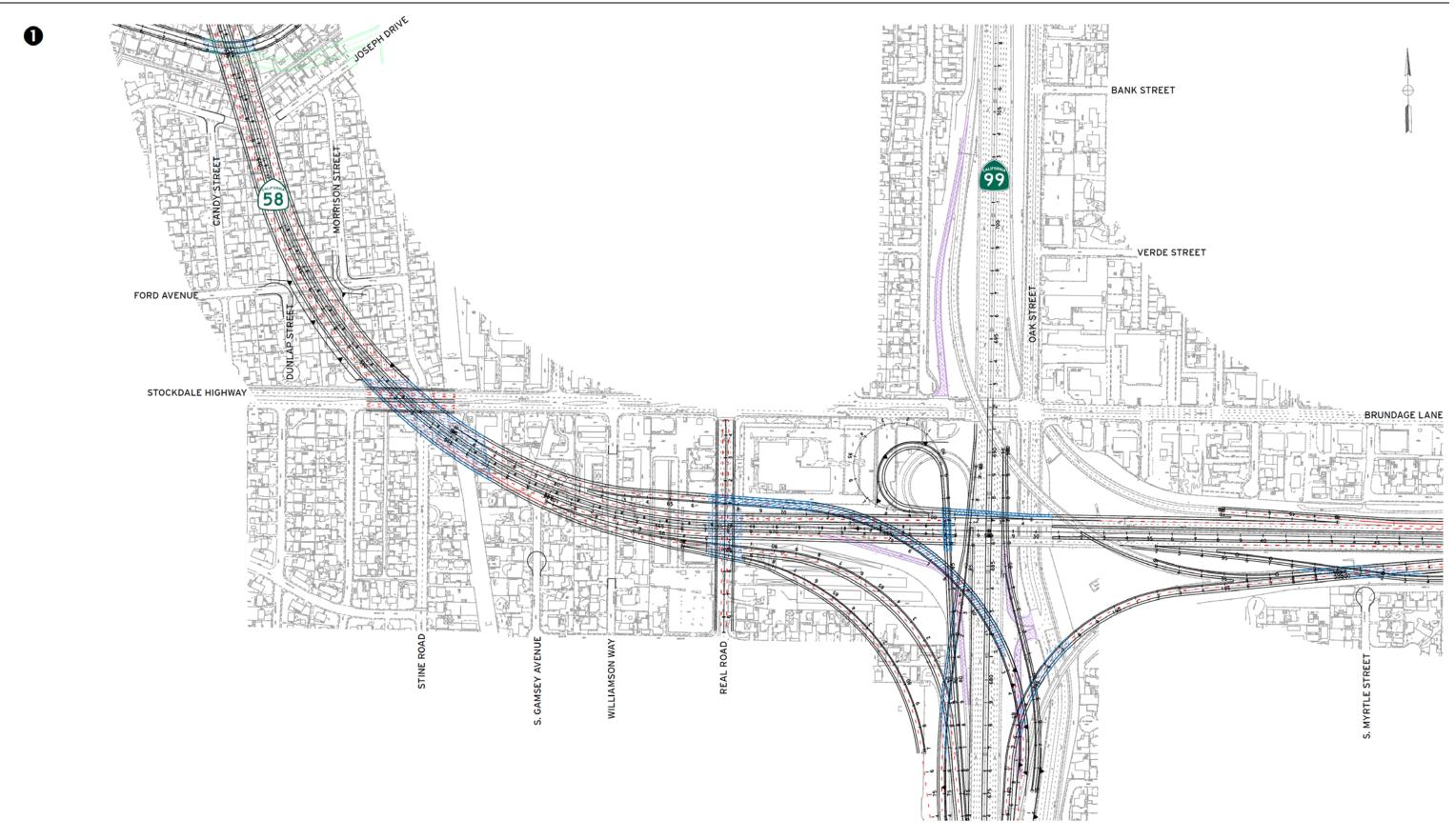


Figure 4-10: Alternative B Proposed Alignment Sheet 1 of 8





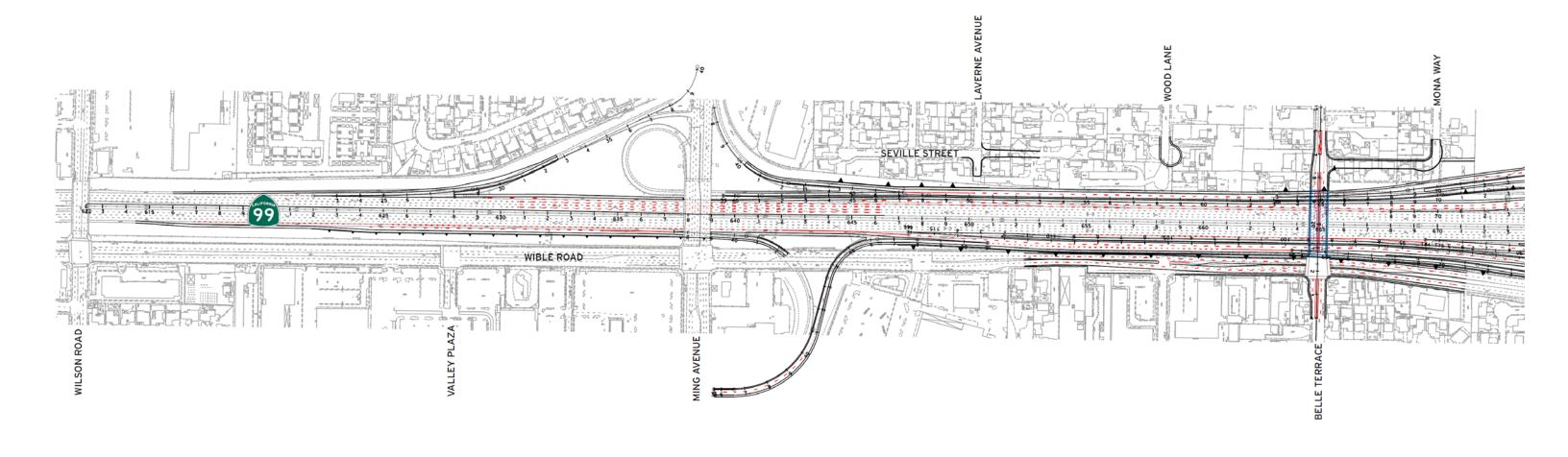


Figure 4-10: Alternative B Proposed Alignment Sheet 2 of 8

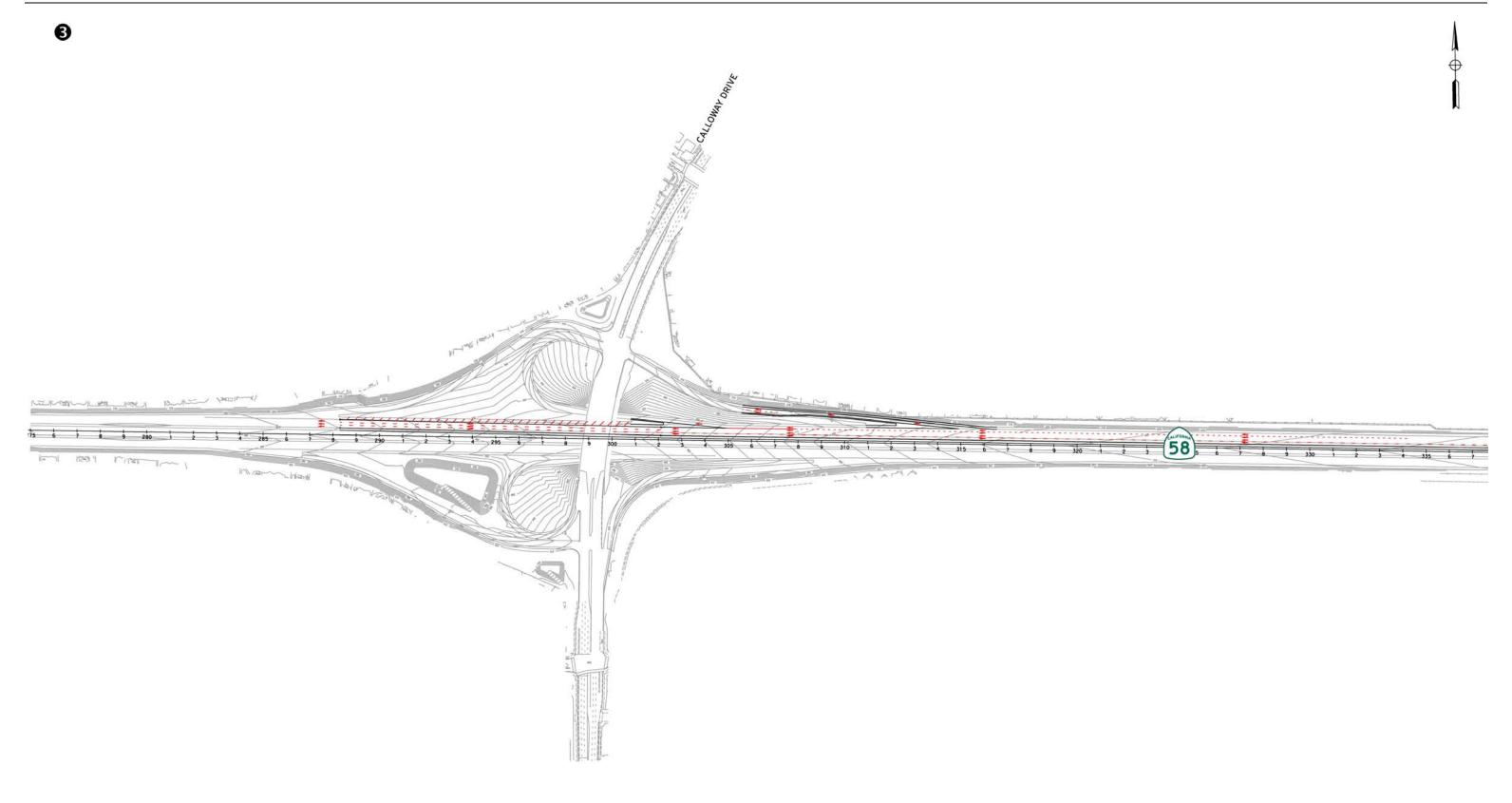


Figure 4-10: Alternative B Proposed Alignment Sheet 3 of 8

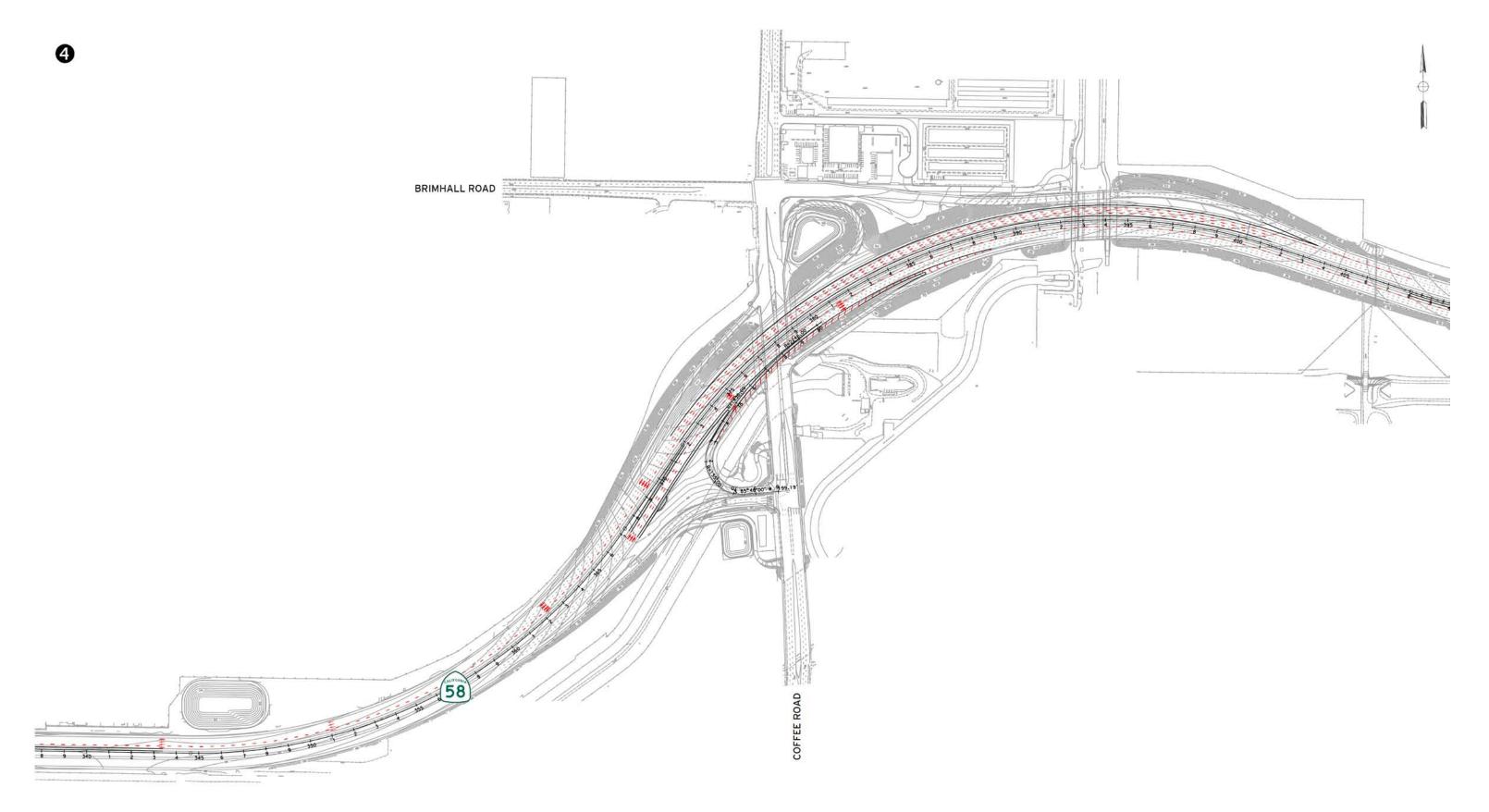
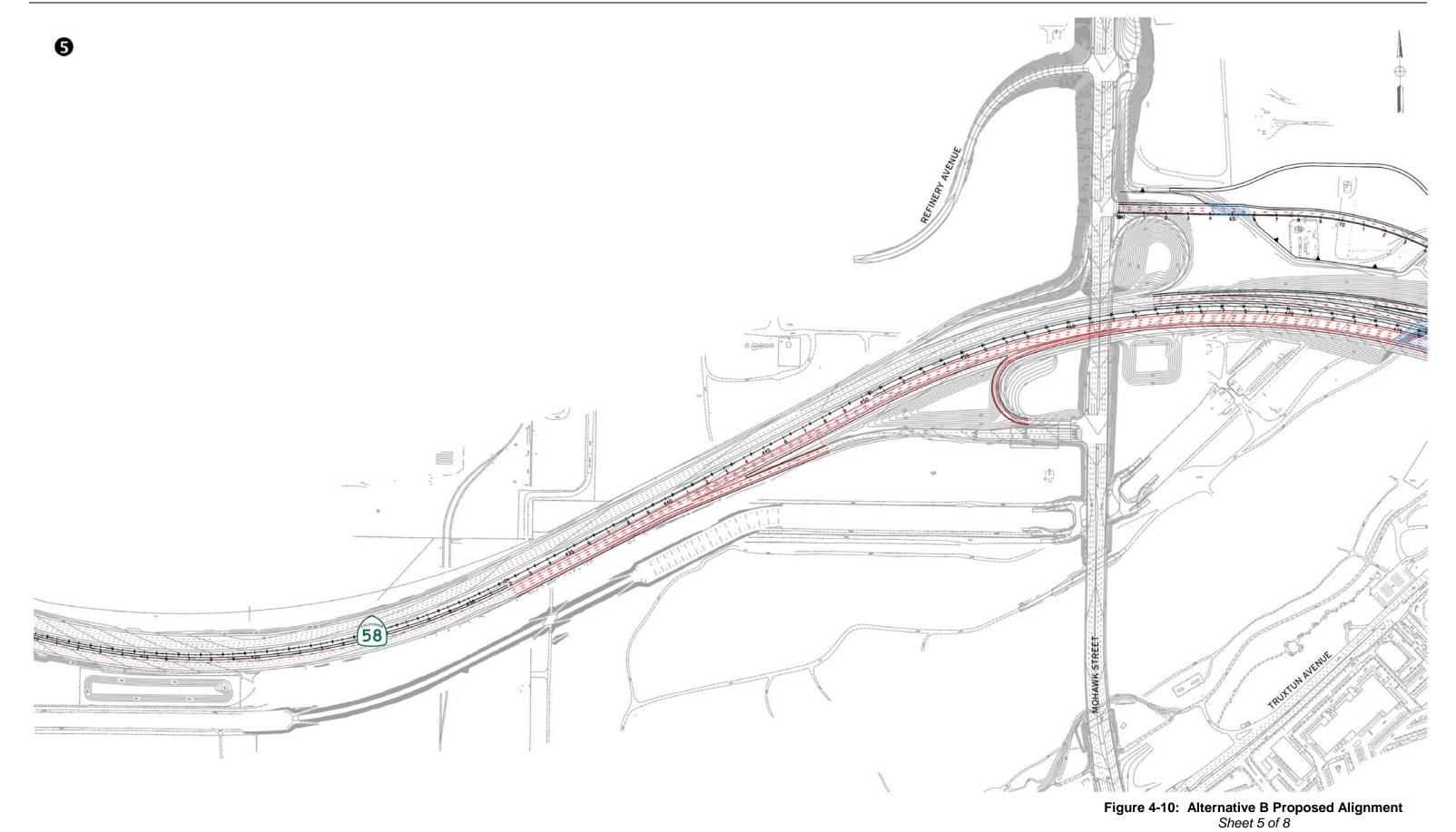
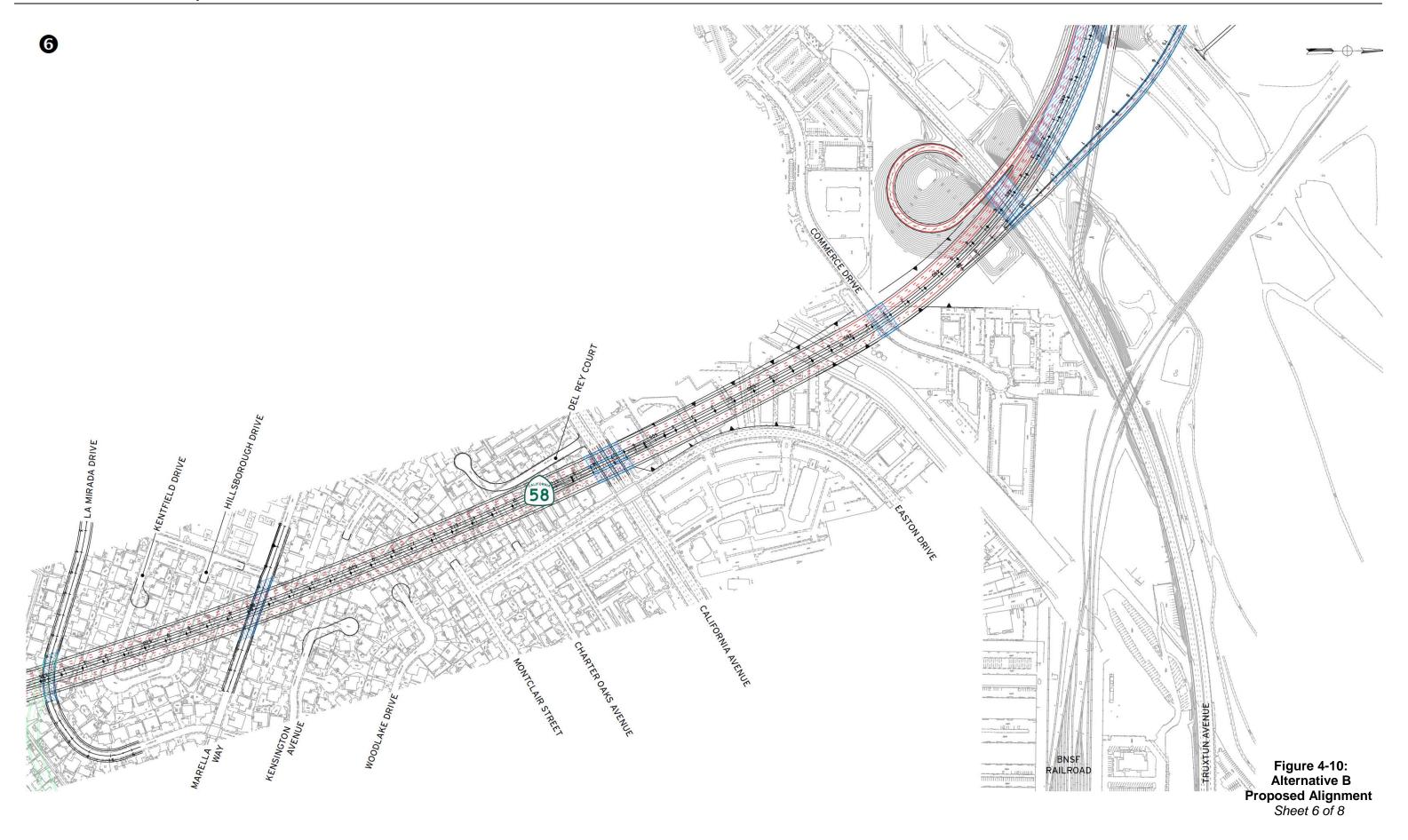


Figure 4-10: Alternative B Proposed Alignment Sheet 4 of 8



PARSONS







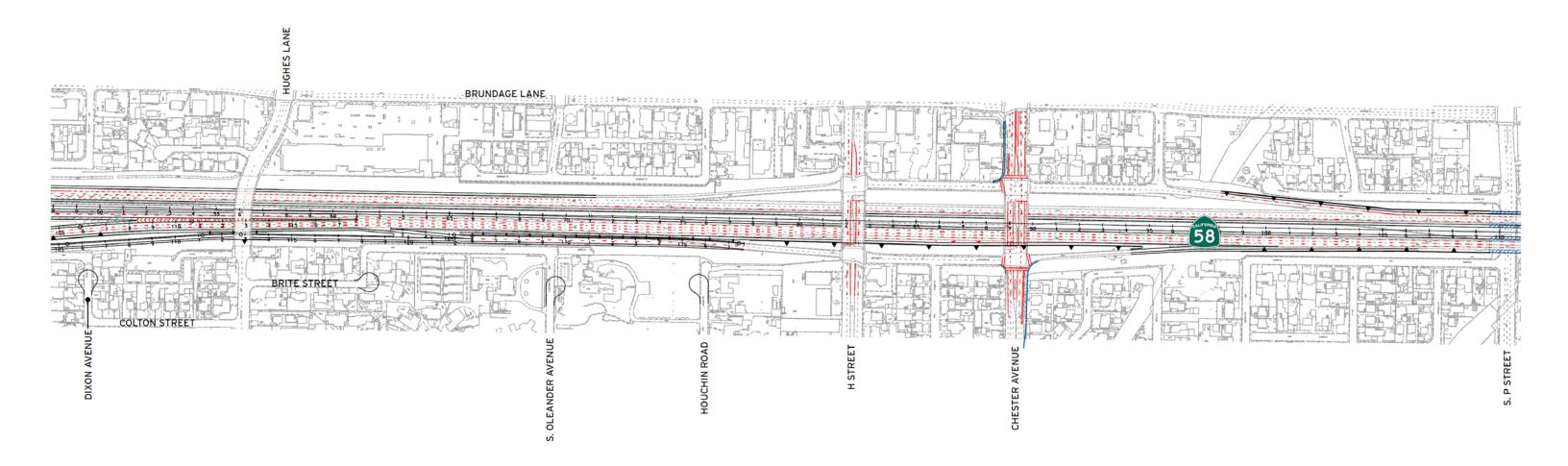


Figure 4-10: Alternative B Proposed Alignment Sheet 7 of 8



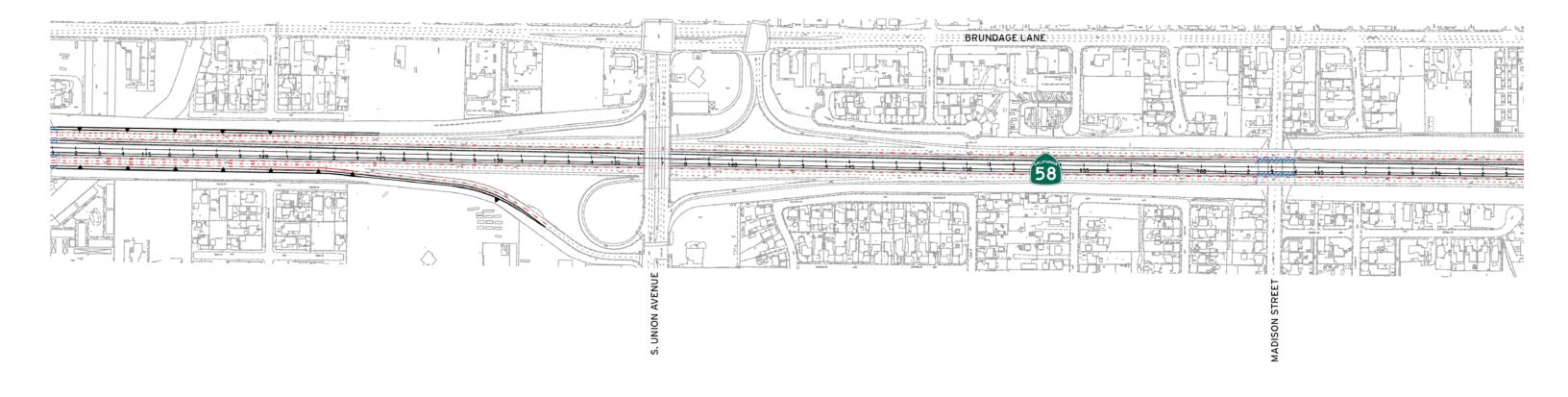


Figure 4-10: Alternative B Proposed Alignment Sheet 8 of 8

State Route 58 Eastbound

2038 ALTERNATIVE B

A. State Route 58 and Westside Parkway

AM Peak PM Peak ADT MAINLINE

2,220 2,215 28,565

State Route 58 Wes	thound	MAINLINE		,	WEST BELTW	AY		MAINLINE		ALLEN ROAD)	MAINI INE		CALL	OWAY [DRIVE		MAINLINE		CC	FFEE R	OAD		MAINLINE
		WAINLINE	ON	MAINLINE	ON	MAINLINE	OFF	IVIAIIVLIIVE	ON	MAINLINE	OFF	IVIAIIVLIIVE	ON	MAINLINE	OFF	MAINLINE	OFF	WAINLINE	ON	MAINLINE	ON	MAINLINE	OFF	IVIAIIVLIIVE
2038 ALTERNATIVE B	AM Peak	1,674	290	1,384	305	1,079	1,485	2,564	380	2,184	1,125	3,309	575	2,734	760	3,494	600	4,094	325	3,769	160	3,609	2,175	5,784
	PM Peak	2,320	360	1,960	450	1,510	2,190	3,700	435	3,265	2,125	5,390	1,015	4,375	1,245	5,620	925	6,545	575	5,970	280	5,690	2,030	7,720
	ADT	28,518	6,050	22,468	5,920	16,548	24,720	41,268	3,650	37,618	24,777	62,395	10,255	52,140	14,800	66,940	9,725	76,665	5,695	70,970	2,965	68,005	29,395	97,400

MAINLINE

2,915 2,645 35,215

MAINLINE

3,290 375 2,945 300 39,765 4,550

> From SR 99 SB

MAINLINE

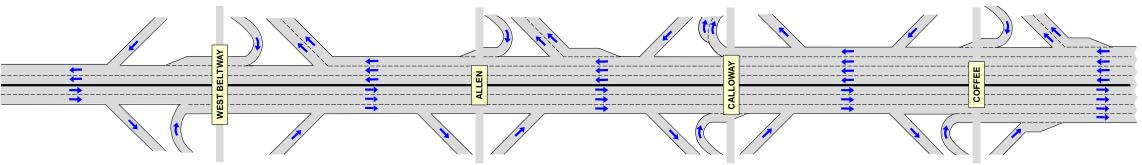
2,990 2,680 36,415 300 265 3,350

To SR 99 SB to Ming Ave

1,495 1,350 17,565

725 865 11,000 1,495 1,330 18,850





1,995 1,850 24,175
 4,910
 950
 3,960
 970
 4,930
 1,125

 4,495
 815
 3,680
 1,010
 4,690
 1,095

 59,390
 11,795
 47,595
 10,500
 58,095
 14,355

6,055 5,785 72,450 1,075 4,980 990 5,970 1,070 875 4,910 1,050 5,960 1,115 11,550 60,900 14,450 75,350 16,050

<u>2</u>	I	MOHAWK ST	REET/TRUX	KTUN AVEN	UE		MAINLINE	ST	ATE ROUTE	99 SYSTEM	INTERCHAN	IGE	MAINLINE	H S	T/CHESTE	RAVE	MAINLINE		UNI	ON AVE	NUE		MAINLINE	COTTO	DOOWNC	ROAD	MAINLINE	State B	oute 58 WB
MOHAWK ON	MAINLINE	MOHAWK ON	MAINLINE	TRUXTUN OI	N MAINLINE	MOHAWK OFF	IVIAIIVLIIVE	ON	MAINLINE	OFF	MAINLINE	OFF	IVIAIIVLIIVE	ON	MAINLINE	OFF	MAINLINE	ON	MAINLINE	ON	MAINLINE	OFF	IVIAIIVLIIVE	ON	MAINLINE	. OFF	MAINLINE	State K	oute 30 WD
2 790	4,994	655	4,339	1,020	3,319	880	4,199	1,685	2,514	1,171	3,685	1,100	4,785	700	4,085	1,095	5,180	580	4,600	375	4,225	1,115	5,340	325	5,015	205	5,220	AM Peak	2038
990	6,730	1,255	5,475	1,865	3,610	745	4,355	1,745	2,610	1,220	3,830	1,095	4,925	715	4,210	1,100	5,310	750	4,560	355	4,205	990	5,195	520	4,675	295	4,970	PM Peak	ALTERNATIVE
11,005	86,395	14,625	71,770	22,085	49,685	9,815	59,500	23,940	35,560	14,990	50,550	15,355	65,905	12,835	53,070	17,190	70,260	9,320	60,940	4,810	56,130	12,625	68,755	5,780	62,975	3,125	66,100	ADT	В
		_/•/	,	1	\ \ \ \ \ \ \ \ \ \ \ \ \ \ \ \ \ \ \	ruxtun Ave		From SR 99 NB	SR 99		To SR 99 N	В		/4/											Ţ	/	1		
4					/,		-	/							~								—						
◆													-]					z									

	MOHAWK STREET/TRUXTUN AVENUE				MAINLINE		STATE ROU	TE 99 SYSTEM INTE	RCHANGE		MAINLINE	H ST	/CHESTER	AVE	MAINLINE		U	NON A	VENUE		MAINLINE	COTTO	DNWOOD F	ROAD	MAINLINE	State D	oute 58 EB
MOHAWK	OFF N	MOHAWK ON	TRUXTUN OFF	MOHAWK ON	IVIAINLINE	OFF	MAINLINE	H ST/TO CHESTER	MAINLINE	ON	MAINLINE	OFF	MAINLINE	ON	WAINLINE	OFF	MAINLINE	ON	MAINLINE	ON	IVIAIIVLIIVE	OFF	MAINLINE	ON	WAINLINE	State K	oute 30 ED
1,82	0	515	1,600	515	4,135	2,040	2,095	333	1,762	2,165	3,927	322	3,605	995	4,600	1,020	3,580	325	3,905	330	4,235	635	3,600	315	3,915	AM Peak	2038
3 1,65	0	1,090	1,400	1,090	5,115	2,355	2,760	367	2,393	3,070	5,463	408	5,055	1,205	6,260	1,050	5,210	635	5,845	565	6,410	940	5,470	295	5,765	PM Peak	ALTERNATIVE
21,80	0	9,575	17,300	9,575	61,875	20,885	40,990	5,653	35,937	35,020	70,957	5,947	65,010	13,830	78,840	16,020	62,820	6,095	68,915	4,685	73,600	9,230	64,370	3,055	67,425	ADT	В

Figure 4-11: Alternative B
Freeway Lane Configuration and
Forecast Traffic Volumes (1 of 2)

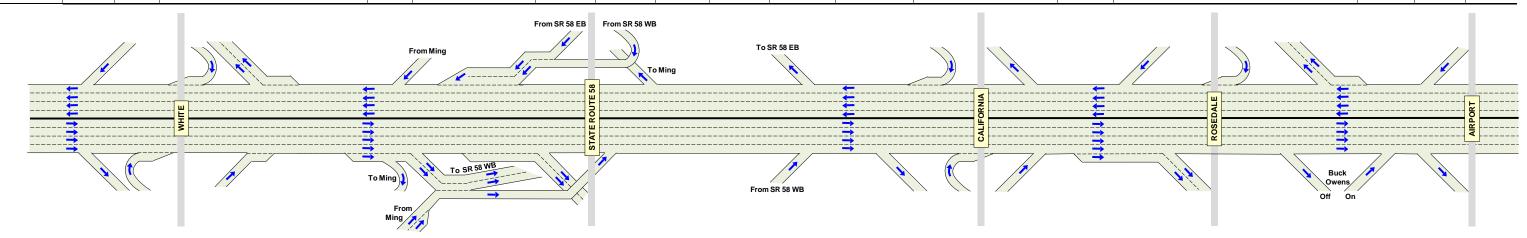
MAINLINE

7,040 7,075 91,400

B. State Route 99



State Route 99 Southbound		MAINLINE		V	WHITE L	ANE		MAINLINE			MING AVE	NUE/STATE	ROUTE 58			MAINLINE	CAL	LIFORNIA A	VENUE	MAINLINE		ROSE	EDALE HIG	GHWAY		MAINLINE	AIRPORT DRIVE	MAINLINE
Southbound			ON	MAINLINE	ON	MAINLINE	OFF		MING ON	MAINLINE	SR 58 ON	MAINLINE	MING OFF	MAINLINE	SR 58 EB OFF		ON	MAINLINE	OFF		ON	MAINLINE	ON	MAINLINE	OFF		ON	
	AM Peak	5,510	435	5,075	240	4,835	1,700	6,535	775	5,760	2,610	3,150	840	3,990	915	4,905	535	4,370	1,455	5,825	655	5,170	1,095	4,075	1,350	5,425	1,400	4,025
2038 ALTERNATIVE B	PM Peak	7,570	625	6,945	540	6,405	2,060	8,465	855	7,610	2,860	4,750	1,125	5,875	1,415	7,290	950	6,340	1,660	8,000	1,070	6,930	1,395	5,535	1,290	6,825	2,085	4,740
ALILINATIVED	ADT	95,410	9,300	86,110	7,380	78,730	20,400	99,130	10,515	88,615	26,220	62,395	12,955	75,350	15,570	90,920	10,250	80,670	21,515	102,185	10,300	91,885	15,700	76,185	15,415	91,600	24,095	67,505



State Route	99	MAINLINE		V	HITE L	NE		MAINLIN				MING AV	'ENUE/STATE R	OUTE 58				MAINLINE		CALIFO	RNIA AV	ENUE		1AINLINE		ROSEDA	ALE HIG	HWAY		MAINLINE	AIRPORT DRIVE	MAINLINE
Northbound			OFF	MAINLIN	E ON	MAINLIN	NE ON		MING OFF	MAINLINE	SR 58 WB OFF	MAINLINE	SR 58 EB OFF	MAINLINE	MING ON	MAINLINE	SR 58 WB ON		OFF	MAINLINE	ON	MAINLINE	ON IV	IAINLINE	FF M	AINLINE	OFF	MAINLINE	ON		OFF	
0000	AM Peak	6,945	1,080	5,865	1,630	7,495	705	8,200	875	7,325	1,270	6,055	1,250	4,805	950	5,755	1,100	6,855	1,100	5,755	1,060	6,815	470	7,285 1,	'40	5,545	820	4,725	695	5,420	1,720	3,700
2038 ALTERNATIVE A	PM Peak	6,695	1,095	5,600	1,630	7,230	675	7,905	860	7,045	1,205	5,840	1,655	4,185	775	4,960	1,095	6,055	685	5,371	1,370	6,740	910	7,650 1,6	30	6,020	370	5,650	775	6,425	1,405	5,020
	ADT	94,670	11,490	83,180	19,000	102,18	8 0 8,81	0 110,990	11,305	99,685	16,655	83,030	19,450	63,580	9,190	72,770	15,355	88,125	5,580	82,545	16,650	99,195 7	7,215 1	1 06,410 20	550 8	35,860	6,900	78,960	10,600	89,560	21,600	67,960

Figure 4-11: Alternative B
Freeway Lane Configuration and
Forecast Traffic Volumes (2 of 2)

illustrated in blue indicated roadways receiving less traffic as a result of building the freeway connector. The difference plot was very similar to that provided for alternative A, as the change in mobility and accessibility offered by alternatives A and B is virtually the same.

In a like manner, Figure 4-12 illustrates the location and severity of traffic congestion on the alternative B road network, much the same as identified for alternative A. In other words, very little difference would be expected between alternatives A and B insofar as regional traffic accommodation. Differences would be expected, however, in local, neighborhood circulation patterns.

Similar to the information reported for the no-build and build alternatives, Table 4-22 presents measures of effectiveness performance data for alternative B AM peak hour conditions, while Table 4-23 presents PM peak hour data. Both sets of data are extracted from the CORSIM traffic simulation model output and pertain to freeway mainline segments. Segments expected to operate with LOS E or F performance in the design year of 2038 are highlighted.

Alternative B freeway performance results are similar to those reported earlier for alternative A. With respect to 2038 AM peak hour conditions, Table 4-22 indicates that only one short segment of the freeway system, northbound State Route 99 between the White Lane loop on-ramp and the White Lane diagonal on-ramp, will operate at LOS E. Of the 80 freeway segments analyzed, 63 will operate at LOS C or better, 16 at LOS D, and one at LOS E, as mentioned above. Speeds will range between 55 and 64 mph and nearly all of the demand volumes will be served.

During the PM peak hour, traffic operations will be nearly as good as AM peak hour conditions. Of the 80 freeway segments analyzed, 53 will operate at LOS C or better, 25 segments will operate at LOS D, and three segments will operate at LOS E or LOS F.

- Southbound State Route 99 between the Rosedale Highway loop on-ramp to the diagonal on-ramp (LOS E)
- Southbound State Route 99 between the Rosedale Highway diagonal on-ramp and the California Avenue off-ramp (LOS F)
- Southbound State Route 99 between the Ming Avenue on-ramp and the White Lane off-ramp (LOS F).

The percentage of demand traffic volumes served will fall below 90 percent in one reach of State Route 99.

• Northbound between the California loop on-ramp and the Airport Drive off-ramp (85 to 88 percent served)

Along the Westside Parkway/State Route 58, speeds will range between 57 and 64 mph. Along State Route 99, speeds will range between 51 and 63 mph for the most part.

Motorists will experience slower speeds, below 50 mph, on three segments of southbound State Route 99 during the PM peak hour.

Freeway on-ramp and off-ramp performance and interchange merge/diverge conditions are reported in Tables 4-24 and 4-25, and 4-26 and 4-27, respectively, for AM and PM peak hours. Ramp terminal and other study intersection level of service performance are provided in Table 4-28. The results are similar to those reported for build alternative A.